

## **Residential Plan Review**



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## **Residential Plan Review**

Todays program is based on the 2018 International Residential Code with Minnesota amendments – the 2020 MN Residential Code. Some of the content & material is reproduced from the 2018 IRC TM, copyright © 2018, Developed by the International Code Council, Inc.

## Learning Statement

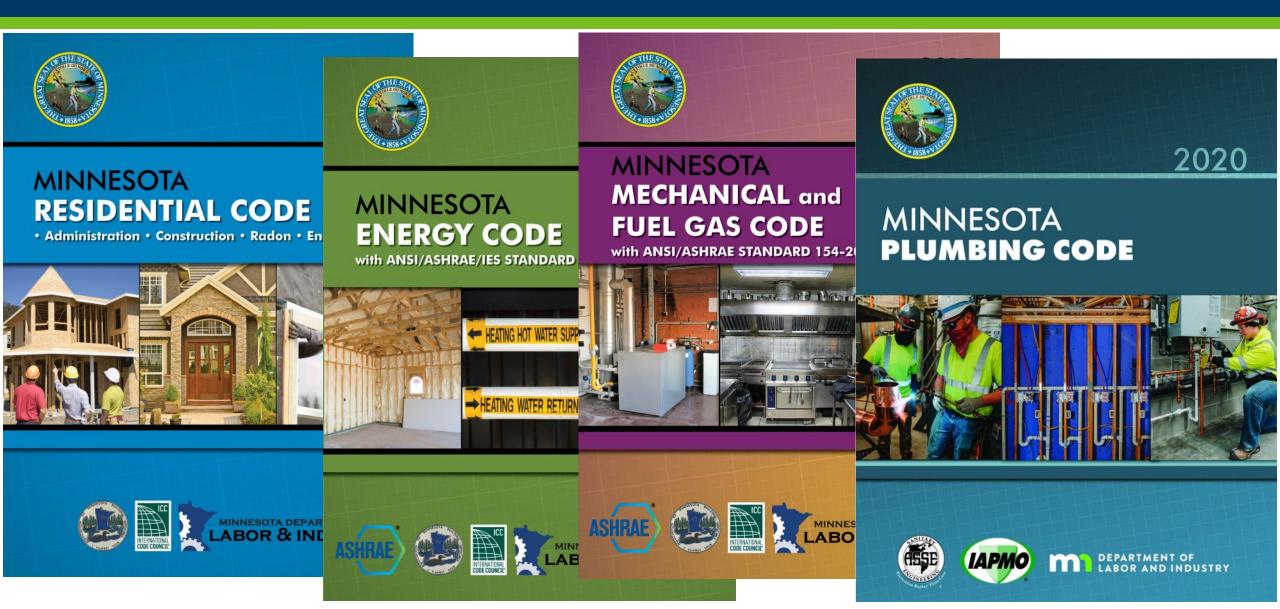
- Students will become familiar with how to maneuver through the Minnesota Residential Code (MNRC).
- Students will be able to understand the review process of a building permit application and the submittal document requirements.
- Students will examine a single-family dwelling house plan in order to identify and understand building code requirements and concerns.

- MN Residential Code book
- Permit Application Process
- Plan review

10/31/202

Agenda

## Code Books



## International Residential Building Code Book - IRC

- Written by ICC (International Code Council)
- IRC Contains:
  - Residential Building.
  - Residential Mechanical/Fuel Gas.
  - Residential Plumbing.
  - Solar Provisions.
  - Electrical.
  - Swimming Pools.
  - Appendices...
- IRC is designed to be a "stand alone" resource for residential construction.



## Minnesota Residential Code Book - MNRC

- MNRC adapted from ICC text based on 2018 IRC with MN amendments
  - 2015 MNRC was based on 2012 IRC.
  - IRC updated on 3-year cycle; MN updated on 6-year cycle.
    - IRC 2012 = MN 2015, IRC 2015 = MN skip, IRC 2018 = MN 2020, IRC 2021 = MN skip, IRC 2024 = MN 2026.
  - Next MN adoption will be 2024 IRC in 2026.
  - Stops at Ch. 10 Chimneys & Fireplaces.
  - Other sections after ch. 10 are deleted and replaced with other texts (MN Mechanical & Fuel Gas code, IAPMO Plumbing code).
  - Most appendices not adopted except for Sound Transmission & Tiny Houses.

## Other MN Code Books – Residential

- Codes mostly adapted from ICC texts
- 2020 MN Energy Code based on 2012 IECC.
  - We did not adopt the 2018 IECC.
- 2020 MN Mechanical & Fuel Gas Code (two books in one compilation) based on 2018 IMC & IFGC.
- 2020 MN Plumbing Code based on 2018 UPC (Uniform Plumbing Code) written by IAPMO (International Association of Plumbing & Mechanical Officials), not an ICC book.

## Familiarization of the Code Book

- Marginal markings/symbols (page vii)
- Effective use of the code (pages ix thru xi)
- Contents (pages ix thru xxi)
- Index (pages 563 thru 570)
- Reference Standards (pages 535 thru 558)
- Numbering system (see Section R602.10.2, page 234)

## Marginal Markings

### **Marginal Markings**

- Indicates where a paragraph or item has been deleted from the requirements of the 2015 International Residential Code.
- > = Indicates model code language deleted by the State of Minnesota.
- Indicates a technical change from the requirements of the 2015 International Residential Code.
- <sup>M</sup> = Indicates a State of Minnesota amendment has been made to the 2018 International Residential Code.

Solid vertical lines in the margins within the body of the code indicate a technical change from the requirements of the 2015 edition. Deletion indicators in the form of an arrow ( $\implies$ ) are provided in the margin where an entire section, paragraph, exception or table has been deleted or an item in a list of items or a table has been deleted.

A single asterisk [\*] placed in the margin indicates that text or a table has been relocated within the code. A double asterisk [\*\*] placed in the margin indicates that the text or table immediately following it has been relocated there from elsewhere in the code. The following table indicates such relocations in the 2018 edition of the *International Residential Code*.

2018 LOCATION	2015 LOCATION
R703.3.1.2	R703.11.1.4

- Ch. 1 Deleted & Replaced with MN Rules 1300 (covered in a separate presentation)
- MN Provisions 1303
  - Footing depth, Snow Load, Radon requirements, etc.
- MN Residential Energy Code 1322
  - Begins with definitions pertaining to Energy section.
  - Amended International Energy Conservation Code (residential portion)

- Ch. 2 Definitions pertaining to Building Code section
- Ch. 3 Building Planning
  - R301 Design:
    - Wind, snow, seismic (seismic not recognized in MN)
  - R302 Fire Resistance
  - R303 Light, Ventilation, & Heating
  - R304 Minimum Room Area
  - R305 Ceiling Height
  - R306 Sanitation
  - R307 Toilet, Bath, & Shower Spaces
  - R308 Glazing

- R309 Garages & Carports
- R310 Emergency Escape & Rescue Openings (EEROs)
- R311 Means of Egress
  - Stairs, landings, handrails, ramps
- R312 Guards & Window Fall Protection
- R313 Automatic Fire Sprinklers Systems
- R314 Smoke Alarms
- R315 Carbon Monoxide Alarms
- R316 Foam Plastic

- R317 Protection of Wood & Wood Based Products Against Decay
- R318 Protection Against Subterranean Termites
- R319 Site Address
- Ch. 4 Foundations
  - Footings
  - Foundation walls
  - Drain tile
  - Water proofing (also located in Energy code, and is more restrictive)
- Ch. 5 Floors
- Ch. 6 Walls
  - Including wall bracing

- Ch. 7 Wall Covering
  - Interior
    - Gypsum/fastening
    - Vapor retarder
  - Exterior
    - Water-resistive barrier
    - Flashing
    - Siding

- Ch. 8 Roof-Ceiling Construction
  - Ceiling joists
  - Rafters
- Ch. 9 Roof Assemblies (roof coverings)
- Ch. 10 Chimneys & Fireplaces

## Reading the Code

### Always:

- Read the entire section
- Look for and read all the exceptions
- Verify the section applies to the situation (context!)



## Reading the Code

**Using Tables:** (This also applies to listed/tested manufacturer's installation or user manuals/tables.)

- Verify the table number corresponds to code section
- Verify that the information in the headings, columns, and rows fits the condition
- Read the footnotes in order of their use

## **Table Footnotes**

TABLE R301.5 MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (in pounds per square foot)							
USE	LIVE LOAD						
Uninhabitable attics without storage <sup>b</sup>	10						
Uninhabitable attics with limited storage <sup>b, g</sup>	20						
Habitable attics and attics served with fixed stairs	30						
Balconies (exterior) and decks <sup>e</sup>	40						
Fire escapes	40						
Guards and handrails <sup>d</sup>	200 <sup>h</sup>						
Guard in-fill components <sup>f</sup>	50 <sup>h</sup>						
Passenger vehicle garages <sup>a</sup>	50 <sup>a</sup>						
Rooms other than sleeping rooms	40						
Sleeping rooms	30						
Stairs	40 <sup>c</sup>						

For SI: 1 pound per square foot = 0.0479 kPa, 1 square inch = 645 mm<sup>2</sup>, 1 pound = 4.45 N.

a. Elevated garage floors shall be capable of supporting a 2,000-pound load applied over a 20-square-inch area.

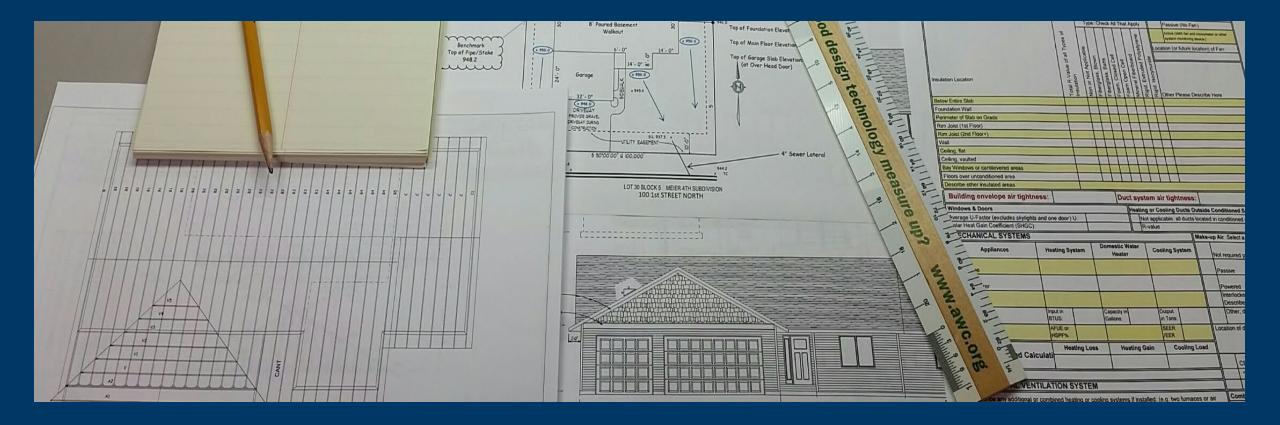
- b. Uninhabitable attics without storage are those where the clear height between joists and rafters is not more than 42 inches, or where there are not two or more adjacent trusses with web configurations capable of accommodating an assumed rectangle 42 inches in height by 24 inches in width, or greater, within the plane of the trusses. This live load need not be assumed to act concurrently with any other live load requirements.
- c. Individual stair treads shall be designed for the uniformly distributed live load or a 300-pound concentrated load acting over an area of 4 square inches, whichever produces the greater stresses.
- d. A single concentrated load applied in any direction at any point along the top.
- e. See Section R507.1 for decks attached to exterior walls.
- f. Guard in-fill components (all those except the handrail), balusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to 1 square foot. This load need not be assumed to act concurrently with any other live load requirement.
- g. Uninhabitable attics with limited storage are those where the clear height between joists and rafters is 42 inches or greater, or where there are two or more adjacent trusses with web configurations capable of accommodating an assumed rectangle 42 inches in height by 24 inches in width, or greater, within the plane of the trusses.

The live load need only be applied to those portions of the joists or truss bottom chords where all of the following conditions are met:

- The attic area is accessed from an opening not less than 20 inches in width by 30 inches in length that is located where the clear height in the attic is not less than 30 inches.
- The slopes of the joists or truss bottom chords are not greater than 2 inches vertical to 12 units horizontal.
- Required insulation depth is less than the joist or truss bottom chord member depth.

The remaining portions of the joists or truss bottom chords shall be designed for a uniformly distributed concurrent live load of not less than 10 pounds per square foot.

h. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independent of one another, and loads are assumed not to occur with any other live load.



# Code, Permit, & Application

## Why do we have a building code?

### Purpose – MN Rules 1300.0030 Subp. 1

- The purpose of this code is to establish minimum requirements to safeguard the public health, safety, and general welfare through structural strength, means of egress facilities, stability, sanitation, adequate light and ventilation, energy conservation, and safety to life and property from fire and other hazards attributed to the built environment and to provide safety to firefighters and emergency responders during emergency operations.
- The purpose of the code is not to create, establish, or designate a particular class or group of persons who will or should be especially protected or benefited by the terms of the code.

## Why do I need a permit?

### Permit Required – MN Rules 1300.0120 Subp. 1

 An owner or authorized agent who intends to construct, enlarge, alter, repair, move, demolish, or change the occupancy of a building or structure, or to erect, install, enlarge, alter, repair, remove, convert, or replace any gas, mechanical, electrical, plumbing system, or other equipment, the installation of which is regulated by the code; or cause any such work to be done, shall first make application to the building official and obtain the required permit.

## What documents are required?

### Application for a Permit – MN Rules 1300.0120 Subp. 7

#### Subp. 7. Application for permit.

To obtain a permit, the applicant shall file an application in writing on a form furnished by the Department of Building Safety for that purpose. The application shall:

- A. Identify and describe the work to be covered by the permit for which application is made;
- B. Describe the land on which the proposed work is to be done by legal description, street address, or similar description that will readily identify and definitely locate the proposed building or work;
- C. Indicate the use and occupancy for which the proposed work is intended;
- D. Indicate the type of construction;
- E. Be accompanied by construction documents and other information as required by the code;
- F. State the valuation of the proposed work;
- G. Be signed by the applicant, or the applicant's authorized agent; and
- H. Give other data and information required by the building official.

## More specifically...

For example...Provide in submittal documents:

- Survey/site plan showing lot size, building size, setbacks from property lines, other buildings, sewer/ water, septic systems, wells, and any other items required by code or ordinance.
- Two sets of complete plans and specifications (or electronic plans).
- Energy code compliance certificate.
  - Insulation type & R-value.
  - Minimum ventilation requirements Minnesota Energy Code 1322 Chapter 4.
  - Combustion air calculations IFGC 304.1, IFGC Appendix E, worksheet E-1.
  - Makeup air calculations. IMC 501.4.1, IMC Chapter 5 Table 501.4.1.
- Floor plans for all levels. Include room size, room use, kitchen/bathroom layout, dimensions of stairway and location, window and door location with sizes, and decks/porches.

## More specifically...

- Exterior elevations, showing top of foundation in relation to final grade, grading and drainage, windows, doors, siding type, roof pitch, roof covering, decks and other pertinent information.
- Typical wall section or section through the building. The following items should include but not be limited to:
  - Footing size (and reinforcing if needed).
  - Foundation type, size, height, and reinforcing.
  - Foundation drainage system (drain tile).
  - Foundation waterproofing.
  - Foundation insulation type & R value.
  - Rim joist insulation type & R value.

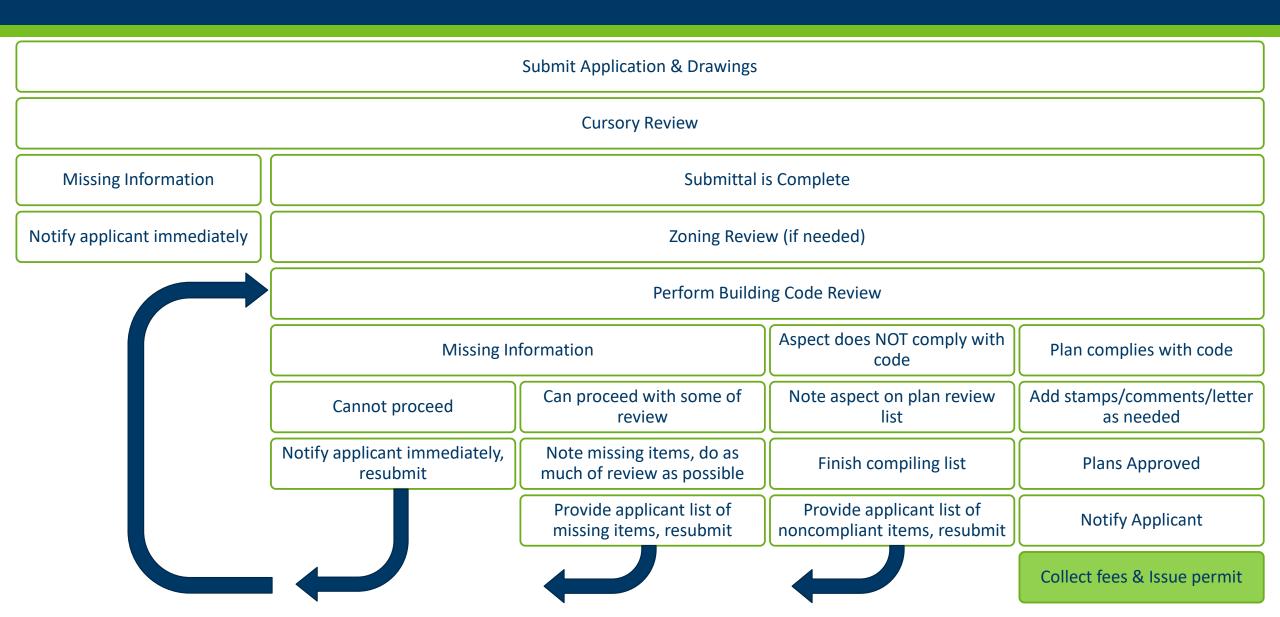
## More specifically...

- Floor joist type, size, & spacing
- Subfloor material & thickness
- Wall thickness & stud spacing
- Wall sheathing type and thickness
- Water-Resistive Barrier method (House wrap)

## Permit Process & Plan Review



## **Permit Application Process**



## Why do we conduct a plan review?

- Catch errors
  - Save time.
  - Save money.
  - Opportunity to identify safety issues.
  - Field corrections are difficult to make.
- Inspector & Contractor/Owner work from the same approved plan
  - Prepares you to conduct a more effective field inspection.
  - Familiarity with the project
- Applicant is *paying* for a plan review! Deliver the service.

## Process

	Cursory Review	
Missing Information		
Notify applicant immediately		

- Look over documents briefly to ensure you have all of the information.
  - Permit tech or administrative support staff.
  - Inspector.
  - Ideally at the counter, or within 24 hrs. of receiving application.
- Notify the applicant immediately if items are missing or incomplete.

# Application

#### MN Rules 1300.0120 Subp. 7

- The permit application shall:
  - Identify work
  - Describe the land
  - Indicate use and occupancy
  - Type of construction
  - Accompanied by construction documents
  - State the value
  - Signed by applicant
  - Other info as required by B.O.

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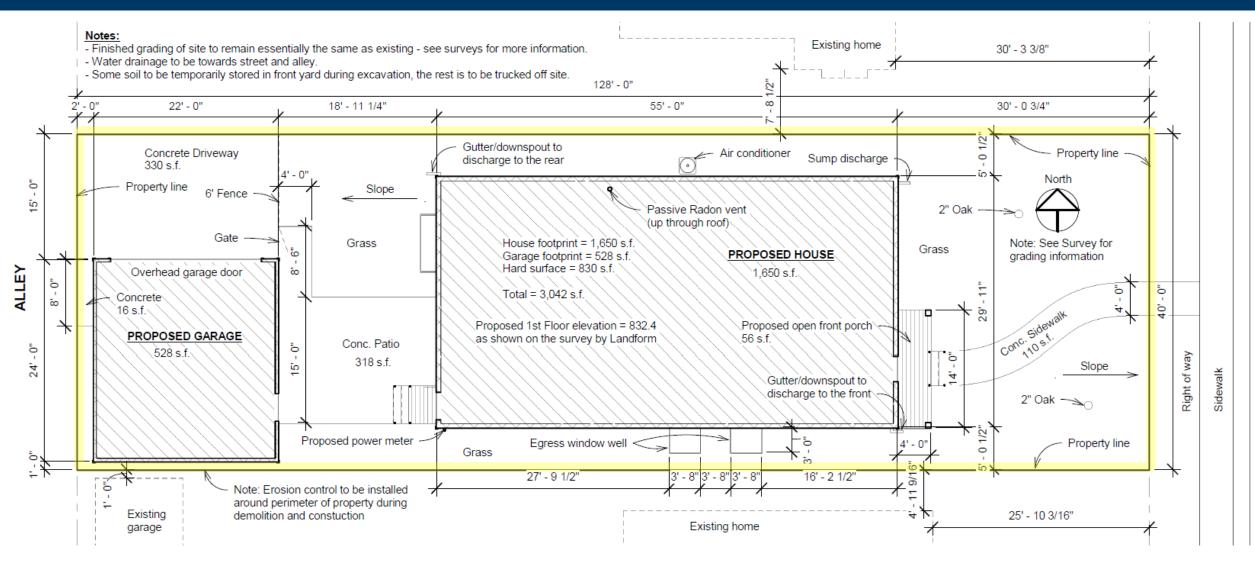
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## Process

Submittal is Complete	
Zoning Review (if needed)	

- If the submittal is complete, route documents through Zoning.
  - Site Plan/Survey will be reviewed.
  - Will the structure fit on the lot?
  - Is the structure too large?
  - Is the structure allowed in that area/zoning district?
  - Is a redesign merited?

## Site Plan



Site

' = 10'-0"

3

AVE S

## Process



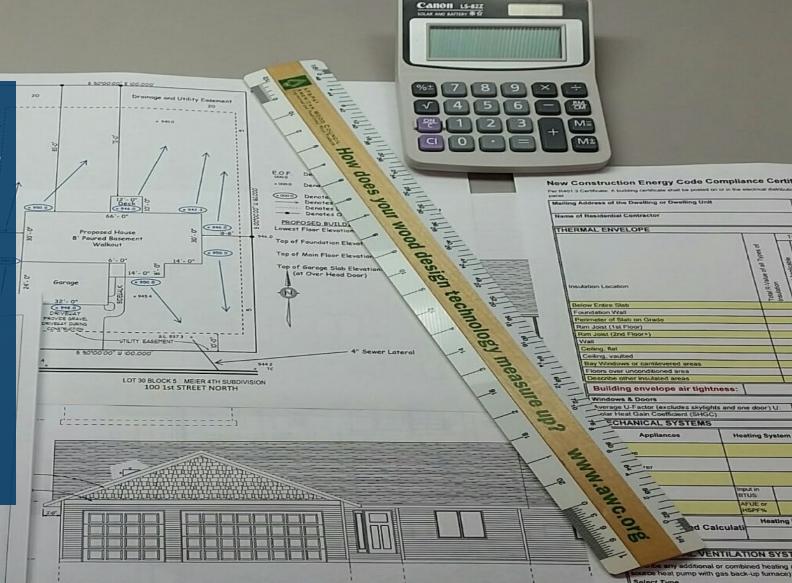
- Begin review missing something?
  - If it prevents you from doing *any* of the review, notify applicant **immediately**.
  - If it is minor, note it and start the review do as much of the review as possible.

### Supplies:

\* Discrepancies

7/1/2016 Residential Plan Review

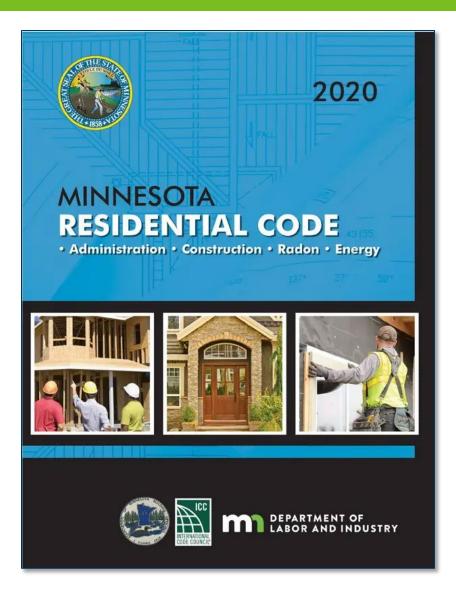
- Two or more sets of drawings
- Red pen
- Different colored highlighters
- Stamps
- Note pad
- Scale
- Calculator
- Large monitors



## How do we conduct a plan review?

#### MN Rules 1300.0070 Subp. 12b.

- What is it?
  - IRC I Dwelling, single-family
  - IRC II Dwelling, two-family
  - IRC III Townhouse
  - IRC IV Accessory structures
    - Garages
    - Storage sheds
    - Similar structures
- Be sure you are in the right book
  - MNRC Residential (1309)
  - MNBC Commercial (1305)



**Defined as:** Occupancies not more than three stories above grade plane in height with a separate means of egress shall comply with chapter 1309...

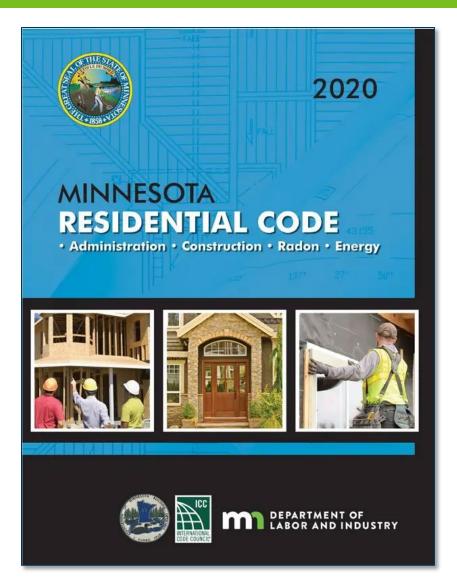






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### MN Rules 1300.0070 Subp. 12b.

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**Subp. 2.** Compliance. Structures classified under part 1300.0070, subpart 12b, as IRC-1, IRC-2, IRC-3, and IRC-4 occupancies not more than three stories above grade plane in height with a separate means of egress shall comply with Chapter 1309 and other applicable rules. Other buildings and structures and appurtenances connected or attached to them shall comply with Chapter 1305 and other applicable rules.

**Exception:** The following structures that meet the scope of Chapter 1305 shall be permitted to be designed to comply with Minnesota Rules, Chapter 1311:

- (1) existing buildings undergoing repair, alteration, change of occupancy, addition, or being moved; and
- (2) historic buildings.



- Begin review
  - Start by familiarizing yourself with the plans.
  - Flip through all of the pages and understand what is being built.
  - Don't focus on getting technical or making calculations until you have a good overview of what is being built.
  - If you notice violations, or think of questions, note them.

# Are the plans submitted and designed by a licensed architect?



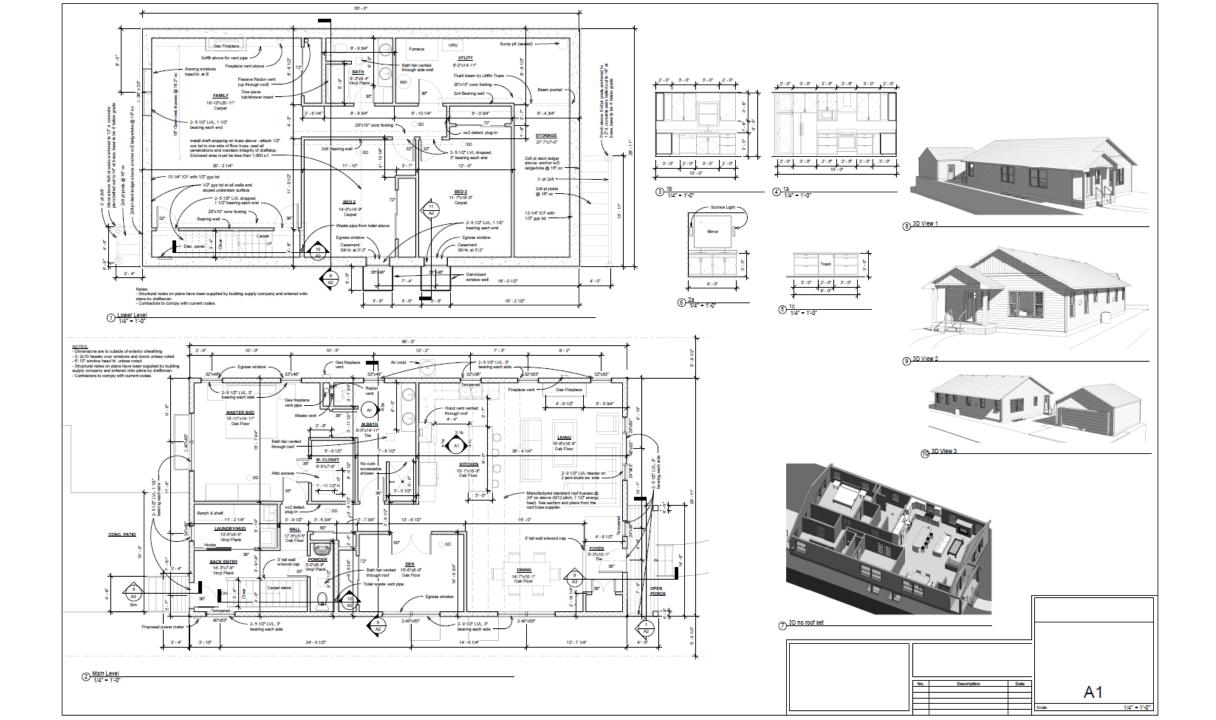
Is it *possible* to be built under the requirements of MN Rules 1309?

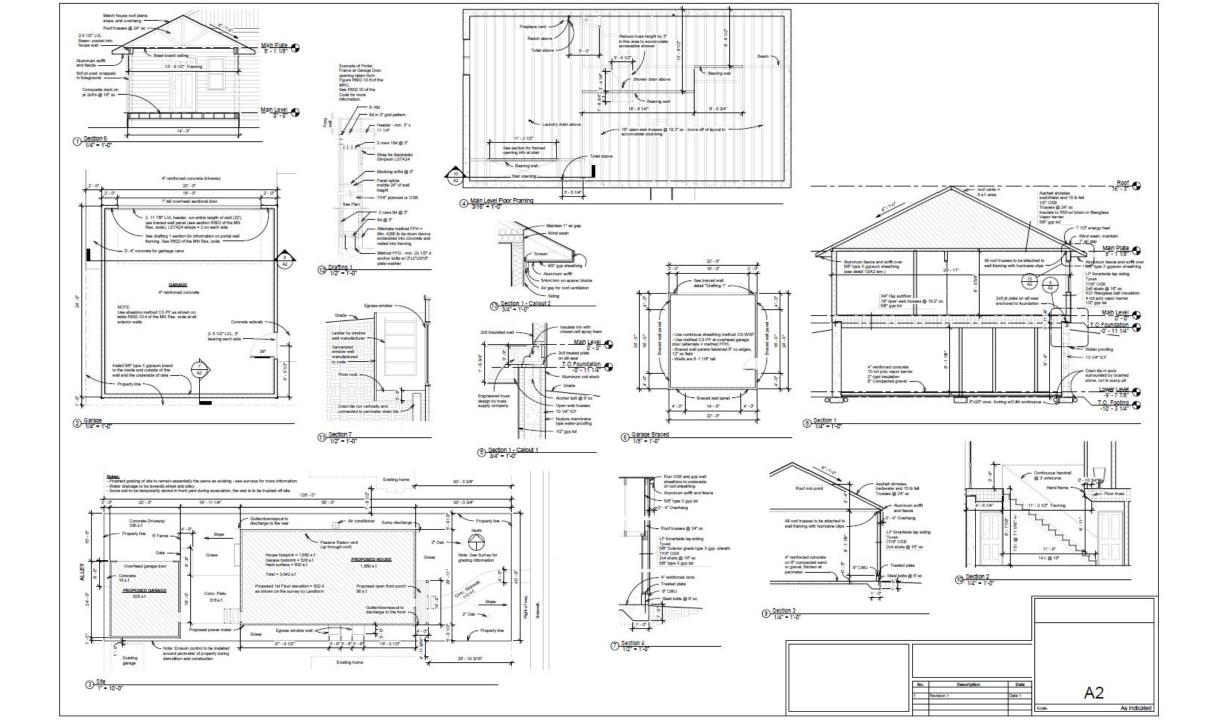
### Design

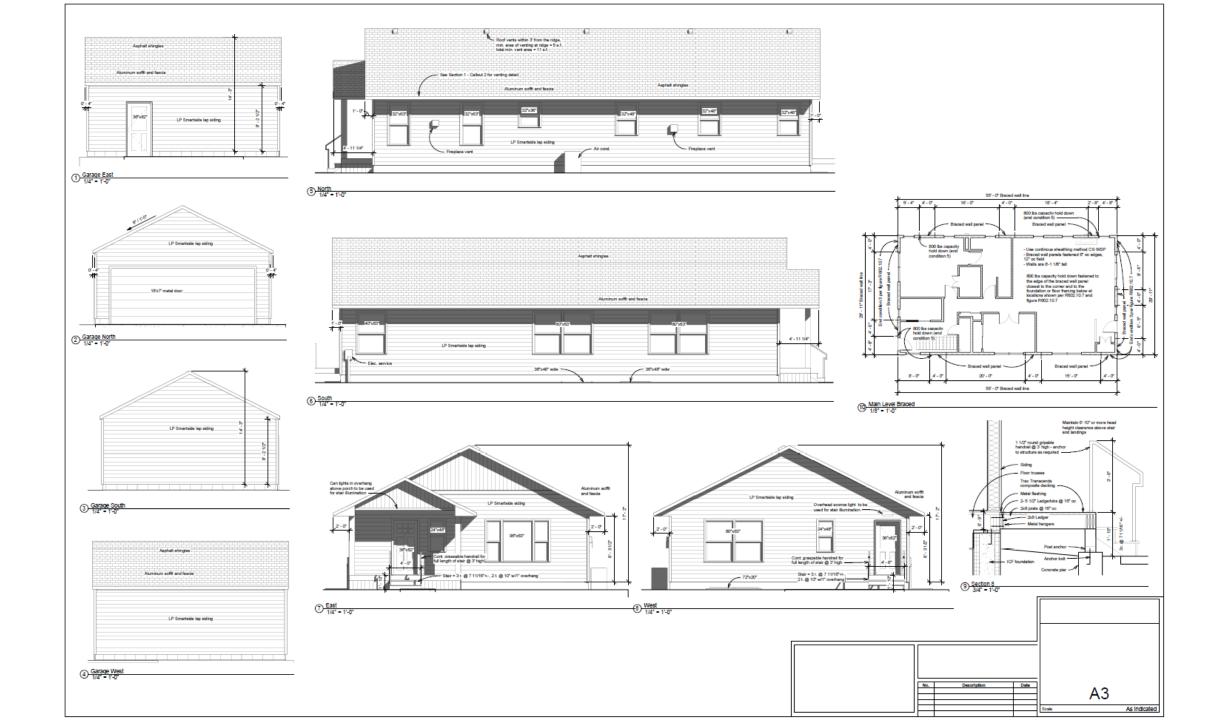
- Start to focus on the design
  - Once you understand *what* is being built, look to see if the overall design is possible under code imagine walking through the house, entering each room.
    - Hallways to rooms
    - Landings at top and bottom of stairs
    - Windows/Egress openings
    - Lighting
    - Headroom
    - (Wind, snow, live/dead load)
    - Determine *If* the structure seems plausible

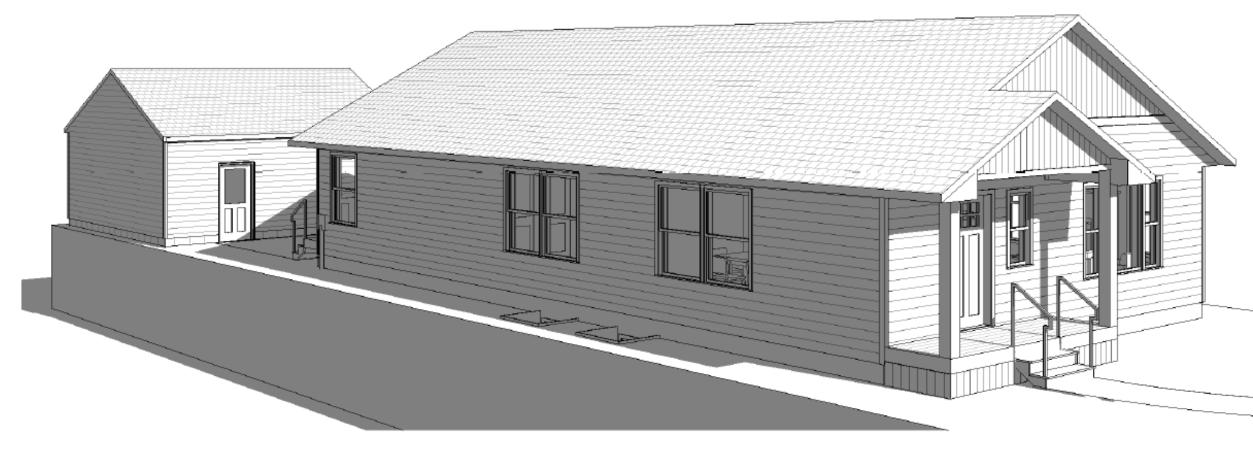


 Once you understand what is being proposed, and determine that the overall design could comply with the code, begin to look technically at the structure through the lens of code.

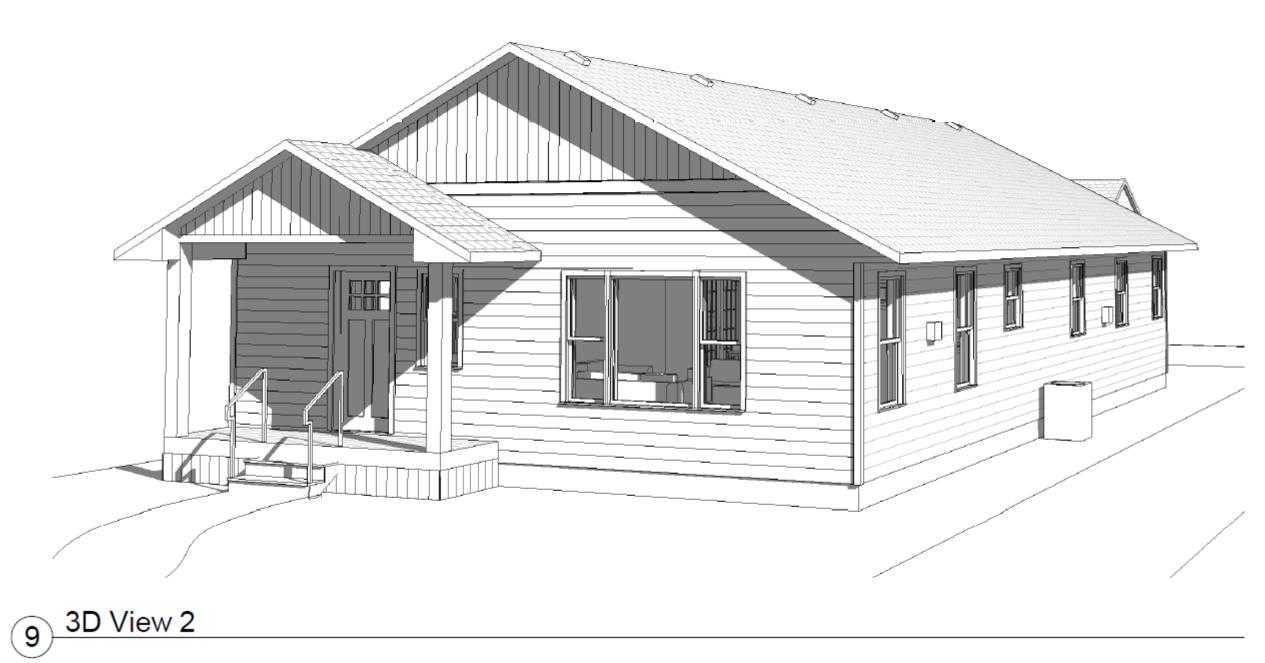


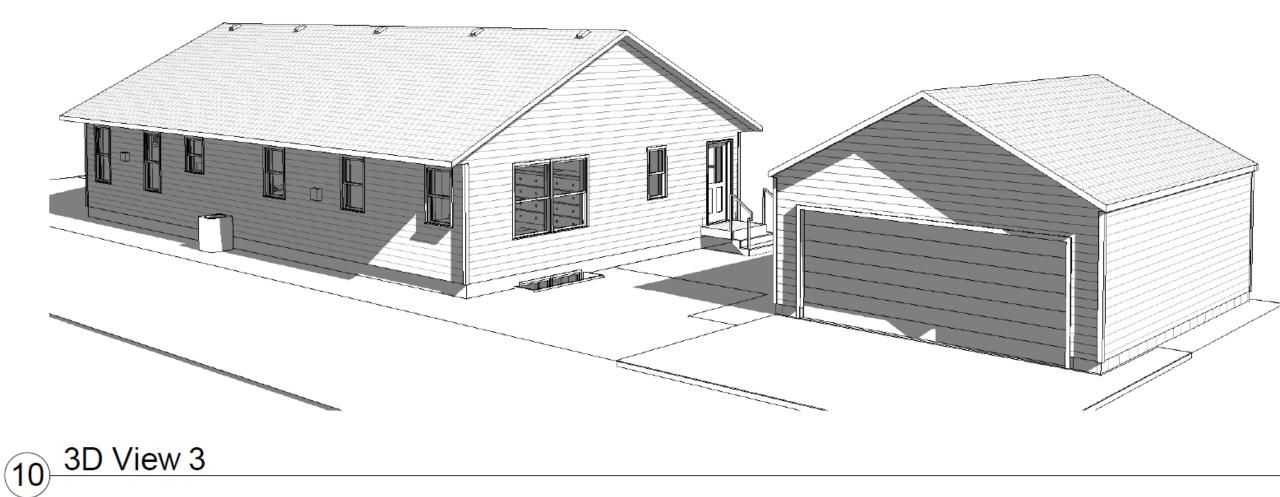


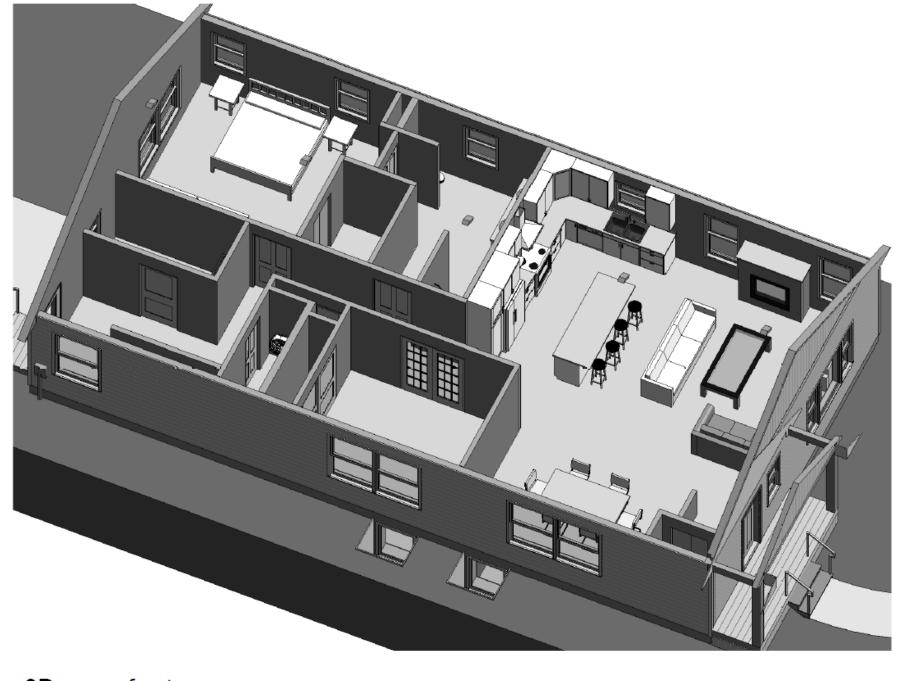












- Though not explicitly stated in code, "design" takes on a few separate but related aspects:
  - Environmental factors
    - How does the location and climate affect the building?
  - Functionality/Layout
    - Does it work?
  - Structural
    - Will the structure support the loads imposed?

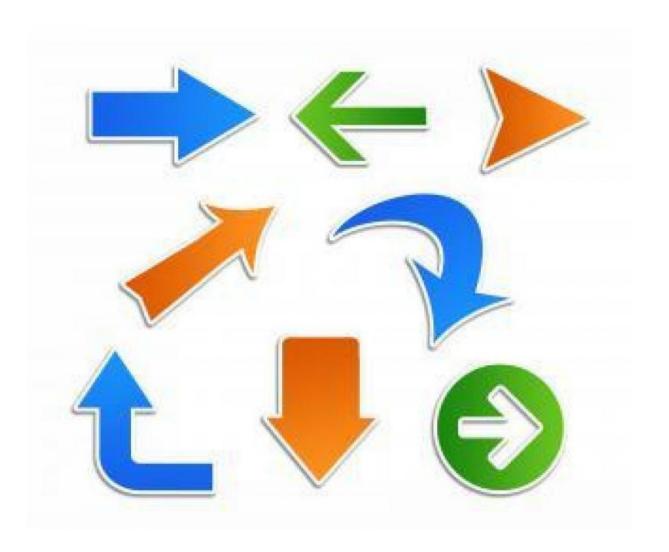
- Environmental factors
  - Ultimate wind speed
  - Wind exposure category
  - Climate Zone, temperature (heating, cooling, insulation, humidity control)
  - Flooding
  - Ground snow load
  - Roof/floor live load
  - Soil type & bearing capacity
  - Soil moisture content (frost heave)
  - Ground water mitigation
  - Radon

- Functionality/layout
  - Egress
  - Minimum room areas
  - Minimum clearances
  - Headroom
  - Halls
  - Stairs
  - Fire separation
  - Habitability/livability

### • Structural

- Loads
  - People/contents, wind, snow
- Material
  - Type/species, span capability
- Footings
- Foundation walls
- Walls
- Floors
- Roof/ceiling

- Develop an approach.
  - Top down?
  - Bottom up?
  - Outside in?
  - Inside out?
- Functionality/layout
- Structural



- Utilize a framework as a guide
  - ICC Checklists
  - Building Department checklist
  - Plan review letter
  - Code book follow table of contents



# Footings – Chapter 4



This card must be kept posted in a conspicuous place on the site of construction.

https://codes.iccsafe.org/content/MNRC2020/chapter-4-foundations

- Size
  - Width x Thickness
  - Projections
- Pads/point loads
- Frost protection
- Min 12" below natural grade (slab on grade)
- Insulation (frost protected shallow foundations)
- Drain tile

### • Soil bearing capacity for your area

• Typically 1,500 - 2,000 psi in met

.500 – 2,000 psi in metro.	X	CLASS OF MATERIAL (pour				
	Sec.	Crystalline bedrock				
		Sedimentary and foliated rock				
	24	Sandy gravel and/or gravel (GW and GP)				
	Sk	Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SP, SM, SC, GM and GC)				
	str	Clay, sandy, silty clay, clayey silt, silt and sandy siltclay (CL, ML, MH and CH)				
This card must be kept posted	J	<ul> <li>For SI: 1 pound per square foot = 0.0479 kPa.</li> <li>a. Where soil tests are required by Section R40 capacities of the soil shall be part of the recom</li> <li>b. Where the building official determines the allowable bearing capacity of less than 1,500 p the site, the allowable bearing capacity shall investigation.</li> </ul>	mendatio at in-pla sf are lik			
	10 A 10	https://codes.iccsafe.org/content/MNRC2020	)/chapte			

VALUES OF FOUNDATION MATERIALS<sup>a</sup> OAD-BEARING PRESSURE ds per square foot) 12,000 4,000 3,000 2,000 1,500<sup>b</sup> e allowable bearing tions.

> 67 construction.

place soils with an ikely to be present at etermined by a soils

**TABLE R401.4.1** 

PRESUMPTIVE LOAD-BEARING

https://codes.iccsafe.org/content/MNRC2020/chapter-4-foundations

### • Minimum compressive strength - 5000psi

#### TABLE R402.2 MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF CONCRETE

		MINIMUM SPECIFIED COMPRESSIVE STRENGTH <sup>a</sup> (f <sup>'</sup> <sub>c</sub> )					
	TYPE OR LOCATION OF CONCRETE CONSTRUCTION		Weathering Potential				
	$\frown$	Negligible	Moderate	Severe			
M N	Footing	5,000	5,000	5,000			
M N	Basement walls, foundations, and other concrete not exposed to the weather	2,500	2,500	2,500 <sup>c</sup>			
	Basement slabs and interior slabs on grade, except garage floor slabs	2,500	2,500	2,500 <sup>c</sup>			
	Basement walls, foundation walls, exterior walls, and other vertical concrete work exposed to the weather	2,500	3,000 <sup>d</sup>	3,000 <sup>d</sup>			
M N	Porches, carport slabs, and steps exposed to the weather, and garage floor slabs	2,500	3,000 <sup>d, e, f</sup>	3,500 <sup>d, e, f</sup>			

For SI: 1 pound per square inch = 6.895 kPa.

a. Strength at 28 days psi.

b. See Table R301.2(1) for weathering potential.

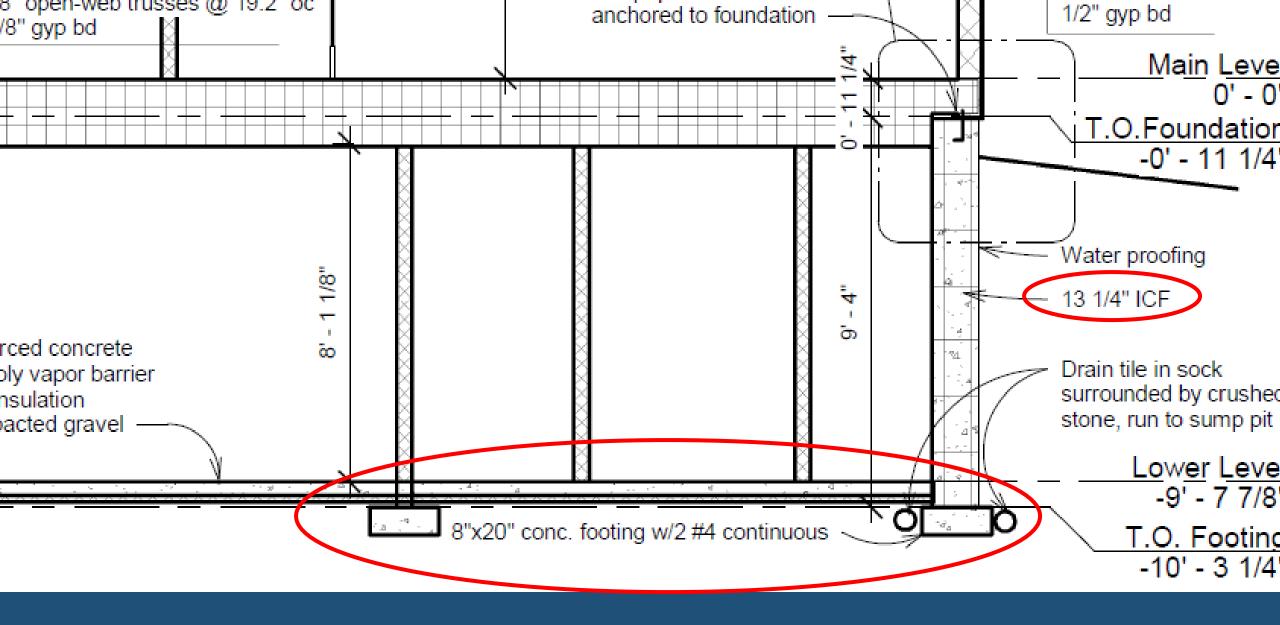
c. Concrete in these locations that may be subject to freezing and thawing during construction shall be air-entrained concrete in accordance with Footnote d.

- d. Concrete shall be air-entrained. Total air content (percent by volume of concrete) shall be not less than 5 percent or more than 7 percent.
- e. See Section R402.2 for maximum cementitious materials content.

f. For garage floors with a steel-troweled finish, reduction of the total air content (percent by volume of concrete) to not less than 3 percent is permitted if the specified compressive strength of the concrete is increased to not less than 4,000 psi.

g. Compressive strength (f'c) of 2,500 psi, with an approved admixture that provides a water and vapor resistance at least equivalent to 5,000 psi concrete.

b. Compressive strength (f'c) of 5,000 psi, is not required for post footings for decks or porches, wood foundations, slab-on-grade foundation walls, and footings for floating slabs.



Plans

## Roof Snow Load

	1			c		TABLE F	<b>APHI</b>	Becker, Beltrami, Carlton, Cass, Clearwater, Cook, Crow Wing, Hubbard, Itasca, Kanabec, Kittson, Koochiching Lake, Lake of the Woods, Mahnomen, Marshall, Mille Lac Morrison, Norman, Otter Tail, Pennington, Pine, Polk, Re Lake, Roseau, St. Louis, Todd, and Wadena. The groun snow load, $P_{\sigma}$ , to be used in determining the design snow					
M N	ROOF SNOW		D DESIGN Topographic effects <sup>k</sup>	SEISMIC DESIGN CATEGORY	SUBJECT Weathering <sup>a</sup>	<b>Erect line</b>	Teri	60 pounds per square foot in the following counties: Aitkin, Becker, Beltrami, Carlton, Cass, Clearwater, Cook, Crow					
	$p_f = 0.7 * p_g$	115	Yes	А	Severe	See MR part 1303.1600	S Foo	Wing, Hubbard, Itasca, Kanabec, Kittson, Koochiching, Lake, Lake of the Woods, Mahnomen, Marshall, Mille Lacs, Morrison, Norman, Otter Tail, Pennington, Pine, Polk, Red					
f	. The ground s Ground Snov	now loa v Load to	ds to be used o verify by cou	The Lake Lake of the Woods Mahnomen Marshall Mille Lage									

M N

M N M

0.7 x 50lbs/sf = <u>35lbs/sf</u> Roof Snow Load (Southern half)
0.7 x 60lbs/sf = 42lbs/sf Roof Snow Load (Northern half)

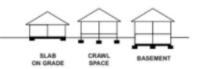
	MINIMUM WIDTH AND THICKNESS FOR CONCRETE FOOT	INGS WITH CAST-IN-PLACE CONCRETE	OR FULLY GROUTED	MASONRY WALL CO	NSTRUCTION (inch	es) <sup>a, b</sup>		
SNOW LOAD OR ROOF LIVE	STORY AND TYPE	LOAD-BEARING V (psf)	ALUE OF SOIL					
LOAD	OF STRUCTURE WITH CMU	1500	2000	2500	3000	3500	4000	
		TABLE R403.1(1)						
	MINIMUM WIDTH AND THICKNESS FOR	. ,			TION (inche	e)a, b		
		CONCILETOOTINGUT				3)		
SNOW LOAD	STORY AND TYPE	LOAD-BEAR	NG VALUE OF S	OIL				
OR ROOF LIVE	OF STRUCTURE WITH	(psf)						
LOAD	LIGHT FRAME	1500	2000	2500	3000	3500	4000	
	1 story—slab-on-grade	15 × 6	12 × 6	12 × 8	12×6	12 × 6	12×8	
	1 story-with crawl space	20 × 6	15 × 6	12 × 6	12 × 6	12 × 6	12 × 8	
		TABLE R403.1(2)						
							a h	
MI	NIMUM WIDTH AND THICKNESS FOR CONCRET	E FOOTINGS FOR LIGHT-F	RAME CONST	RUCTION WITH	H BRICK VEN	IEER (inches	) <sup>a, b</sup>	
NOW LOAD	STORY AND TYPE OF	LOAD-BEAR	NG VALUE OF S	OIL				
R ROOF LIVE	STRUCTURE	(psf)						
LOAD	WITH BRICK VENEER	1500	2000	2500	3000	3500	4000	
		1500	2000	2300	0000	0500	4000	
	2 story—slab-on-grade	27 × 8	20 × 6	16 × 6	13 × 6	12 × 6	12 × 8	
50 psf	2 story—with crawl space	32 × 11	24 × 7	19 × 6	18 × 8	14 × 8	12 × 6	
		TABLE R403.1(3)						
		( )						ava. b
MINIMUM WIDTH A	ND THICKNESS FOR CONCRETE FOOTINGS W	TH CAST-IN-PLACE CONC	RETE OR FULI	Y GROUTED	ASONRY W	ALL CONSTR	UCTION (inche	51-7-
MINIMUM WIDTH A		TH CAST-IN-PLACE CONC	RETE OR FULI	Y GROUTED N	MASONRY W	ALL CONSTR	UCTION (inche	(5)-/-
			RETE OR FULI		MASONRY W	ALL CONSTR	UCTION (inche	(5)-/-
MINIMUM WIDTH A SNOW LOAD OR ROOF LIVE	STORY AND TYPE				MASONRY W	ALL CONSTR	UCTION (inche	.5)-,
SNOW LOAD DR ROOF LIVE		LOAD-BEA (psf)	RING VALUE OF	SOIL				
SNOW LOAD DR ROOF LIVE	STORY AND TYPE	LOAD-BEA			MASONRY W			
SNOW LOAD DR ROOF LIVE	STORY AND TYPE	LOAD-BEA (psf)	RING VALUE OF	SOIL				
SNOW LOAD	STORY AND TYPE OF STRUCTURE WITH CMU	LOAD-BEA (psf) 1500	RING VALUE OF	SOIL 2500	3000	350	0 4000	

TABLE R403.1(3)

For SI: 1 inch = 25.4 mm, 1 plf = 14.6 N/m, 1 pound per square foot = 47.9 N/m<sup>2</sup>.

a. Interpolation allowed. Extrapolation is not allowed.

b. Based on 32-tool-wide house with load-bearing center wail that carries half of the tributary attic, and floor framing. For every 2 feet of adjustment to the width of the house add or subtract 2 inches of footing width and 1 inch of footing thickness (but not less than 6 inches thick).

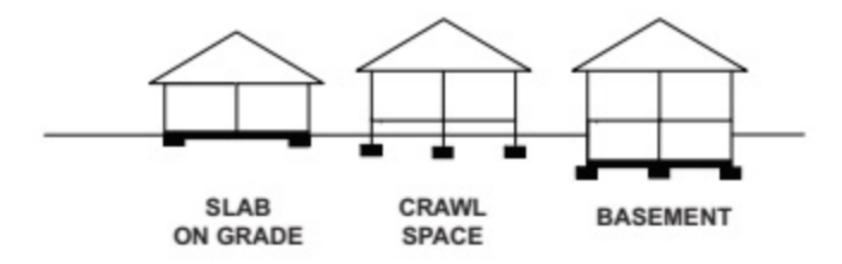


- Which chart?
  - CIP or Fully Grouted Masonry
- Example plans have ICF walls (Essentially CIP walls)
- Snow Load
  - MN Provisions 1303.1700
  - 60psf in Zone 1
  - 50psf in Zone 2
  - See Table 301.2(1) Roof snow load is 70% of ground snow load
    - $50 \times .7 = 35psf roof snow load$

TABLE R403.1(3) MINIMUM WIDTH AND THICKNESS FOR CONCRETE FOOTINGS									
SNOW LOAD	WITH CAST-IN-PLACE CONCRE STORY AND TYPE	TE OR FULLY	LLY GROUTED MASONRY WALL CONSTRUCTION (inches) <sup>4, b</sup> LOAD-BEARING VALUE OF SOIL (psf)						
LOAD	OF STRUCTURE WITH CMU	1500	2000	2500	3000	3500	4000		
	1 story-slab-on-grade	$14 \times 6$	$12 \times 6$	12 × 6	12 × 6	$12 \times 6$	12 × 6		
	1 story-with crawl space	19 × 6	$14 \times 6$	12 × 6	12 × 6	12 × 6	$12 \times 6$		
	1 story-plus basement	25 × 8	19 × 6	15 × 6	13 × 6	12 × 6	$12 \times 6$		
	2 story-slab-on-grade	23 × 7	18 × 6	14 × 6	12 × 6	12 × 6	12 × 6		
20 psf	2 story-with crawl space	29 × 9	22 × 6	17 × 6	$14 \times 6$	12 × 6	12 × 6		
5(	2 story-plus basement	35 × 12	$26 \times 8$	21 × 6	17 × 6	15 × 6	13 × 6		
	3 story-slab-on-grade	32×11	24 × 7	19 × 6	16 × 6	$14 \times 6$	12 × 6		
	3 story-with crawl space	38 × 14	28 × 9	23 × 6	19 × 6	16 × 6	$14 \times 6$		
	3 story-plus basement	43 × 17	33 × 11	$26 \times 8$	22 × 6	19 × 6	16 × 6		
	1 story-slab-on-grade	15 × 6	12 × 6	12 × 6	12 × 6	12 × 6	12 × 6		
	1 story-with crawl space	$20 \times 6$	15 × 6	12 × 6	12 × 6	12 × 6	$12 \times 6$		
	1 story-plus basement	$26 \times 8$	$20 \times 6$	16 × 6	13 × 6	12 × 6	12 × 6		
	2 story-slab-on-grade	24 × 7	$18 \times 6$	15 × 6	12 × 6	12 × 6	12 × 6		
30 psf	2 story-with crawl space	30 × 10	22 × 6	18 × 6	15 × 6	13 × 6	12 × 6		
3(	2 story-plus basement	36 × 13	27 × 8	21 × 6	18 × 6	15 × 6	13 × 6		
	3 story-slab-on-grade	33 × 12	25 × 7	$20 \times 6$	17 × 6	14 × 6	12 × 6		
	3 story-with crawl space	39 × 14	29 × 9	23 × 7	19 × 6	17 × 6	$14 \times 6$		
	3 story-plus basement	$44 \times 17$	33 × 12	27×8	22 × 6	19 × 6	17 × 6		
	1 story-slab-on-grade	17 × 6	13 × 6	12 × 6	12 × 6	12 × 6	12 × 6		
	1 story-with crawl space	22 × 6	17 × 6	13 × 6	12 × 6	12 × 6	$12 \times 6$		
	1 story-plus basement	28×9	21 × 6	17 × 6	14 × 6	$12 \times 6$	$12 \times 6$		
	2 story-slab-on-grade	$27 \times 8$	$20 \times 6$	16 × 6	13 × 6	12 × 6	$12 \times 6$		
50 psf	2 story-with crawl space	32 × 11	24 × 7	19 × 6	16 × 6	14 × 6	12 × 6		
Ň	2 story-plus basement	38 × 14	28 × 9	23 × 6	19 × 6	16 × 6	$14 \times 6$		
	3 story-slab-on-grade	35 × 13	27 × 8	21 × 6	18 × 6	15 × 6	13 × 6		
	3 story-with crawl space	41 × 15	31 × 10	24 × 7	$20 \times 6$	17 × 6	$15 \times 6$		
	3 story-plus basement	47 × 18	35 × 12	28 × 9	23 × 7	$20 \times 6$	$17 \times 6$		
		-							

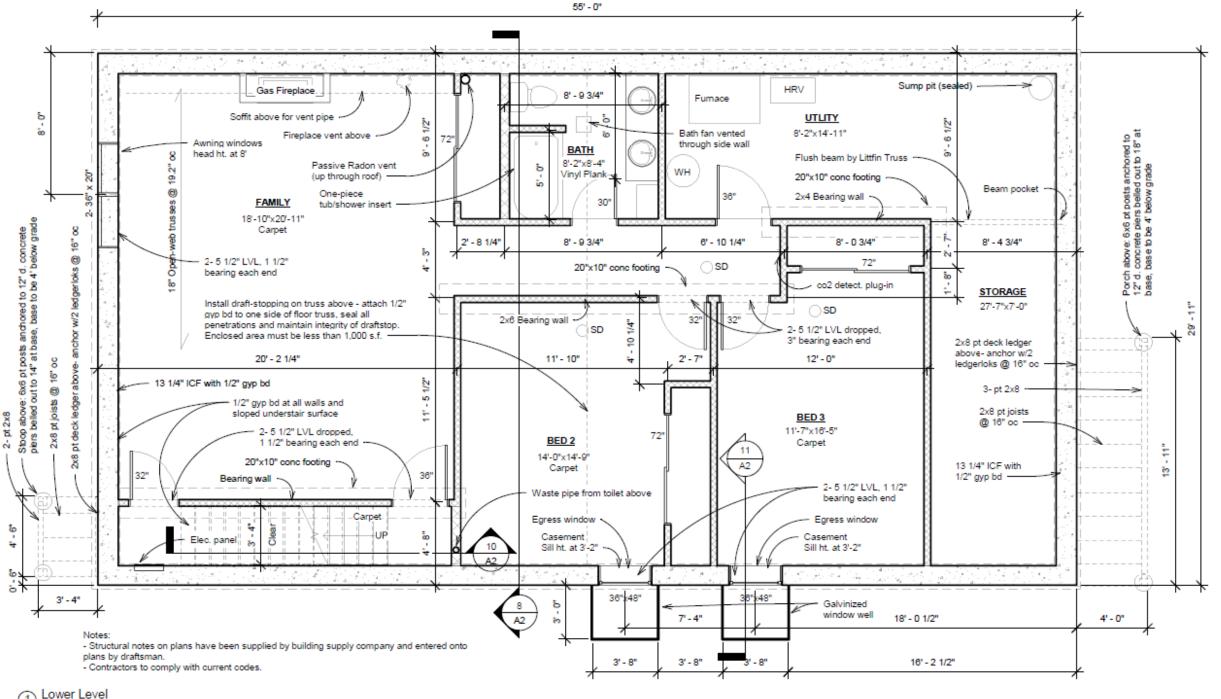
2020 MN Residential Code

- a. Interpolation allowed. Extrapolation is not allowed.
- b. Based on 32-foot-wide house with load-bearing center wall that carries half of the tributary attic, and floor framing. For every 2 feet of adjustment to the width of the house, add or subtract 2 inches of footing width and 1 inch of footing thickness (but not less than 6 inches thick).



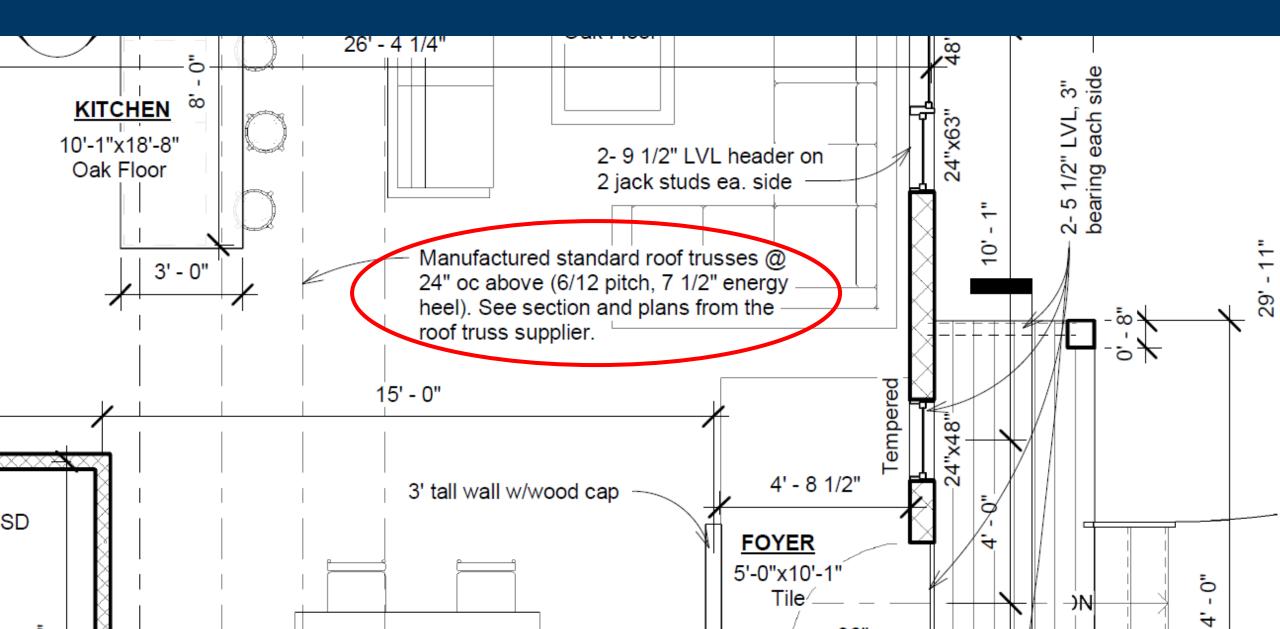
For Hand Framed Homes (Not for clear spanning roof or floor truss...)

- Footnote a: Interpolation allowed. Extrapolation is not allowed.
  - Look for values between 30psf 50psf.
- Footnote b: Based on 32' wide house with load-bearing center wall that carries half the tributary attic, and floor framing. For every 2' of adjustment to the width of the house add or subtract 2" of footing width and 1" of footing thickness (but not less than 6").
- Example house is 30' (29'-11" see A1)
  - Subtract 2" from minimum footing width reduced from 20-21" (based on Zone 1 or 2 snow load) to 18-19".
- The roof truss in our example house are clear spanning and bear solely on perimeter walls with no center bearing wall, so it doesn't fit the application of the table.

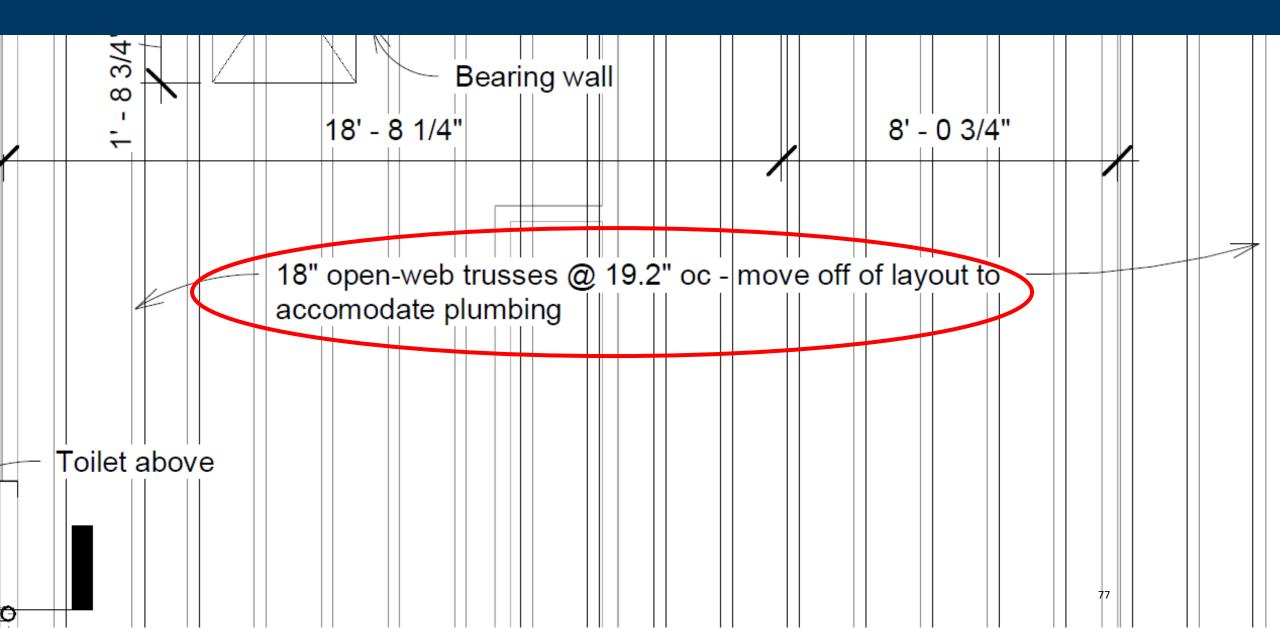


1/4" = 1'-0"

### What if it has Truss?



### What if it has Truss?



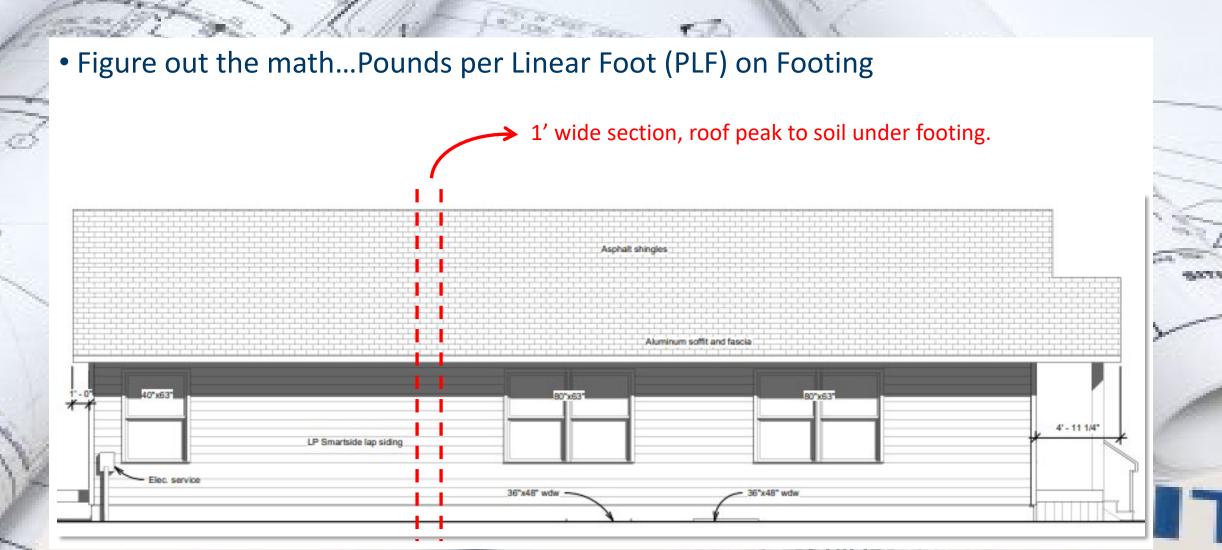
### Footings

- Figure out the math...Pounds per Linear Foot (PLF) on Footing
- Need the linear foot (LF) weight of:
  - Design roof load
  - Wall load, including cladding
  - Design floor load
  - Foundation weight
  - Footing weight (may be negligible)

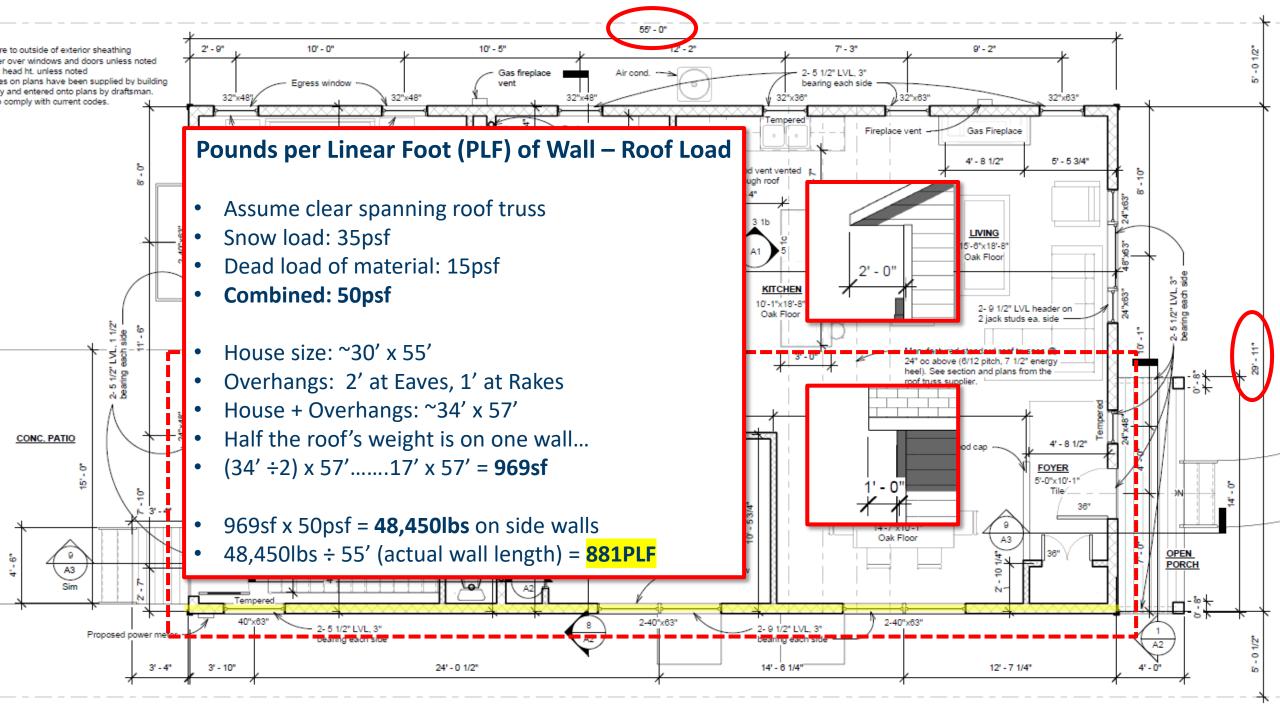


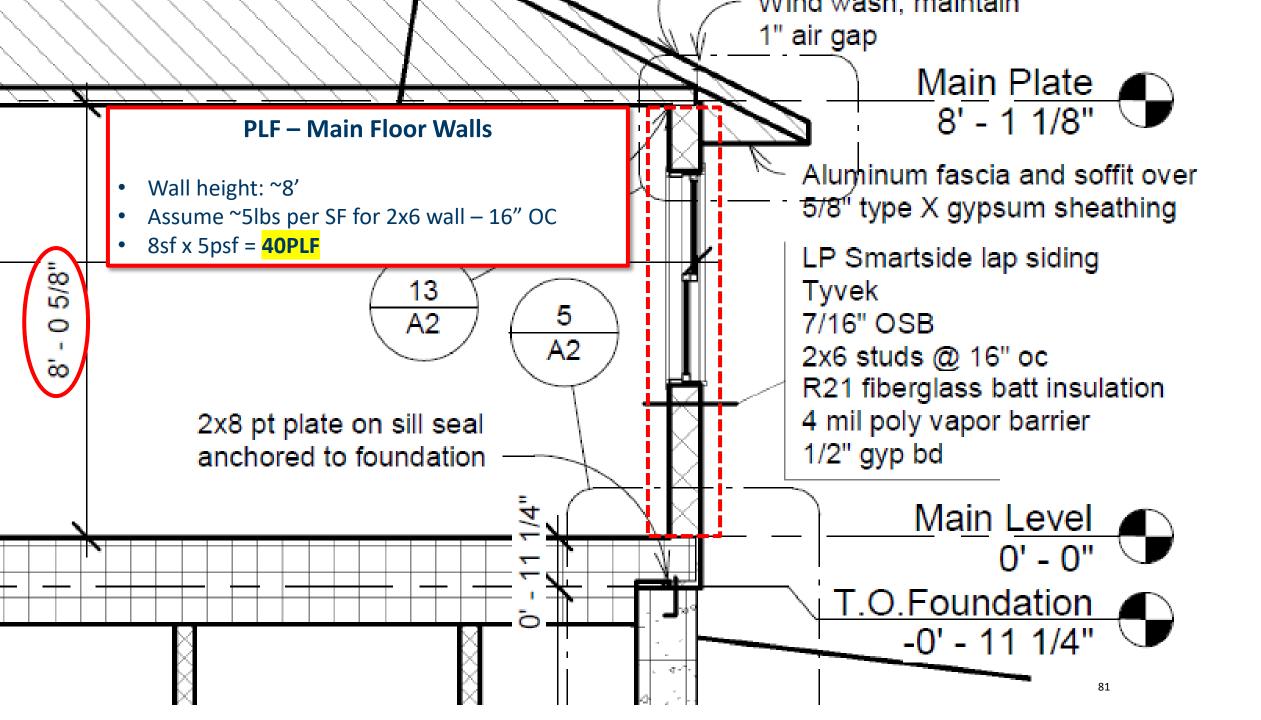
-- serine site of construction.

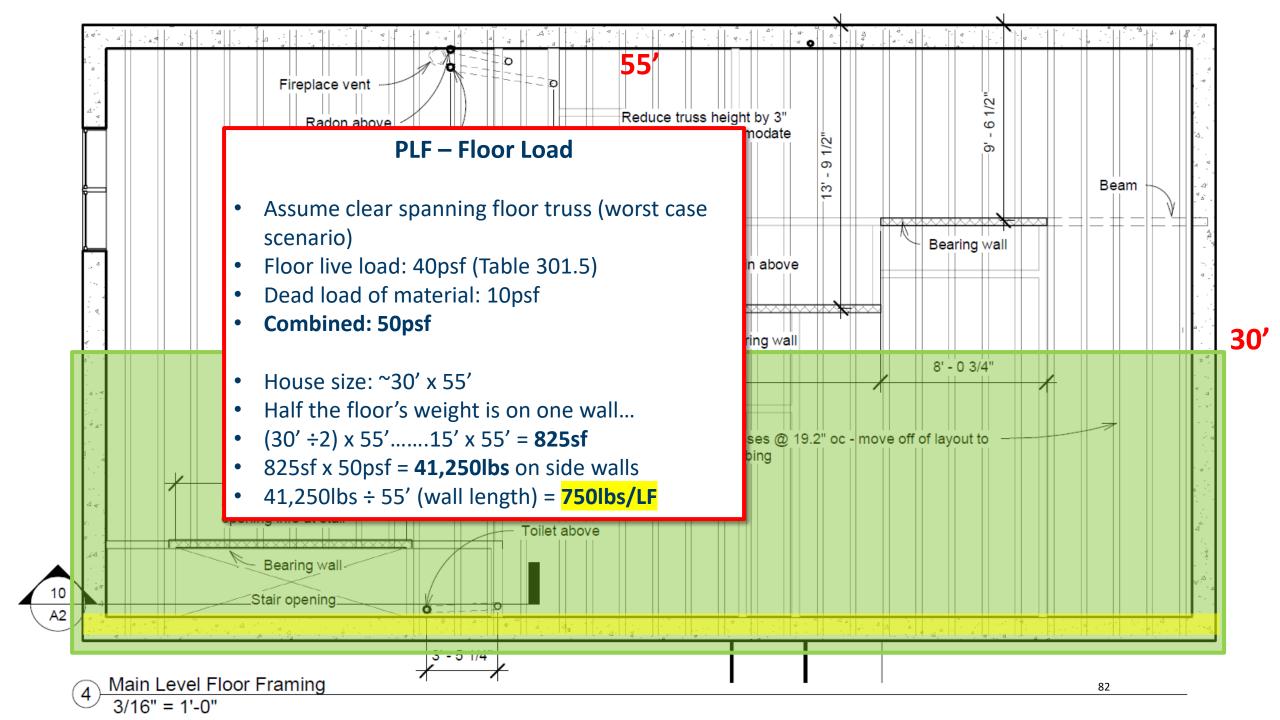
### Footings

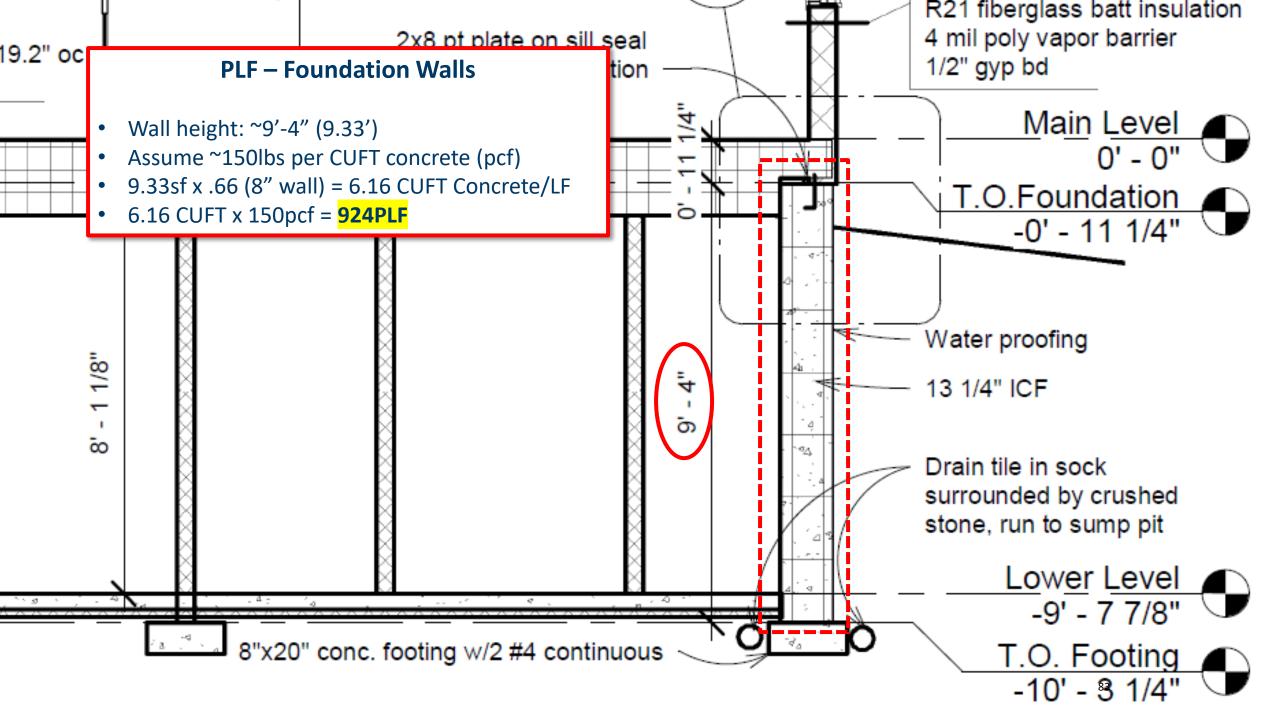


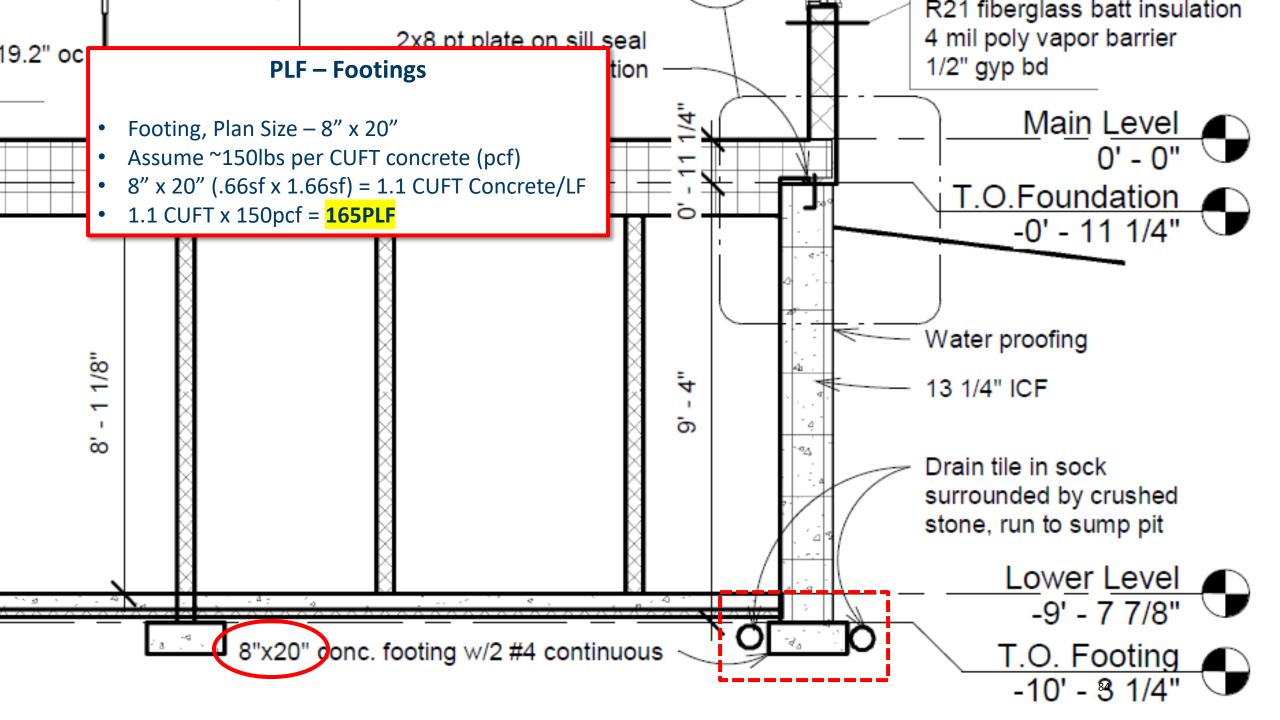
the site of construction.











### Add the Totals:

ATTACTOR TO ATTACTOR

- Figure out the math...
- Roof: 881PLF
- Main Floor Walls: 40PLF
- Main Floor: 750PLF
- Foundation Wall: 924PLF
- Footing: 165PLF
- TOTAL PLF: **2,760PLF**

- Figure out the math...
- TOTAL PLF: 2,760PLF
- Sizing the strip footing:
  - Divide the total weight on each linear foot by the soil bearing capacity to arrive at the minimum footing width in feet.
  - Must still check projections & depth.

- Figure out the math...
- TOTAL PLF: **<u>2,760PLF</u>**
- Formula: Total weight per LF ÷ Soil bearing capacity = Width of footing (in feet)
- Examples:
  - 1. Assumed soil bearing capacity 2,000lbs
    - 2,760 (total weight) ÷ 2,000 (soil capacity) = 1.38' wide footing. Convert feet to inches: 1.38 x 12 = 16.6"
    - Round up, minimum footing width based on calculation = <u>17</u>" wide
  - 2. Assumed soil bearing capacity 1,500lbs
    - 2,760 ÷ 1,500 = 1.84' wide footing. Convert feet to inches: 1.84 x 12 = 22.1"
    - Round up, minimum footing width based on calculation = <u>23</u>" wide

• Check projections & depth...Example #1:

- Assumed soil bearing capacity 2,000lbs
  - Plans indicate: 18" footing, 8" wall (do not count the foam on ICF wall as part of the wall thickness).
  - 18" 8"(*wall thickness*) = 10"
  - $10" \div 2 = 5"$  projections on each side.
  - Min footing thickness? Table 403.1(1,2,3) –
     "...not less than 6" thick."
  - Minimum 18" x 6" footing ok?
    - 5" projections are less than 6" thickness footing complies.



**R403.1.1 Minimum size.** The minimum width, W, and thickness, T, for concrete footings shall be in accordance with Tables R403.1(1) through R403.1(3) and Figure R403.1(1) or R403.1.3, as applicable. The footing width shall be based on the load-bearing value of the soil in accordance with Table R401.4.1. Footing projections, P, shall be not less than 2 inches (51 mm) and shall not exceed the thickness of the footing. Footing thickness and projection

PERN

insprenous place on the site of construction.

- Check projections & depth...Example #2:
- Assumed soil bearing capacity 1,500lbs
  - Plans indicate: 23" footing, 8" wall (do not count the foam on ICF wall as part of the wall thickness).
  - 23" 8" (wall thickness) = 15"
  - 15" ÷ 2 =  $7\frac{1}{2}$ " projections on each side.
  - Min thickness? Table 403.1(1,2,3) "...not less than 6" thick."
  - Minimum 23" x 6"
    - $7\frac{1}{2}$ " projections are *greater* than thickness does <u>NOT</u> comply unless thickness is increased.
  - 23" x 8" would comply.



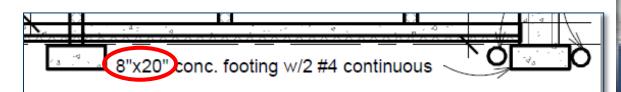
**R403.1.1 Minimum size.** The minimum width, W, and thickness, T, for concrete footings shall be in accordance with Tables R403.1(1) through R403.1(3) and Figure R403.1(1) or R403.1.3, as applicable. The footing width shall be based on the load-bearing value of the soil in accordance with Table R401.4.1. Footing projections, P, shall be not less than 2 inches (51 mm) and shall not exceed the thickness of the footing. Footing thickness and projection

PERM

insprenous place on the site of construction.

### Footings – Check Size

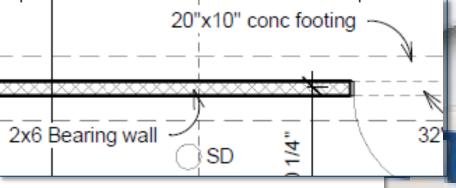
- Reference house drawings page A2.
- Exterior Walls perimeter strip footings
  - Page A2 plans indicate perimeter footings are 20" x 8" (w/ 2 #4 bar continuous)
  - 20" x 8" min based on math is ok for 2,000lb soil bearing capacity. (Math indicated 18" x 6" was minimum compliant footing)
- Check footing thickness & projections.
  - R403.1.1 Minimum Size
    - Footing projections ...shall not exceed the thickness of the footing.
  - 20" footing 8" wall = 12" remaining
  - $12'' \div 2 = 6''$
  - Projections of  $6^{"}$  is less than 8" thickness, so it complies.



## Footings

1. 2.

- Reference house drawings page A1
- Interior footings
  - Center bearing wall footings 20" x 10"
  - Width is ok based on previous exercise checking perimeter footings (20" min). Perimeter was carrying roof load, interior footings are only carrying floor load.
  - Thickness 10" is ok (6" was tabular minimum).
  - Check projections for worst situation (2x4 wall):
    - $20^{"}footing 3\frac{1}{2}^{"}(2x4 \ bearing \ wall) = 16\frac{1}{2}^{"}$
    - $16\frac{1}{2}^{"} \div 2 = 8\frac{1}{4}^{"}$  projections
    - Projection of  $8\frac{1}{4}$  is less than 10" thickness, so it complies.
    - The 2x6 bearing wall has a smaller projection, no need to check.



20"x10" conc footing

8' - 0 3/4

2x4 Bearing wall

### Footings – Pads/Point Loads

1.23

- Sizing pads/point load footing
  - Figure out the loading on the post or column
  - Formula Area x Floor Load
    - L x W = Area
    - Area x Floor Load (40psf living, 30 sleeping) = Total Load (TL)
    - (More detail later in presentation.)

#### TABLE R301.5 MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (in pounds per square foot)

USE	LIVE LOAD
Uninhabitable attics without storage <sup>b</sup>	10
Uninhabitable attics with limited storage <sup>b, g</sup>	20
Habitable attics and attics served with fixed stairs	30
Balconies (exterior) and decks <sup>e</sup>	40
Fire escapes	40
Guards and handrails <sup>d</sup>	200 <sup>h</sup>
Guard in-fill components <sup>f</sup>	50 <sup>h</sup>
Passenger vehicle garages <sup>a</sup>	50 <sup>a</sup>
Rooms other than sleeping rooms	40
Sleeping rooms	30
Stairs	40 <sup>c</sup>

For SI: 1 pound per square foot = 0.0479 kPa, 1 square inch =  $645 \text{ mm}^2$ , 1 pound = 4.45 N.

a. Elevated garage floors shall be capable of supporting a 2,000-pound load applied over a 20-square-inch area.

conspicuous place on the site of construction.

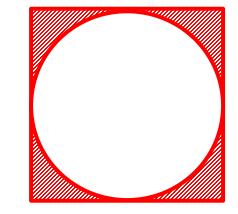
### Footings – Sizing Pads/Point Loads

• Formulas

$$\int \left(\frac{TL}{Soil\ Capacity}\right) x\ 144 =$$
Square Footing Size

- Square Footing Size × 1.13 = Round Footing Size
- Example 10,000lb load on a column, assumed 2000lb soil capacity.
  - $\frac{10,000}{2000} \times 144 = 720$
  - $\sqrt{720} = 26.83$  or 27" x 27" square footing.
  - 26.83 x 1.13 = 30.32 or 30" diameter **round footing.**

in a conspicuous place on the site of const



### Footings – Pads/Point Loads

- How thick?
- R403.1.1 Minimum Size
  - Footing projections ...shall not exceed the thickness of the footing.
- Back to example
  - 10,000lb load on a column, assumed 2000lb soil capacity = 27" x 27" square footing

inspicuous place on the

- $27 5\frac{1}{2}$  (column size) =  $21\frac{1}{2}$
- $21\frac{1}{2} \div 2 = 10\frac{3}{4}$
- So, pad footing must be 11" thick min.

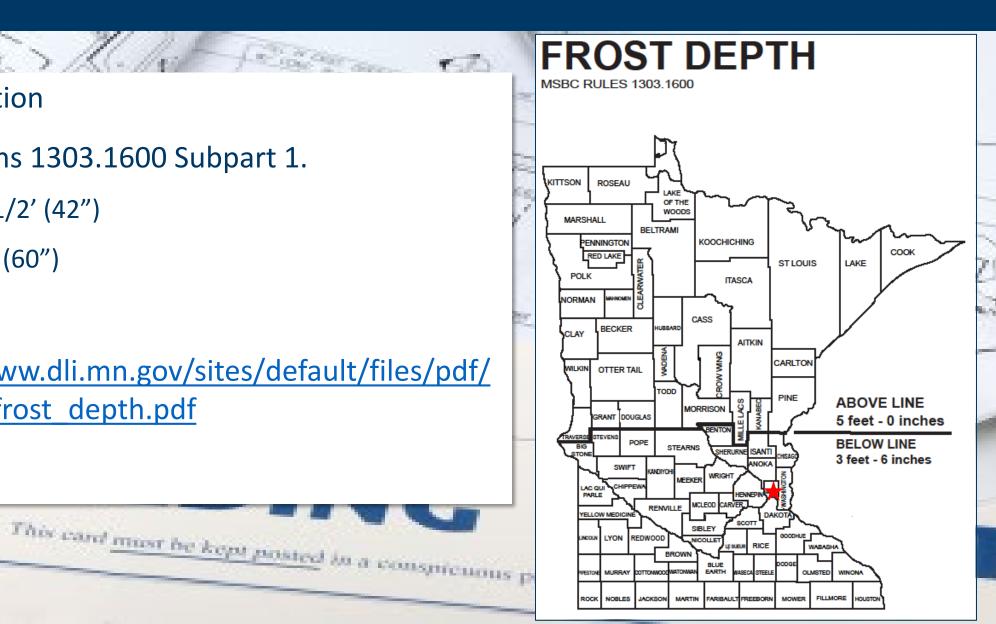
### Footing Depth

P. P.

- Frost protection
- MN Provisions 1303.1600 Subpart 1.

ming Alla

- Zone I 3-1/2' (42")
- Zone II 5' (60")
- DLI Website:
  - https://www.dli.mn.gov/sites/default/files/pdf/ bc map frost depth.pdf



### Footing Depth

- House Plans A2
  - A2 Section 1 full depth basement complies.
- Garage Plans A2 Section 3
  - Slab on grade.
  - R403.1.4 Minimum Depth
  - Exterior footings shall be placed not less than 12" below the undisturbed ground surface.

This card must be kept posted in a

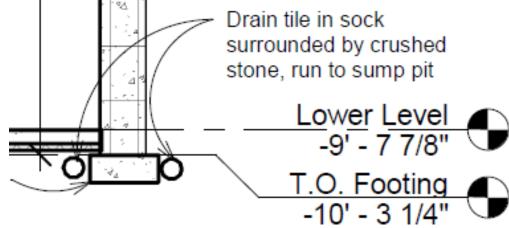
• – 12" below grade – complies.

R403.1.4 Minimum depth. Exterior footings shall be placed not less than 12 inches (305 mm) below the undisturbed ground surface. Where applicable, the depth of footings shall also conform to Sections R403.1.4.1 through R403.1.4.2. Steel b ap siding oc oc

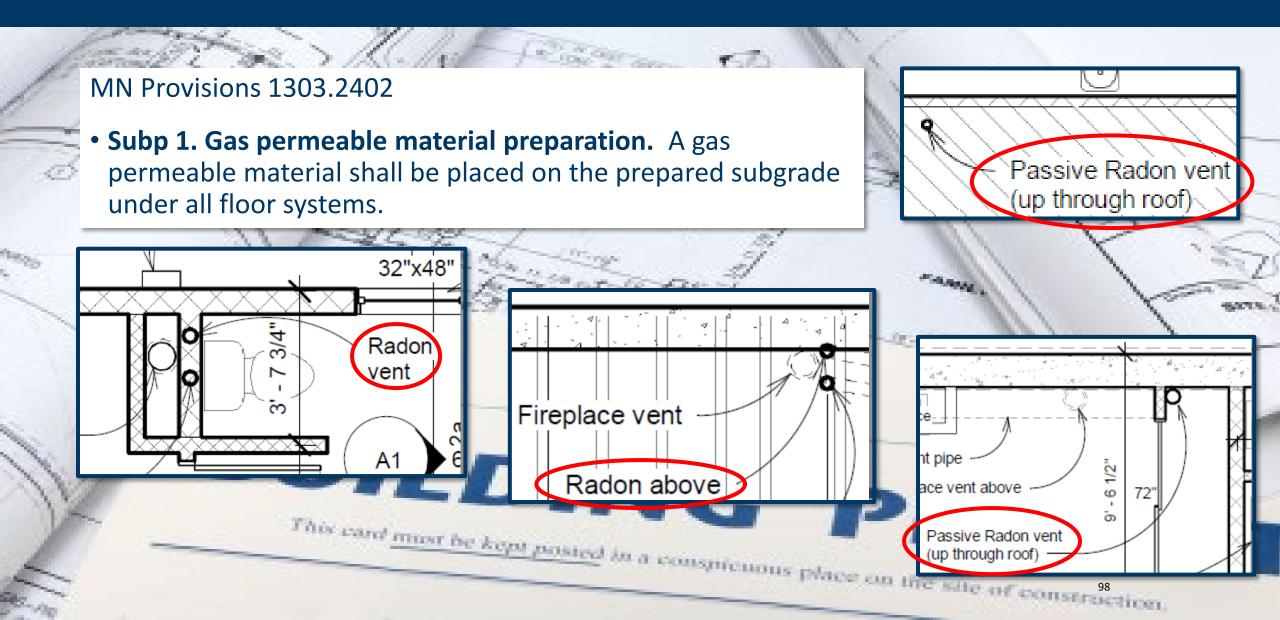
### Footings

### • Drain tile

- **R405.1 Concrete or masonry foundations.** Drains shall be provided around concrete or masonry foundations that retain earth and enclose habitable or usable spaces located below *grade*. Drainage tiles, gravel or crushed stone drains, perforated pipe or other *approved* means or systems shall be installed at or below the top of the footing or below the bottom of the slab and shall discharge by gravity or mechanical means into an *approved* drainage system.
  - Exception: A drainage system is not required where the foundation is installed on well-drained soil ground or sand gravel mixture soils according to the unified soil classification system, group I soils, as detailed in table R 405.1.
- A2 Section 1



### Radon



### Other Helpful Resources...

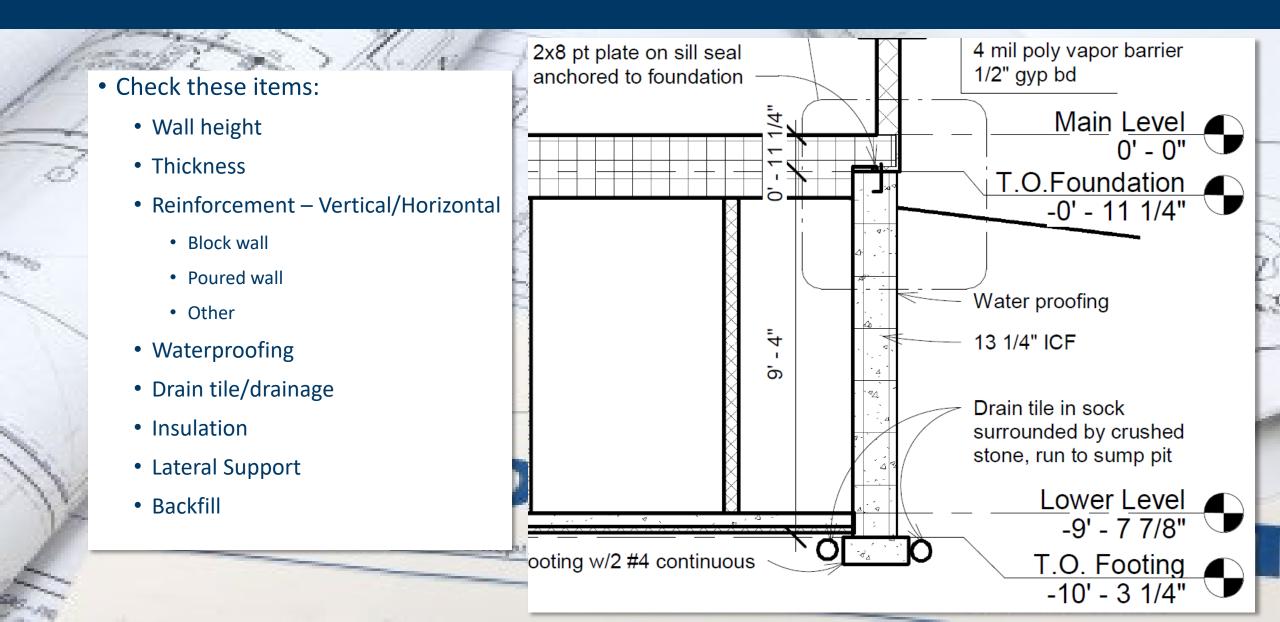
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	ices are <u>closed to walk-in customers</u> .
	For business > Manufactured structures > Formulas, maps and tables
APPRENTICESHIP AND DUAL TRAINING	FORMULAS, MAPS AND TABLES
CODES AND LAWS	<ul> <li>Formulas for determining footing spacing and size</li> </ul>
ELECTRICAL CONTRACTORS	<ul> <li>Example: Formulas for determining footing space and size</li> </ul>
ELEVATOR CONTRACTORS	<ul> <li>Snow load map - ground load and roof load map per Minnesota Rules 1303.1700</li> </ul>
EMPLOYMENT PRACTICES	Frost depth map - Minnesota Rules 1303.1600
HIGH PRESSURE PIPING	<ul> <li>Round footing sizing table</li> </ul>
CONTRACTORS	<ul> <li>Soil classification and bearing table</li> </ul>
INDEPENDENT CONTRACTOR	Square footing sizing table
REGISTRATION	Miscellaneous information
LICENSE FORMS, PERMITS,	<ul> <li>Egress windows for manufactured homes Jan. 2, 2008</li> </ul>
PLAN REVIEW AND LOCAL	Code verification sign-off

STRUCTURES



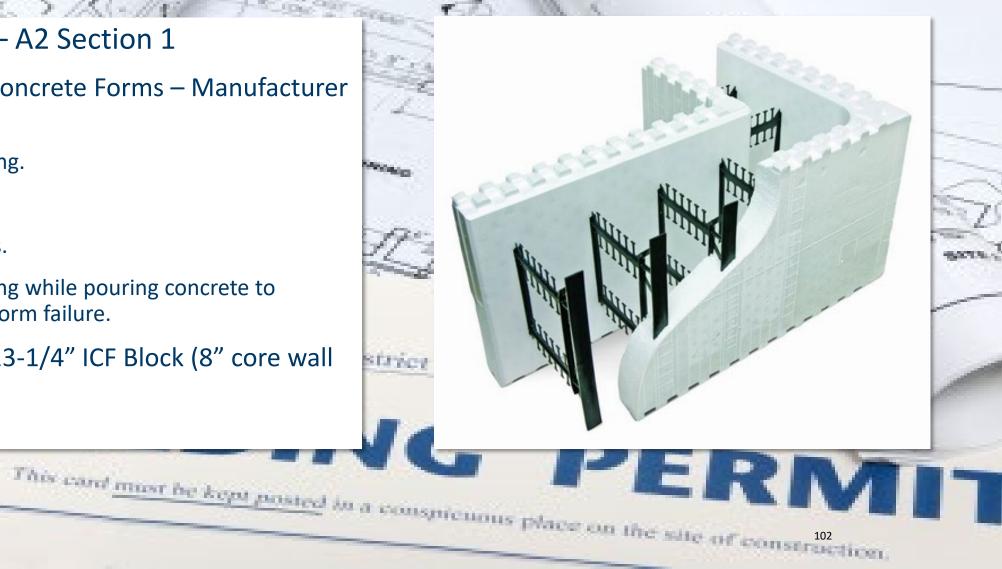
# Foundation Wall – Chapter 4

### **Foundation Wall**



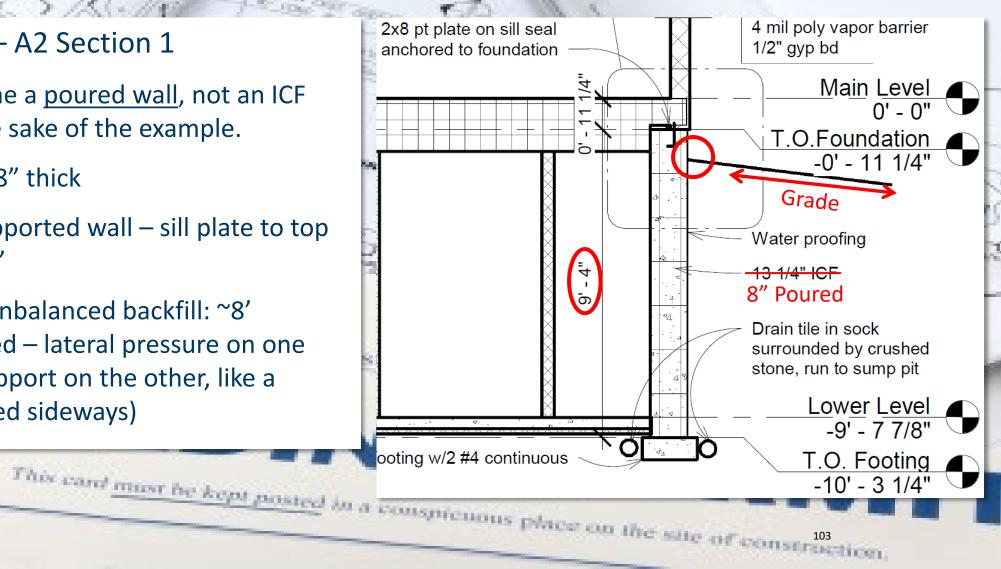
### **Foundation Wall**

- Wall height A2 Section 1
  - Insulated Concrete Forms Manufacturer Specs.
    - Reinforcing.
    - Lintels.
    - Openings.
    - Reinforcing while pouring concrete to prevent form failure.
  - 9-4" wall, 13-1/4" ICF Block (8" core wall thickness).



### Foundation Wall Reinforcement

- Wall height A2 Section 1
  - Let's assume a poured wall, not an ICF wall for the sake of the example.
  - 9'-4" wall, 8" thick
  - Total unsupported wall sill plate to top of floor: ~9'
  - Height of unbalanced backfill: ~8' (Unbalanced – lateral pressure on one side, no support on the other, like a beam turned sideways)



### Foundation Wall – Vertical Reinforcement

la a	TABLE R404.1.2(3) MINIMUM VERTICAL REINFORCEMENT FOR <u>8-INCH (</u> 203 mm) NOMINAL <u>FLAT CONCRETE</u> BASEMENT WALLS <sup>b, c, d, e, f, h, i, j</sup>				
23			MINIMUM VERTICAL REINFORCEMENT-BAR SIZE AND SPACING (inches) Soil classes <sup>a</sup> and design lateral soil (psf per foot of depth)		
14	MAXIMUM UNSUPPORTED WALL HEIGHT (feet)	MAXIMUM UNBALANCED BACKFILL HEIGHT <sup>®</sup> (feet)			
~			GW, GP, SW, SP 30	GM, GC, SM, SM-SC and ML 45	SC, ML-CL and inorganic Cl 60
~		4	NR	NR	NR
		5	NR	NR	NR
	8	6	NR	NR	6 @ 37
		7	NR	6 @ 36	6 @ 35
		8	6 @ 41	6 @ 35	6 @ 26
	9	4	NR	NR	NR
		5	NR	NR	NR
		6	NR	NR	6 @ 35
X		7	NR	6 @ 35	6 @ 32
1		8	6 @ 36	6 @ 32	6 @ 23
1		9	6 @ 35	6 @ 25	6 @ 18
L	10	4	NR	NR	NR
9		5	NR	NR	NR
		6	NR	NR	6 @ 35
		7	NR	6 @ 35	6 @ 29
		8	6 @ 35	6 @ 29	6 @ 21
		9	6 @ 34	6 @ 22	6 @ 16
		10	6 @ 27	6 @ 17	6 @ 13

### Table Footnotes

For SI: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 pound per square foot per foot = 0.1571 kPa<sup>2</sup>/m, 1 pound per square inch = 6.895 kPa. NR = Not Required.

- a. Soil classes are in accordance with the Unified Soil Classification System. Refer to Table R405.1.
- b. Table values are based on reinforcing bars with a minimum yield strength of 60,000 psi, concrete with a minimum specified compressive strength of 2,500 psi and vertical reinforcement being located at the centerline of the wall. See Section R404.1.3.3.7.2.
- c. Vertical reinforcement with a yield strength of less than 60,000 psi and bars of a different size than specified in the table are permitted in accordance with Section R404.1.3.3.7.6 and Table R404.1.2(9).
- d. NR indicates vertical reinforcement is not required.
- e. Deflection criterion is L/240, where L is the height of the basement wall in inches.
- f. Interpolation is not permitted.
- g. Where walls will retain 4 feet or more of unbalanced backfill, they shall be laterally supported at the top and bottom before backfilling.
- h. See Section R404.1.3.2 for minimum reinforcement required for basement walls supporting above-grade concrete walls.
- i. See Table R608.3 for tolerance from nominal thickness permitted for flat walls.
- j. The use of this table shall be prohibited for soil classifications not shown.



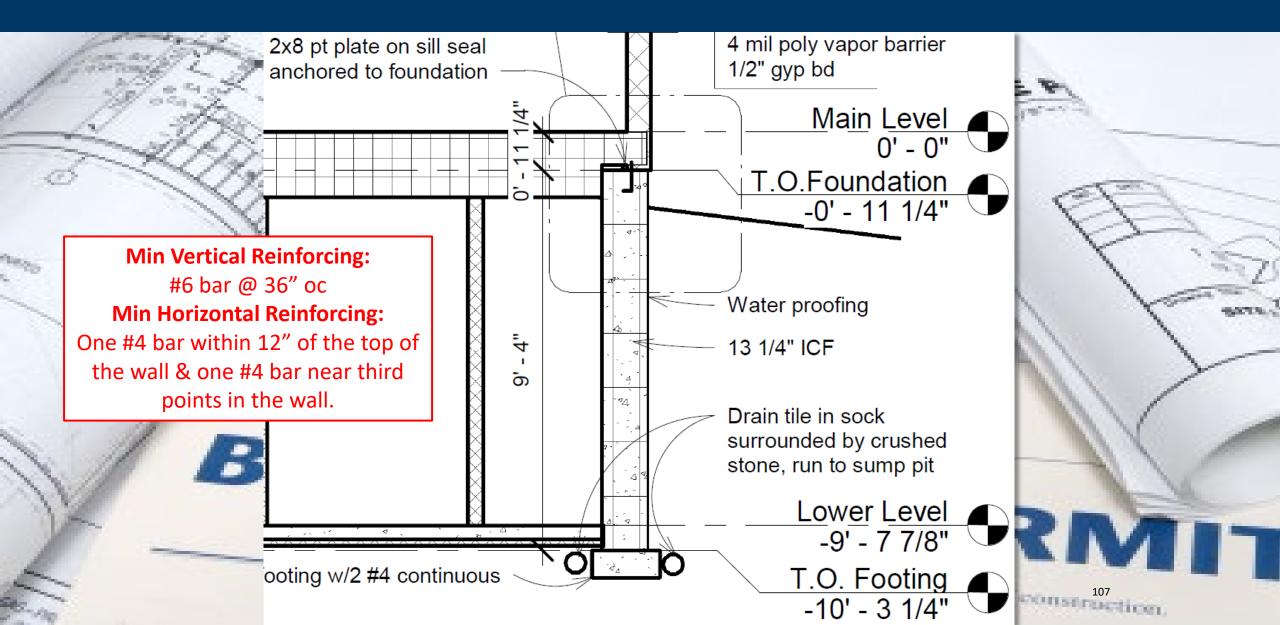
### Foundation Wall – Horizontal Reinforcement

TABLE R404.1.2(1) MINIMUM HORIZONTAL REINFORCEMENT FOR CONCRETE BASEMENT WALLS <sup>a,b</sup>						
MAXIMUM UNSUPPORTED HEIGHT OF BASEMENT WALL (feet)	LOCATION OF HORIZONTAL REINFORCEMENT					
≤ 8	One No. 4 bar within 12 inches of the top of the wall story and one No. 4 bar near mid-height of the wall story.					
> 8	One No. 4 bar within 12 inches of the top of the wall story and one No. 4 bar near third points in the wall story.					
	= 304.8 mm, 1 pound per square inch = 6.895 kPa.					

- a. Horizontal reinforcement requirements are for reinforcing bars with a minimum yield strength of 40,000 psi and concrete with a minimum concrete compressive strength of 2,500 psi.
- b. See Section R404.1.3.2 for minimum reinforcement required for foundation walls supporting above-grade concrete walls.

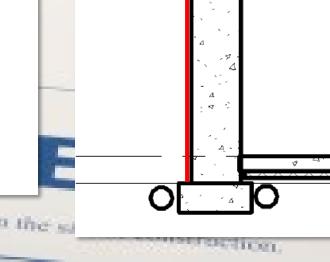


### Foundation Wall Reinforcing



• **R406.2 Concrete and masonry foundation waterproofing.** Exterior foundation walls that retain earth and enclose below grade interior spaces, floors, and crawl spaces shall be waterproofed. Waterproofing shall be installed at a minimum from the top of the footing to the finished grade or in accordance with the manufacturer's installation instructions. Walls shall be waterproofed in accordance with one of the following:

- 1. Two-ply hot-mopped felts.
- 2. Fifty-five-pound (25 kg) roll roofing.
- 3. Six-mil (0.15 mm) polyvinyl chloride.
- 4. Six-mil (0.15 mm) polyethylene.
- 5. Forty-mil (1 mm) polymer-modified asphalt.
- 6. Sixty-mil (1.5 mm) flexible polymer cement.
- 7. One-eighth-inch (3 mm) cement-based, fiber-reinforced, waterproof coating.
- 8. Sixty-mil (1.5 mm) solvent-free liquid-applied synthetic rubber.



• **R406.2 Concrete and masonry foundation waterproofing.** Exterior foundation walls that retain earth and enclose below grade interior spaces, floors, and crawl spaces shall be waterproofed. Waterproofing shall be installed at a minimum from the top of the footing to the <u>finished grade</u> or in accordance with the manufacturer's installation instructions. Walls shall be waterproofed in accordance with one of the following:

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- 8. Sixty-mil (1.5 mm) solvent-free liquid-applied synthetic rubber.

nous place on the si

• **R406.2 Concrete and** foundation walls that real floors, and crawl space installed at a minimum in accordance with the be waterproofed in acc

- 1. Two-ply hot-moppe
- 2. Fifty-five-pound (25
- 3. Six-mil (0.15 mm) p
- 4. Six-mil (0.15 mm) p
- 5. Forty-mil (1 mm) pc
- 6. Sixty-mil (1.5 mm) f
- 7. One-eighth-inch (3 coating.
- 8. Sixty-mil (1.5 mm) s

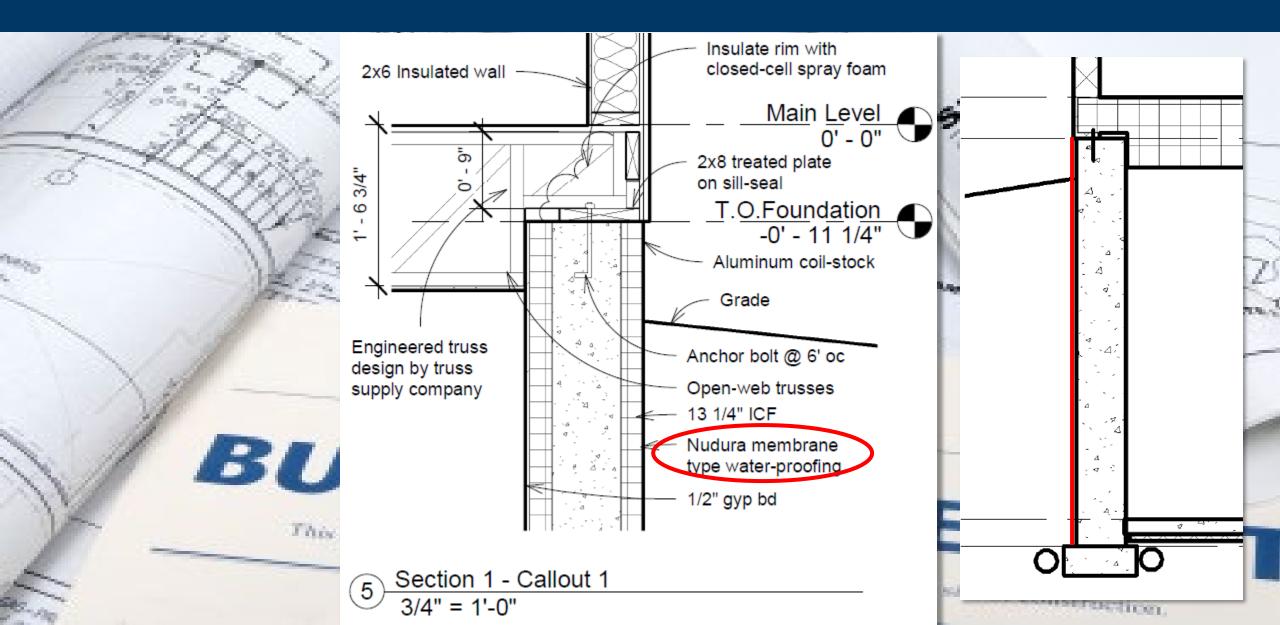
#### SECTION R402 BUILDING THERMAL ENVELOPE

foundation walls that real **R402.1 General (Prescriptive).** The *building thermal envelope* floors, and crawl space shall meet the requirements of Sections R402.1.1 through R402.1.4.

**R402.1.1 Insulation, waterproofing, and fenestration criteria.** The building thermal envelope shall meet the requirements of Table R402.1.1 based on the climate zone specified in Chapter 3, and the requirements contained in Section R402.2. Cast-in-place concrete and masonry block foundation walls shall be waterproofed according to IRC Section R406 and the following requirements:

- 1. The waterproofing shall extend from the top interior wall edge, across the top of the wall, and down the exterior wall face to the top of the footing. If a full width, closed-cell material is installed to create a seal between the sill plate and the top of the foundation wall, the installation is deemed to meet the requirements for the top of the wall waterproofing.
- 2. If the walls are exposed to the exterior environment, the waterproofing system shall have a rigid, opaque,

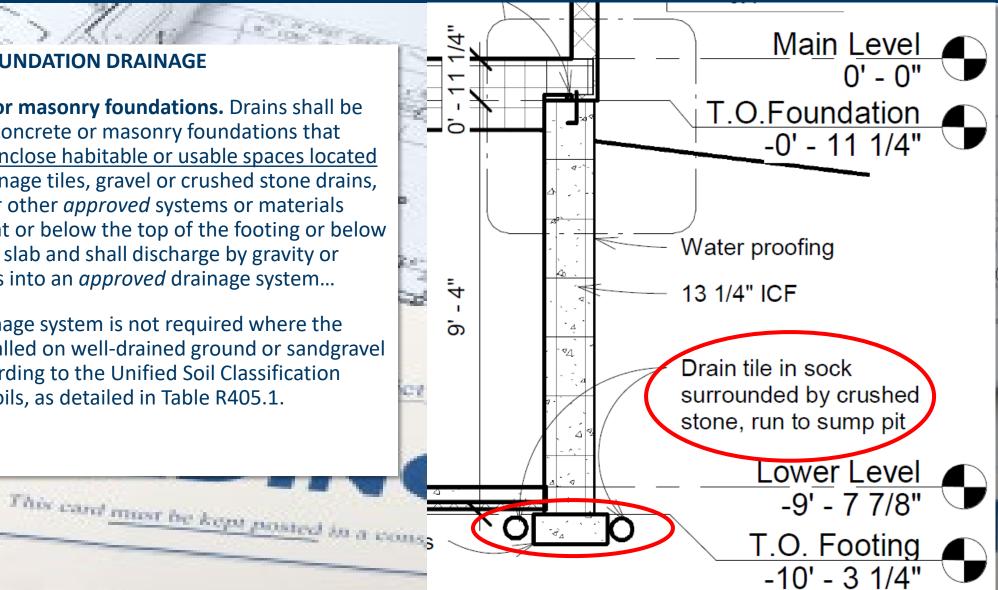
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### Foundation Wall Drainage

#### **SECTION R405 FOUNDATION DRAINAGE**

- R405.1 Concrete or masonry foundations. Drains shall be provided around concrete or masonry foundations that retain earth and enclose habitable or usable spaces located below grade. Drainage tiles, gravel or crushed stone drains, perforated pipe or other *approved* systems or materials shall be installed at or below the top of the footing or below the bottom of the slab and shall discharge by gravity or mechanical means into an *approved* drainage system...
- Exception: A drainage system is not required where the foundation is installed on well-drained ground or sandgravel mixture soils according to the Unified Soil Classification System, Group I soils, as detailed in Table R405.1.



## Foundation Wall Insulation

#### Residential Energy Code

	TABLE R402.1.1 INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT <sup>a</sup>											
CLIMATE ZONE	FENESTRATION U-FACTOR <sup>b</sup>	SKYLIGHT <sup>b</sup> <i>U</i> -FACTOR	GLAZED FENESTRATION SHGC <sup>b,e</sup>	CEILING <sup>i</sup> <i>R</i> -VALUE	WOOD FRAME WALL <i>R</i> -VALUE <sup>f</sup>	MASS WALL <i>R-</i> VALUE <sup>i,g,h</sup>	FLOOP <i>R</i> -VALUE	BASEMENT <sup>c, i</sup> WALL <i>R</i> -VALUE	SLAB <sup>d</sup> R-VALUE & DEPTH	CRAWL SPACE <sup>c, i</sup> WALL <i>R</i> -VALUE		
6	0.32	0.55	NR	49	20, 13+5	15/20	30 <sup>e</sup>	15	10, 3.5 ft	15		
7	0.32	0.55	NR	49	21	19/21	38 <sup>e</sup>	15	10, 5 ft	15		

For SI: 1 foot = 304.8 mm.

a. R-values are minimums. U-factors and SHGC are maximums. When insulation is installed in a cavity that is less than the label or design thickness of the insulation, the installed R-value of the insulation shall not be less than the R-value specified in the table.

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b. The fenestration U-factor column excludes skylights. The SHGC column applies to all glazed fenestration.

c. See Section R402.2.8.

d. Insulation R-values for heated slabs shall be installed to the depth indicated or to the top of the footing, whichever is less.

e. Or insulation sufficient to fill the framing cavity, R-19 minimum.

f. First value is cavity insulation, second is continuous insulation or insulated siding, so "13+5" means R-13 cavity insulation plus R-5 continuous insulation or insulated siding. If structural sheathing covers 40 percent or less of the exterior, continuous insulation R-value shall be permitted to be reduced by no more than R-3 in the locations where structural sheathing is used to maintain a consistent total sheathing thickness.

g. The second R-value applies when more than half the insulation is on the interior of the mass wall.

h. When using log-type construction for thermal mass walls the following applies:

(1) a minimum of a 7-inch diameter log shall be used; and

(2) the U-value of fenestration products shall be 0.29 overall on average or better.

i. See Section 402.2.8. A minimum R-19 cavity insulation is required in wood foundation walls.

j. Roof/ceiling assemblies shall have a minimum 6-inch energy heel.

#### Foundation Wall Insulation

- **R402.2.8 Basement walls.** Walls associated with conditioned basements shall be insulated from the top of the basement wall down to 10 feet below grade or to the top of the footing, whichever is less. Foundation insulation shall be installed according to the manufacturer's installation instructions. Walls associated with unconditioned basements shall meet the requirements of this section unless the floor overhead is insulated in accordance with Sections R402.1.1 and R402.2.7 and the following requirements:
  - a. R-15 insulation for concrete and masonry foundations shall be installed according to R402.I.I.1 to R402.I.I.8 and a minimum of a R-10 shall be installed on the exterior of the wall. Interior insulation, other than closed cell spray foam, shall not exceed R-11. Foundations shall be waterproofed in accordance with the applicable provisions of the International Residential Code (IRC).
    - Exception: R-10 continuous insulation on the exterior of each foundation wall shall be permitted to comply with this code if the tested air leakage rate required in Section R402.4.1.2 does not exceed 2.6 air changes per hour and the total square feet between the finished grade and the top of each foundation wall does not exceed 1.5 multiplied by the total lineal feet of each foundation wall that encloses conditioned space. Interior insulation, other than closed cell spray foam, shall not exceed R-11. See footnote c to Table R402.2.I.

conspicuous place on the site of construction

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## Foundation Wall Insulation

1. C.

- ICF Foundation wall has insulation incorporated into itself. See manufacturer specifications.
- For example, let's assume an 8" poured or block wall...
- 9'-4" from top of footing (tof) to sill plate.
- Insulation may be draining or nondraining insulation.

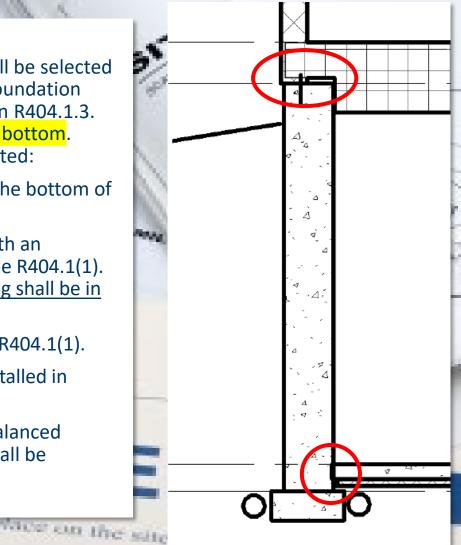
BUILDING

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## Foundation Wall Lateral Support

#### SECTION R404 FOUNDATION AND RETAINING WALLS

- **R404.1 Concrete and masonry foundation walls.** Concrete foundation walls shall be selected and constructed in accordance with the provisions of Section R404.1.2. Masonry foundation walls shall be selected and constructed in accordance with the provisions of Section R404.1.3. Concrete and masonry foundation walls shall be laterally supported at the top and bottom. Foundation walls that meet all of the following shall be considered laterally supported:
- 1. Full basement <u>floor shall be 3.5 inches</u> thick concrete slab poured tight against the bottom of the foundation wall.
- Floor joists and blocking shall be connected to the sill plate at the top of wall with an approved connector with listed capacity meeting the top of wall reaction in Table R404.1(1). <u>Maximum spacing of floor joists shall be 24 inches on center.</u> Spacing of blocking shall be in accordance with Table R404.1(1).
- 3. <u>Bolt spacing for the sill plate shall be no greater than the requirements in Table R404.1(1).</u>
- 4. <u>The floor shall be blocked perpendicular to the floor joists</u>. Blocking shall be installed in accordance with footnote f of Table R404.1(1).
- **Exception:** Cantilevered concrete and masonry foundation walls supporting unbalanced backfill that do not have permanent lateral support at the top of the foundation shall be constructed according to 404.1.1(5), Table R404.1.1(6), or TableR404.1.1(7).



## Foundation Wall Lateral Support

MAX. WALL HEIGHTMAX. UNBALANCED BACKFILL HEIGHTSOL $8'-0''$ $GW, GW, GW, GW, GW, GW, GW, GW, GW, GW, $	BOLT AND BLOCKING SF	SOIL LOAD	SUPPORTED			,	$\sim$	
MAX. WALL HEIGHTUNBALANCED BACKFILL HEIGHTSOL $8'-0''$ $GW, GW, GW, GW, GW, GW, GW, GW, GW, GW, $		SOIL LOAD	TODOEWALL			T		
8'-0'' $7'-6''$ $GM, GC, S$ $9'-0''$ $8'-6''$ $GW, G$ $9'-0''$ $8'-6''$ $GM, GC, S$ $10'-0''$ $9'-6''$ $GM, GC, S$ $10'-0''$ $9'-6''$ $GM, GC, S$ $SC, MH,$ $GW, G$ $10'-0''$ $9'-6''$ $GM, GC, S$ $SC, MH,$		(pcf/ft)	REACTION (plf) <sup>e</sup>	<sup>1</sup> / <sub>2</sub> " DIAMETER ANCHOR BOLT SPACING (inches) <sup>b, c, d</sup>	SPACING OF BLOCKING PERPENDICULAR TO FLOOR JOISTS (inches) <sup>f</sup>	9		
SC, MH, $9'-0''$ $8'-6''$ $GW, 0$ $9'-0''$ $8'-6''$ $GM, GC, S$ $10'-0''$ $9'-6''$ $GM, GC, S$ $10'-0''$ $9'-6''$ $GM, GC, S$ $SC, MH,$ $GW, 0$ $10'-0''$ $9'-6''$ $GM, GC, S$ $SC, MH,$ $SC, MH,$ For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.         a. Soil classes are in accordance with the Unified	GP, SW, & SP	30	260	72	72	N. 5		
9'-0'' $8'-6''$ $GW, GW, GW, GW, GW, GW, GW, GW, GW, GW,$	SM, SM-SC, & ML	45	400	72	72	11		-
9'-0"         8'-6"         GM, GC, S           10'-0"         9'-6"         GW, G           10'-0"         9'-6"         GM, GC, S           SC, MH,         SC, MH,           SC, MH,         SC, MH,           For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.         a. Soil classes are in accordance with the Unified	I, ML-CL, & I-CL	60	530	48	48	M.C.		-
$\begin{array}{c c} & & & & \\ & & & \\ & & & \\ \hline & & & \\ 10'-0'' & 9'-6'' & & \\ & & & \\ & & & \\ \hline & & & \\ & & \\ \hline & & & \\ & & \\ \hline & & \\ \hline & & \\ & & \\ \hline \\ \hline$	GP, SW, & SP	30	340	72	72		ά. ΈΔ.	
10'-0'' $9'-6''$ $GW, GW, GW, GW, GW, GW, GW, GW, GW, GW,$	SM, SM-SC, & ML	45	510	48	48	ran	í à	
10'-0"         9'-6"         GM, GC, S           SC, MH,         SC, MH,           For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.           a. Soil classes are in accordance with the Unified	I, ML-CL, & I-CL	60	680	32	32		· • ·	8
SC, MH,         For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.         a. Soil classes are in accordance with the Unified	GP, SW, & SP	30	430	64	64			
For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm. a. Soil classes are in accordance with the Unified	SM, SM-SC, & ML	45	640	40	40		, <sup>™</sup> - 44	
a. Soil classes are in accordance with the Unified	I, ML-CL, & I-CL	60	860	24	24	•		
<ul> <li>b. Anchor bolts shall be cast-in-place with a minible within 8 inches of the vertical reinforcing an less than 1 inch of grout measured from the instance. The sill plate shall be 2 x 6 minimum. Anchor d. Anchor bolts shall have a 2 inch by <sup>1</sup>/<sub>8</sub> inch this noncountersunk washers is permitted where an e. Minimum load to be used for the sizing of accerding to the foundation wall. The blocking shall be 2-by the full adjacent to the foundation wall. The blocking</li> </ul>	nimum 7-inch embed. Where ve and are to be spaced as required uside face of the masonry and the r bolts shall be placed at least 2 hick round or square washer tig unchor bolt spacing is half the sp cepted anchors or fasteners if and l depth joists or an approved al	rertical reinforcin d by this table. A the anchor bolt. $2^{1/2}$ inches from ghtened and cours spacing required anchor bolts are p alternative full d	ng is required by nchor bolts insta the edge of the s ntersunk $\frac{1}{4}$ inch by this table. not used. epth joist materi	illed in masonry shall be ill plate and the edge of into the top of the sill ial that is installed in th	e grouted in place with not f the foundation wall. plate. Use of standard and he first three joists spaces	E		4 4 <u>A A A A</u>

connector. Blocking shall be installed within 8 inches of an anchor bolt location.

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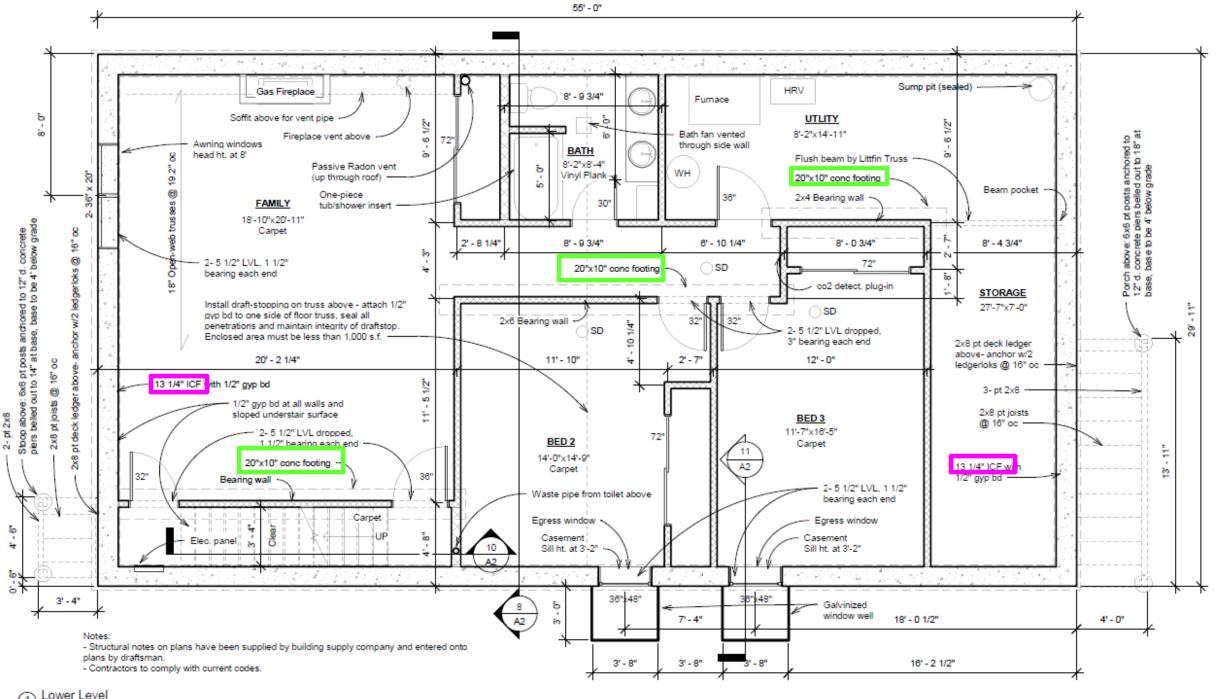
### Foundation Wall Backfill

West District

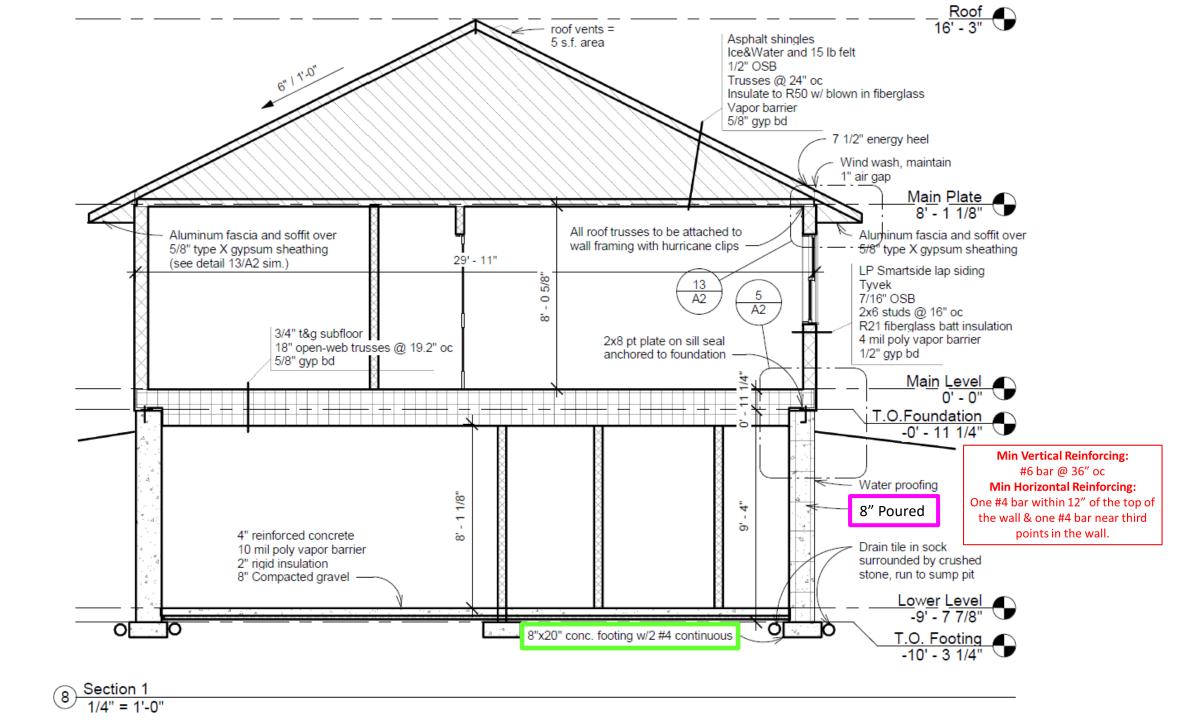
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- **R404.1.7 Backfill placement.** Backfill shall not be placed against the wall until the wall has sufficient strength and has been anchored to the floor above, or has been sufficiently braced to prevent damage by the backfill.
  - Exception: Bracing is not required for walls supporting less than 4 feet (1219 mm) of unbalanced backfill.

BUILDING



1/4" = 1'-0"



# Floors – Chapter 5



#### Floors

#### in this area to personal at

Check these items:

- Member type
  - Dimensional lumber
  - Truss
  - I-Joists
- Depth
- Span
- Draftstopping

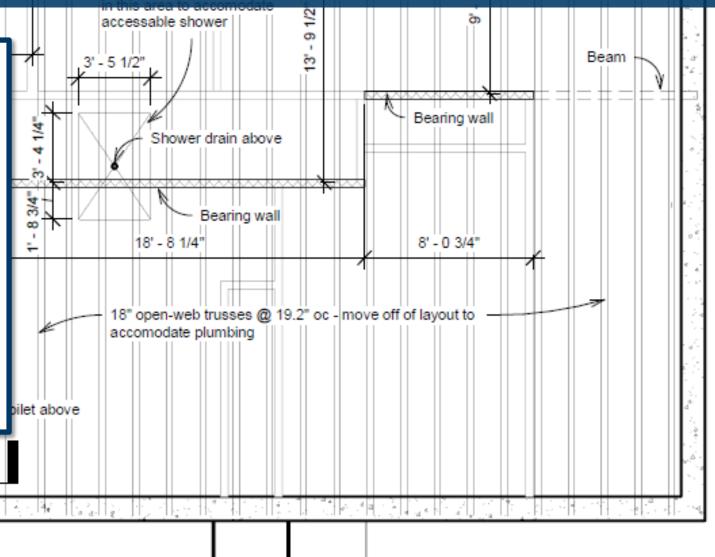
A. P. S. A. S. A. A.

 Mechanical room – 80sf max unprotected (no drywall on ceiling)

3' - 5 1/4"

Bearing wall-

≤Stair opening

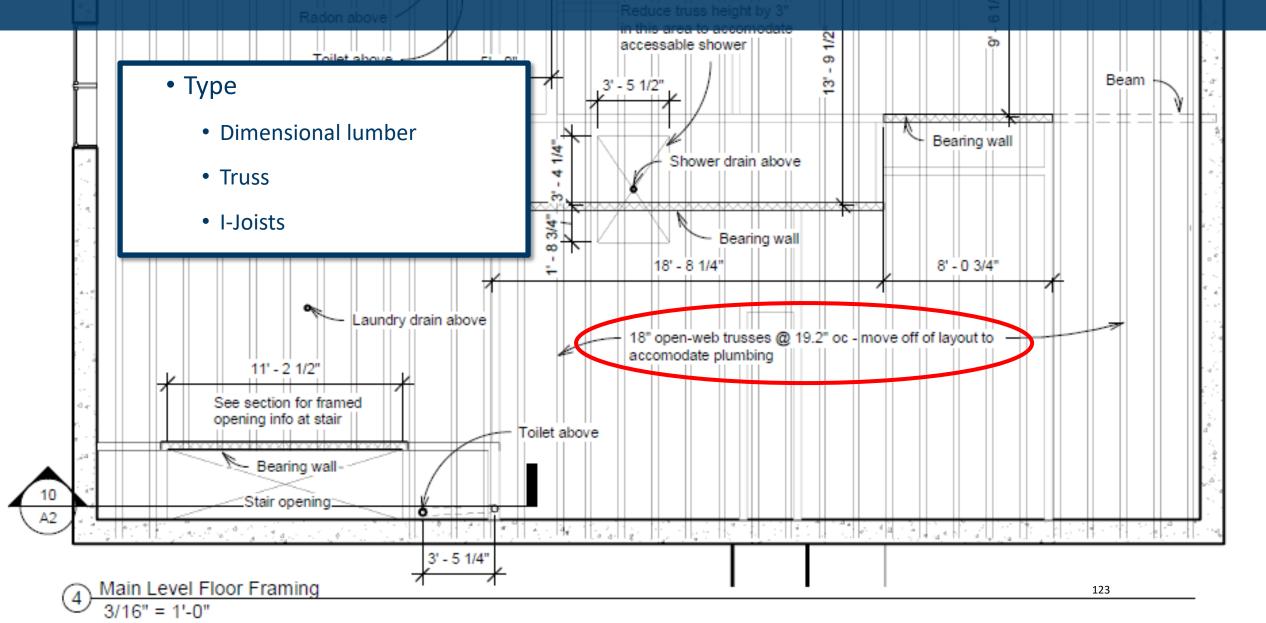


122

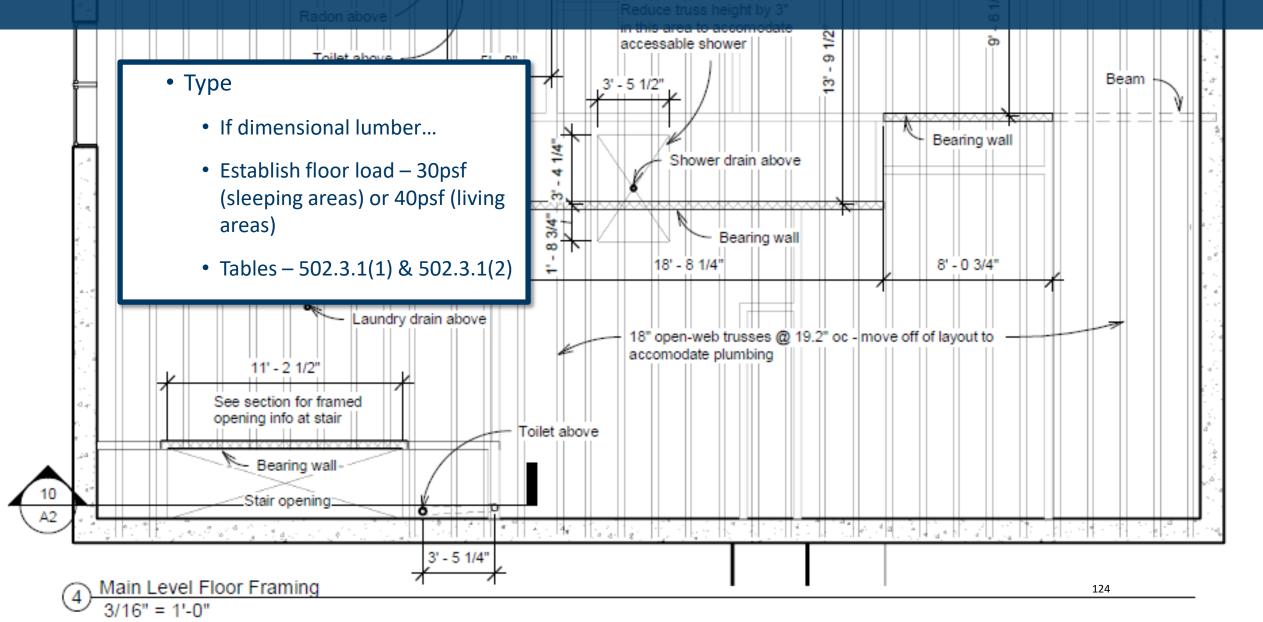
Main Level Floor Framing 3/16" = 1'-0"

Δ2

#### Floor Joists

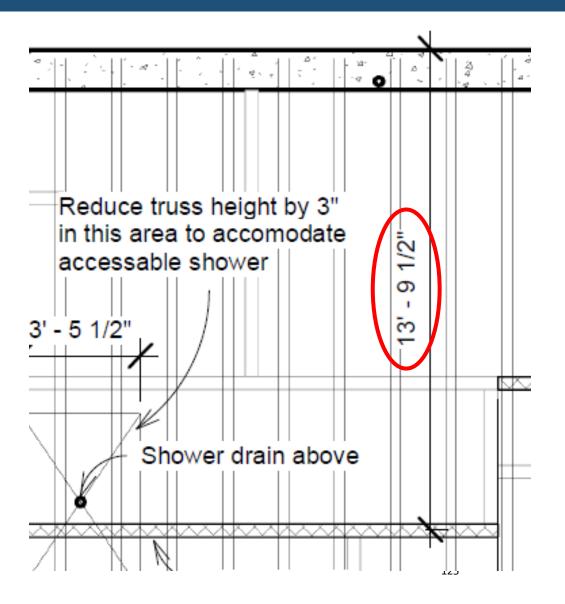


### **Floor Joists**



### Floors

- Tables 502.3.1(1) & 502.3.1(2)
- Main floor 40psf
  - Table 502.3.1(2)
  - Let's imagine plans specified dimensional lumber floor joists for the example. Check span.



#### TABLE R502.3.1(2) FLOOR JOIST SPANS FOR COMMON LUMBER SPECIES (Residential living areas, live load = 40 psf, $L/\Delta$ = 360)<sup>b</sup>

•	Tables –	502.3	.1(1)	& 502.	3.1(2)

- Main floor 40psf
  - Table 502.3.1(2)
  - Let's imagine plans said 2x10 #2 SPF floor joists, 16" oc.
  - Check span.
- Plans indicated span of  $13'-9\frac{1}{2}''$ 
  - 2x10 #2 SPF floor joists, 16" oc complies with a 10psf dead load.
  - Still complies if the load increases to a 20psf dead load.

	(Residential living areas, live load = 40 psf, $L/\Delta$ = 360) <sup>b</sup>												
				DEAD LOA	D = 10 psf			DEAD LOA	\D = 20 psf				
JOIST SPACING	SPECIES AND G	RADE	2×6	2 × 8	2 × 10	2 × 12	2×6	2 × 8	2 × 10	2 × 12			
(inches)							m floor joist spans						
			(ft in.)	(ft in.)	(ft in.)	(ft in.)	(ft in.)	(ft in.)	(ft in.)	(ft in.)			
	Douglas fir-larch	SS	11-4	15-0	19-1	23-3	11-4	15-0	19-1	23-3			
	Douglas fir-larch	#1	10-11	14-5	18-5	22-0	10-11	14-2	17-4	20-1			
	Douglas fir-larch	#2	10-9	14-2	18-0	20-11	10-8	13-6	16-5	19-1			
	Douglas fir-larch	#3	8-11	11-3	13-9	16-0	8-1	10-3	12-7	14-7			
	Hem-fir	SS	10-9	14-2	18-0	21-11	10-9	14-2	18-0	21-11			
	Hem-fir	#1	10-6	13-10	17-8	21-6	10-6	13-10	17-1	19-10			
	Hem-fir	#2	10-0	13-2	16-10	20-4	10-0	13-1	16-0	18-6			
12	Hem-fir	#3	8-8	11-0	13-5	15-7	7-11	10-0	12-3	14-3			
12	Southern pine	SS	11-2	14-8	18-9	22-10	11-2	14-8	18-9	22-10			
	Southern pine	#1	10-9	14-2	18-0	21-11	10-9	14-2	16-11	20-1			
	Southern pine	#2	10-3	13-6	16-2	19-1	9-10	12-6	14-9	17-5			
	Southern pine	#3	8-2	10-3	12-6	14-9	7-5	9-5	11-5	13-6			
	Spruce-pine-fir	SS	10-6	13-10	17-8	21-6	10-6	13-10	17-8	21-6			
	Spruce-pine-fir	#1	10-3	13-6	17-3	20-7	10-3	13-3	16-3	18-10			
	Spruce-pine-fir	#2	10-3	13-6	17-3	20-7	10-3	13-3	16-3	18-10			
	Spruce-pine-fir	#3	8-8	11-0	13-5	15-7	7-11	10-0	12-3	14-3			
	Douglas fir-larch	SS	10-4	13-7	17-4	21-1	10-4	13-7	17-4	21-1			
	Douglas fir-larch	#1	9-11	13-1	16-5	19-1	9-8	12-4	15-0	17-5			
	Douglas fir-larch	#2	9-9	12-9	15-7	18-1	9-3	11-8	14-3	16-6			
	Douglas fir-larch	#3	7-8	9-9	11-11	13-10	7-0	8-11	10-11	12-7			
	Hem-fir	SS	9-9	12-10	16-5	19-11	9-9	12-10	16-5	19-11			
	Hem-fir	#1	9-6	12-7	16-0	18-10	9-6	12-2	14-10	17-2			
	Hem-fir	#2	9-1	12-0	15-2	17-7	8-11	11-4	13-10	16-1			
	Hem-fir	#3	7-6	9-6	11-8	13-6	6-10	8-8	10-7	12-4			
16	Southern pine	SS	10-2	13-4	17-0	20-9	10-2	13-4	17-0	20-9			
	Southern pine	#1	9-9	12-10	16-1	19-1	9-9	12-7	14-8	17-5			
	Southern pine	#2	9-4	11-10	14-0	16-6	8-6	10-10	12-10	15-1			
	Southern pine	#3	7-1	8-11	10-10	12-10	6-5	8-2	9-10	11-8			
	Spruce-pine-fir	SS	9-6	12-7	16-0	19-6	9-6	12-7	16-0	19-6			
	Spruce-pine-fir	#1	9-4	12-3	15-5	17-10	9-1	11-6	14-1	16-3			
	Spruce-pine-fir	#2	9-4	12-3	15-5	17-10	9-1	11-6	14-1	16-3			
•	Spruce-pine-fir	#3	7-6	9-6	11-8	13-6	6-10	8-8	10-7	12-4			
	• •		_	_					-	-			

#### Floors

- Plans indicate 18" open web truss.
- Truss specs will need to be reviewed, and may or may not be included with submittal.

https://www.midwestmanufacturing.com/Midw estWebsite/productType.do?productGroupId=7

#### Floors



- Plan submittals indicating I-Joists should include the spec book and installation instructions, along with the size and model of the members to be used.
- Cross check spans on drawings with allowable spans in spec book.

# Floors - Draftstopping

• A simple note or stamp would be well to add to the plans...

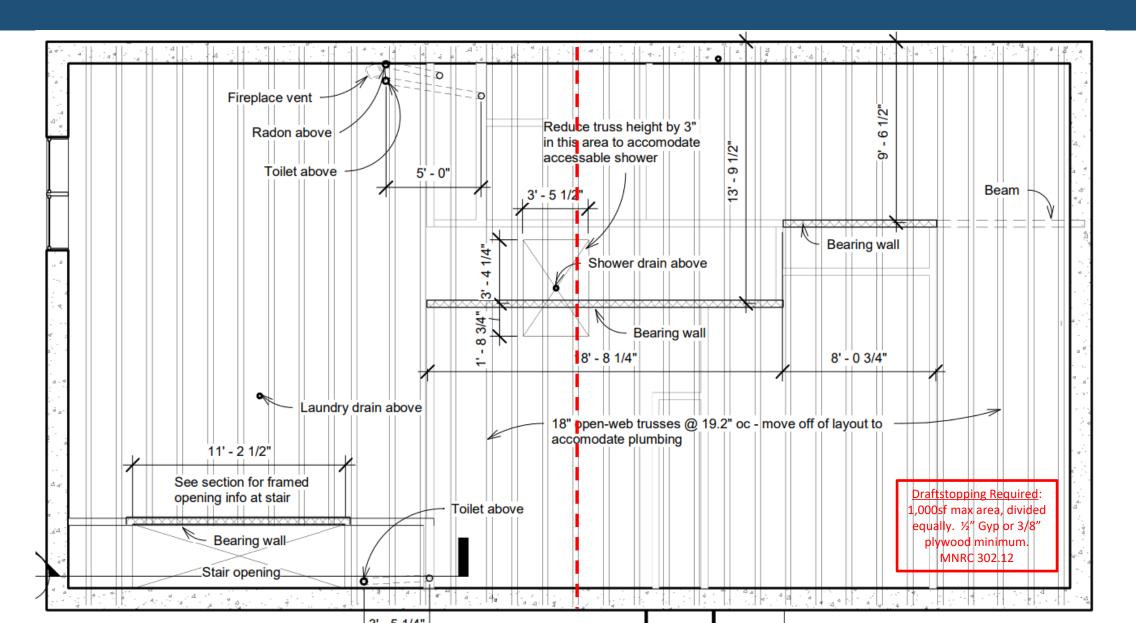


**R302.12 Draftstopping.** In combustible construction where there is usable space both above and below the concealed space of a floor-ceiling assembly, draftstops shall be installed so that the area of the concealed space does not exceed 1.000 square feet (92.9 m<sup>2</sup>). Draftstopping shall divide the concealed space into approximately equal areas. Where the assembly is enclosed by a floor membrane above and a ceiling membrane below, draftstopping shall be provided in floor-ceiling assemblies under the following circumstances:

- 1. Ceiling is suspended under the floor framing.
- Floor framing is constructed of truss-type open-web or perforated members.

**R302.12.1 Materials.** Draftstopping materials shall be not less than  $\frac{1}{2}$ -inch (12.7 mm) gypsum board,  $\frac{3}{8}$ -inch (9.5 mm) wood structural panels or other *approved* materials adequately supported. Draftstopping shall be installed parallel to the floor framing members unless otherwise *approved* by the *building official*. The integrity of the draftstops shall be maintained.

### Floors - Draftstopping



# **Floors - Fire Protection**

**R302.13 Fire protection of floors.** Floor assemblies that are not required elsewhere in this code to be fire-resistance rated, shall be provided with a 1/2-inch (12.7 mm) gypsum wall-board membrane, 5/8-inch (16 mm) wood structural panel membrane, or equivalent on the underside of the floor framing member. Penetrations or openings for ducts, vents, electrical outlets, lighting, devices, luminaires, wires, speakers, drainage, piping and similar openings or penetrations shall be permitted.

#### **Exceptions:**

- Floor assemblies located directly over a space protected by an automatic sprinkler system in accordance with Section P2904, NFPA 13D, or other approved equivalent sprinkler system.
- 2. Floor assemblies located directly over a crawl space not intended for storage or for the installation of fuel-fired or electric-powered heating appliances.

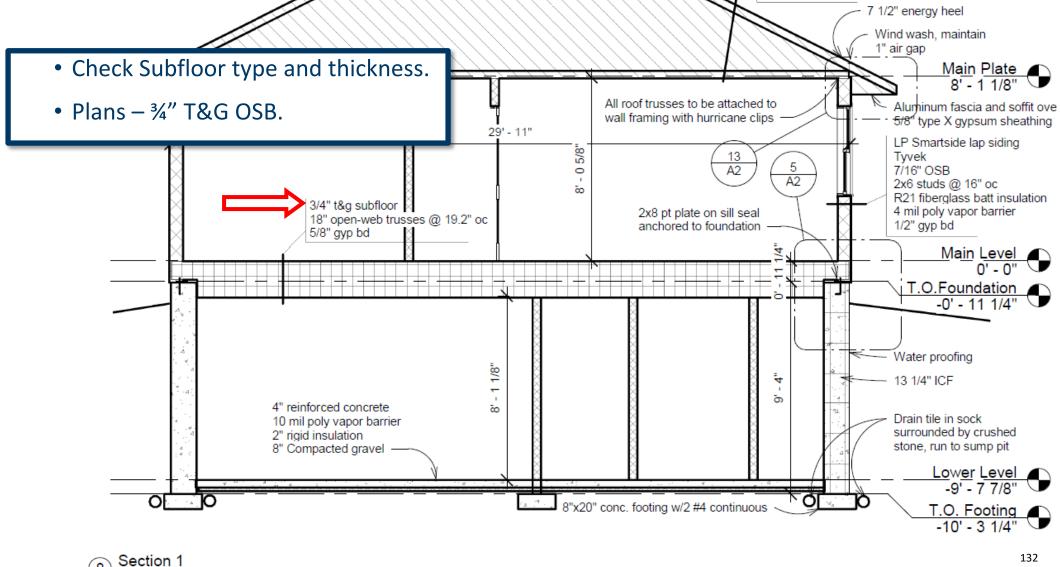
3. Portions of floor assemblies shall be permitted to be unprotected where complying with the following:

3.1. The aggregate area of the unprotected portions does not exceed 80 square feet (7.4 m<sup>2</sup>) per story.

- 3.2. Fireblocking in accordance with Section R302.11.1 is installed along the perimeter of the unprotected portion to separate the unprotected portion from the remainder of the floor assembly.
- 4. Wood floor assemblies using dimension lumber or structural composite lumber equal to or greater than <u>2-inch by 10-inch</u> (50.8 mm by 254 mm) nominal dimension, or other approved floor assemblies demonstrating equivalent fire performance.



#### Floors - Subfloor



8

1/4" = 1'-0"

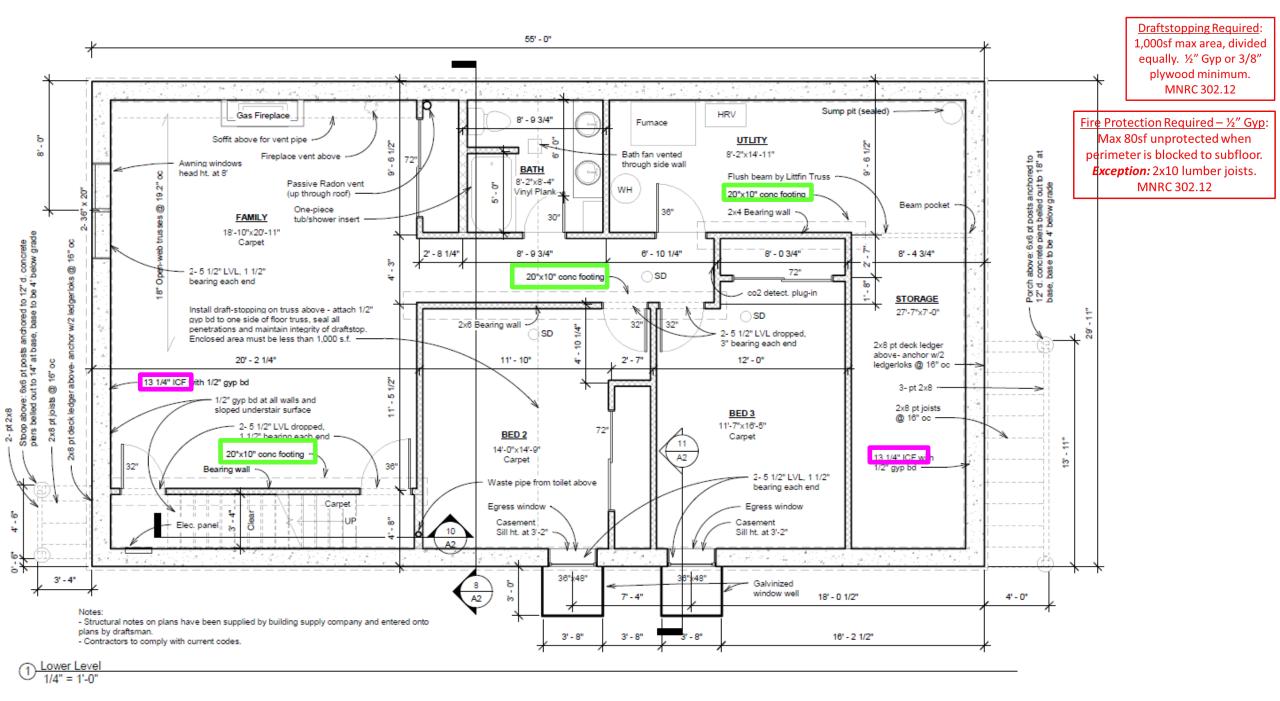
<u>Roof</u>

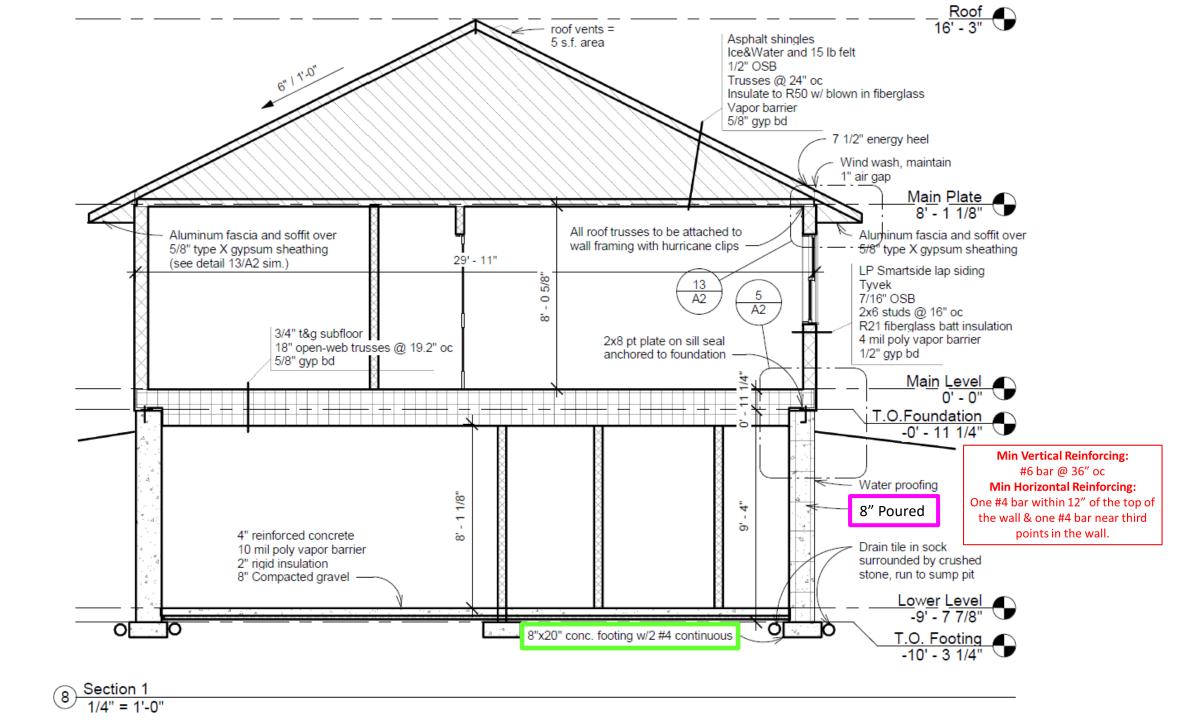
## Floors - Subfloor

- Check Subfloor type and thickness.
- Plans ¾" T&G OSB.
- Floor truss are 19.2" oc – complies.

		ALLOWABLE LIVE LOAD (psf) <sup>h, I</sup>		MAXIMUI (inch		LOAD (pounds per square foot, at maximum span)		MAXIMUM SPAN	
SPAN RATING	PANEL THICKNESS (inch)	SPAN @ 16 <sup>//</sup> o.c.	SPAN @ 24″ o.c.	With edge support <sup>d</sup>	Without edge support	Total load	Live load	<mark>(</mark> inches)	
Sheathing <sup>e</sup>					Roof			Subfloor <sup>i</sup>	
16/0	<sup>3</sup> / <sub>8</sub>	30	—	16	16	40	30	0	
20/0	<sup>3</sup> / <sub>8</sub>	50		20	20	40	30	0	
24/0	3/8	100	30	24	20 <sup>g</sup>	40	30	0	
24/16	7/16	100	40	24	24	50	40	16	
32/16	<sup>15</sup> / <sub>32</sub> , <sup>1</sup> / <sub>2</sub>	180	70	32	28	40	30	16 <sup>h</sup>	
40/20	<sup>19</sup> / <sub>32</sub> , <sup>5</sup> / <sub>8</sub>	305	130	40	32	40	30	20 <sup>h, i</sup>	
48/24	<sup>23</sup> / <sub>32</sub> , <sup>3</sup> / <sub>4</sub>	—	175	48	36	45	35	24	
60/32	7/ <sub>8</sub>		305	60	48	45	35	32	
Underlayment, C-C plugged, single floor <sup>e</sup>					•	Combination subfloo underlayment <sup>k</sup>			
16 o.c.	<sup>19</sup> / <sub>32</sub> , <sup>5</sup> / <sub>8</sub>	100	40	24	24	50	40	16 <sup>i</sup>	
20 o.c.	<sup>19</sup> / <sub>32</sub> , <sup>5</sup> / <sub>8</sub>	150	60	32	32	40	30	20 <sup>i, j</sup>	
24 o.c.	<sup>23</sup> / <sub>32</sub> , <sup>3</sup> / <sub>4</sub>	240	100	48	36	35	25	24	
32 o.c.	7/8		185	48	40	50	40	32	
48 o.c.	$1^{3}/_{32}, 1^{1}/_{8}$		290	60	48	50	40	48	

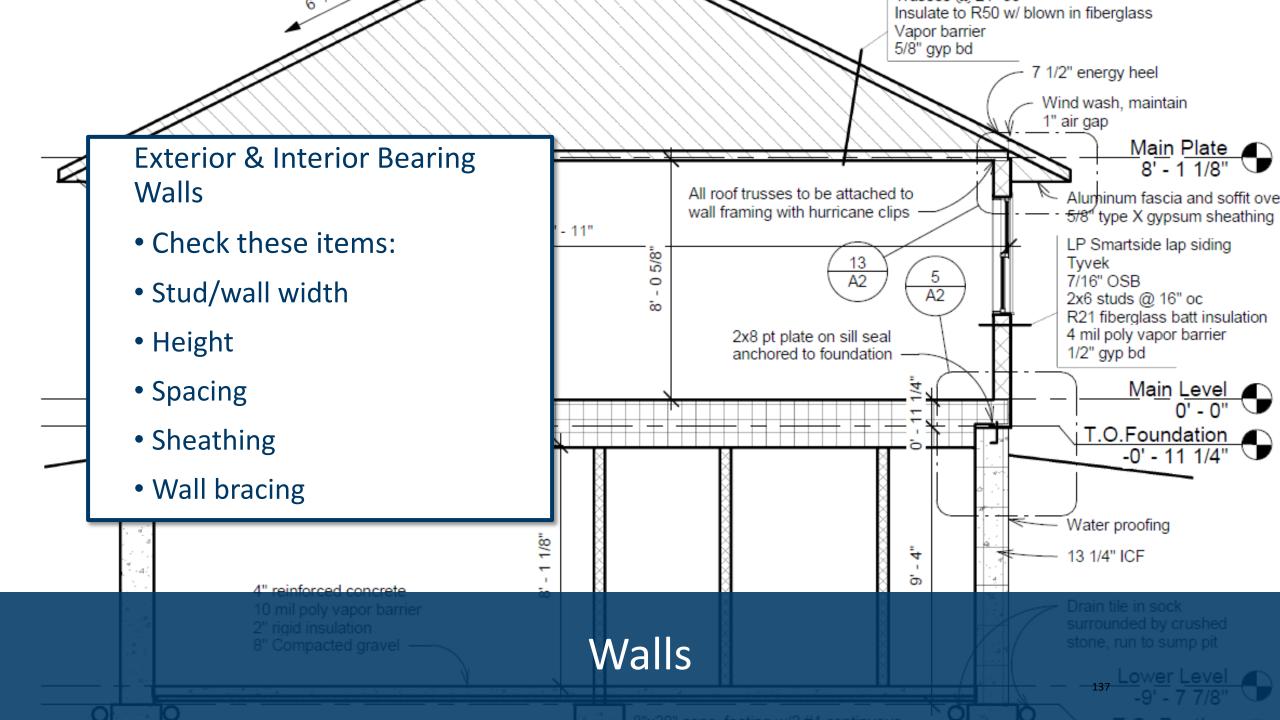
TABLE R503.2.1.1(1)

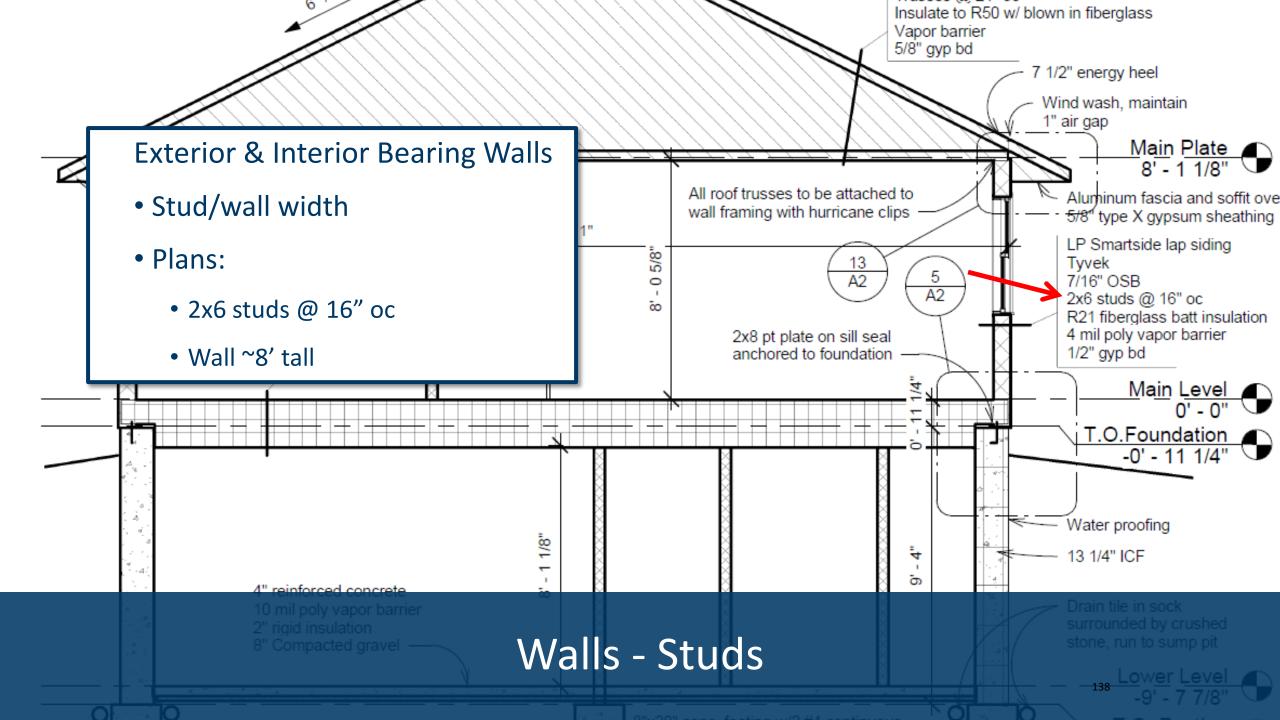


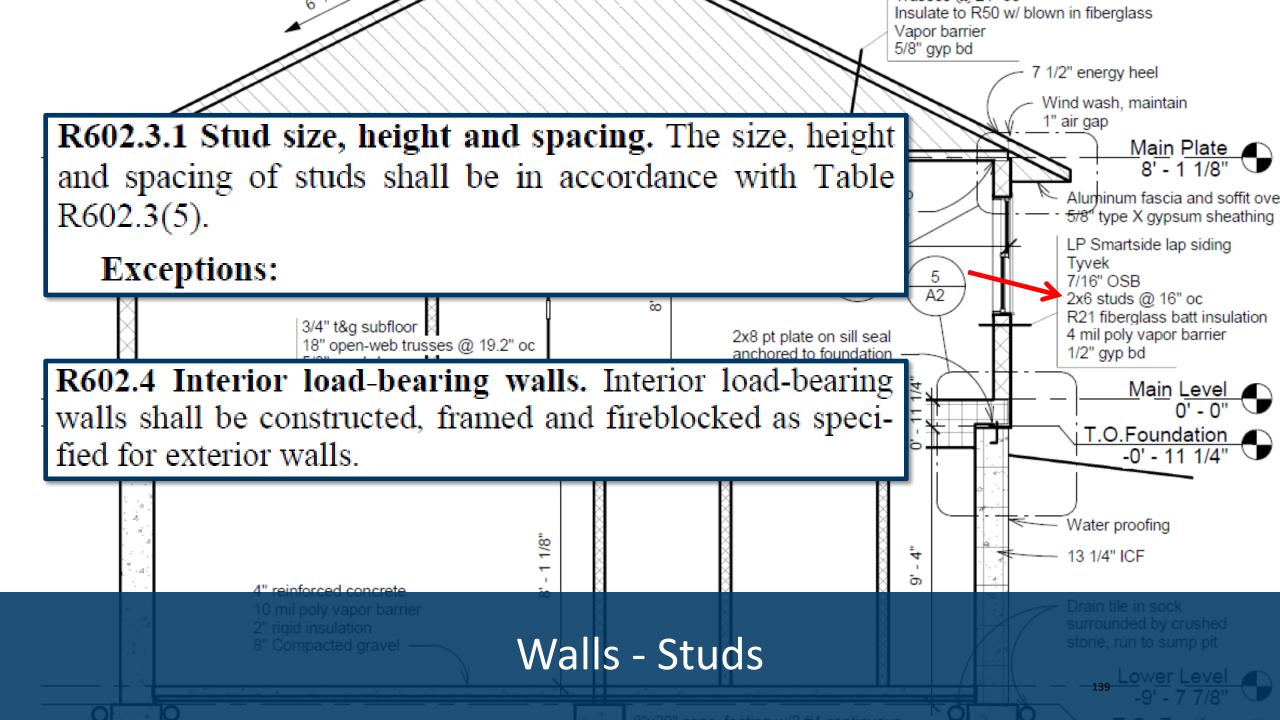




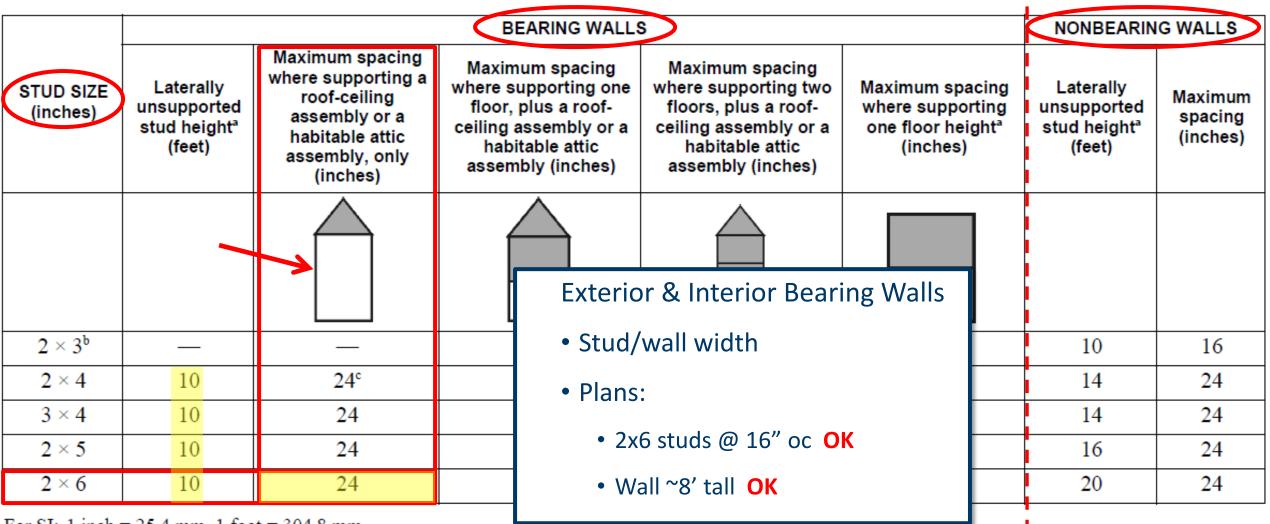
Walls – Chapter 6







#### TABLE R602.3(5) SIZE, HEIGHT AND SPACING OF WOOD STUDS<sup>a</sup>

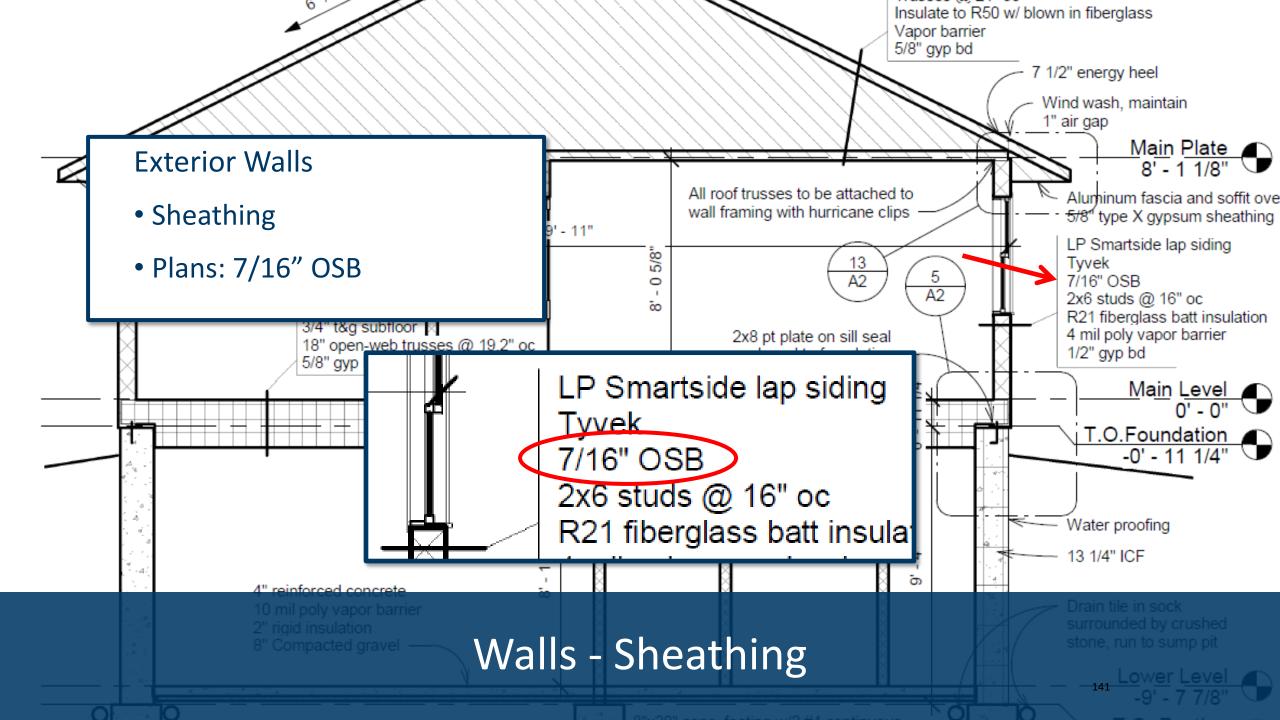


For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

a. Listed heights are distances between points of lateral support placed perpendicular to the plane of the wall. Bearing walls shall be sheathed on not less than one side or bridging shall be installed not greater than 4 feet apart measured vertically from either end of the stud. Increases in unsupported height are permitted where in compliance with Exception 2 of Section R602.3.1 or designed in accordance with accepted engineering practice.

b. Shall not be used in exterior walls.

c. A habitable attic assembly supported by  $2 \times 4$  studs is limited to a roof span of 32 feet. Where the roof span exceeds 32 feet, the wall study shall be increased



#### TABLE R602.3(3) REQUIREMENTS FOR WOOD STRUCTURAL PANEL WALL SHEATHING USED TO RESIST WIND PRESSURES<sup>a, b, c</sup>

	AIL	MINIMUM WOOD STRUCTURAL	MINIMUM NOMINAL	MAXIMUM WALL	PANEL NA	ULTIMATE DESIGN WIND SPEED V <sub>ult</sub> (mph)				
Size	Penetration (inches)	PANEL SPAN RATING	PANEL THICKNESS (inches)	STUD SPACING (inches)	Edges (inches o.c.)	Field (inches o.c.)	Wind exposure category			
	(inches)				(menes o.e.)	(menes o.c.)	В	С	D	
6d Common (2.0" × 0.113")	1.5	24/0	<sup>3</sup> / <sub>8</sub>	16	6	12	140	115	110	
8d Common	1.75	24/16	77	16	6	12	170	140	135	
(2.5"×0.131")	1.75		716	24	6	12	140	115	110	

For SI: 1 inch = 25.4 mm, 1 mile per hour = 0.447 m/s.

**B** – Urban Area

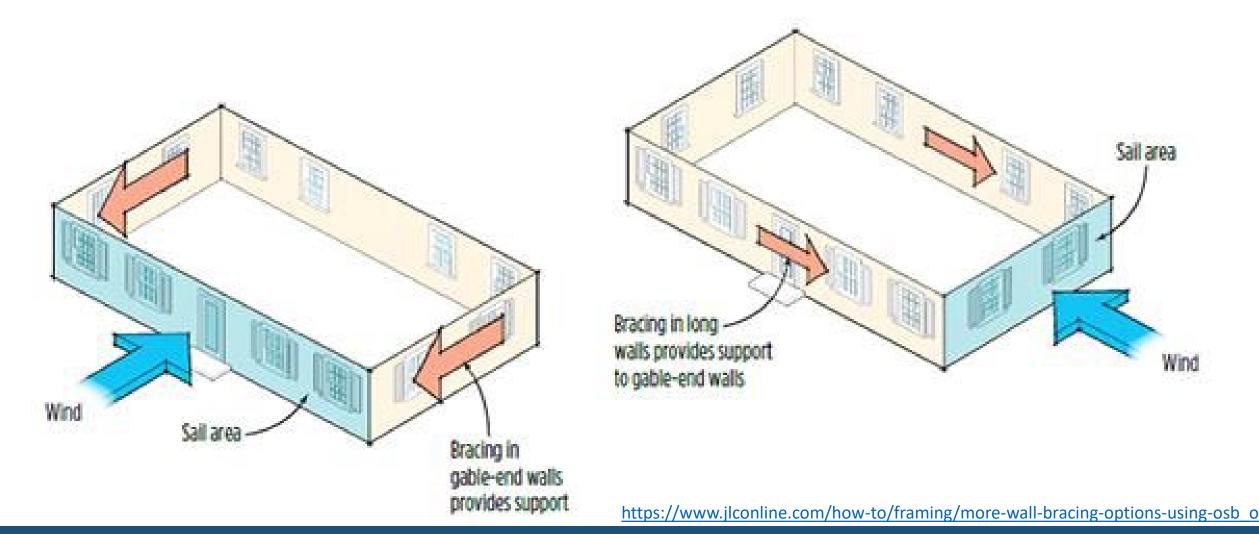
a. Panel strength axis parallel or perpendicular to supports. Three-ply plywood sheathing with studs spaced more than 16 inches on center shall be applied with panel strength axis perpendicular to supports.

b. Table is based on wind pressur Sheathing span | Floor span ing surfaces in accordance with Section R301.2. Lateral bracing requirements shall be in accordance with Section R602.10.

c. Wood structural panels with span ratings of Wall-16 or Wall-24 shall be permitted as an alternate to panels with a 24/0 span rating. Plywood siding rated 16 o.c. or 24 o.c. shall be permitted as an alternate to panels with a 24/16 span rating. Wall-16 and Plywood siding 16 o.c. shall be used with studs spaced not

# Walls - Sheathing

#### Walls Act Like Sails in the Wind



Walls - Bracing

#### Walls Act Like Sails in the Wind

**[RB] BRACED WALL LINE.** A straight line through the building plan that represents the location of the lateral resistance provided by the wall bracing.

**[RB] BRACED WALL LINE, CONTINUOUSLY SHEATHED.** A *braced wall line* with structural sheathing applied to all sheathable surfaces including the areas above and below openings.

**[RB] BRACED WALL PANEL.** A full-height section of wall constructed to resist in-plane shear loads through interaction of framing members, sheathing material and anchors. The panel's length meets the requirements of its particular bracing method, and contributes toward the total amount of bracing required along its *braced wall line* in accordance with Section R602.10.1.

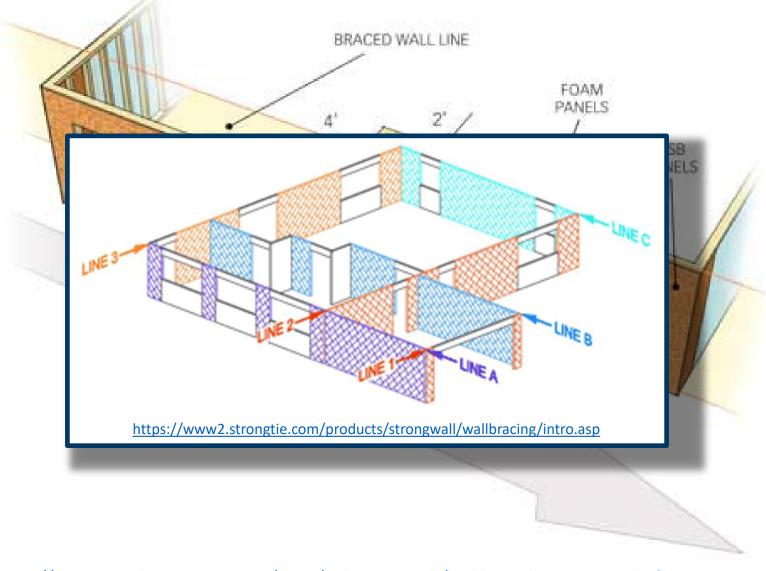
Wind

Saillar



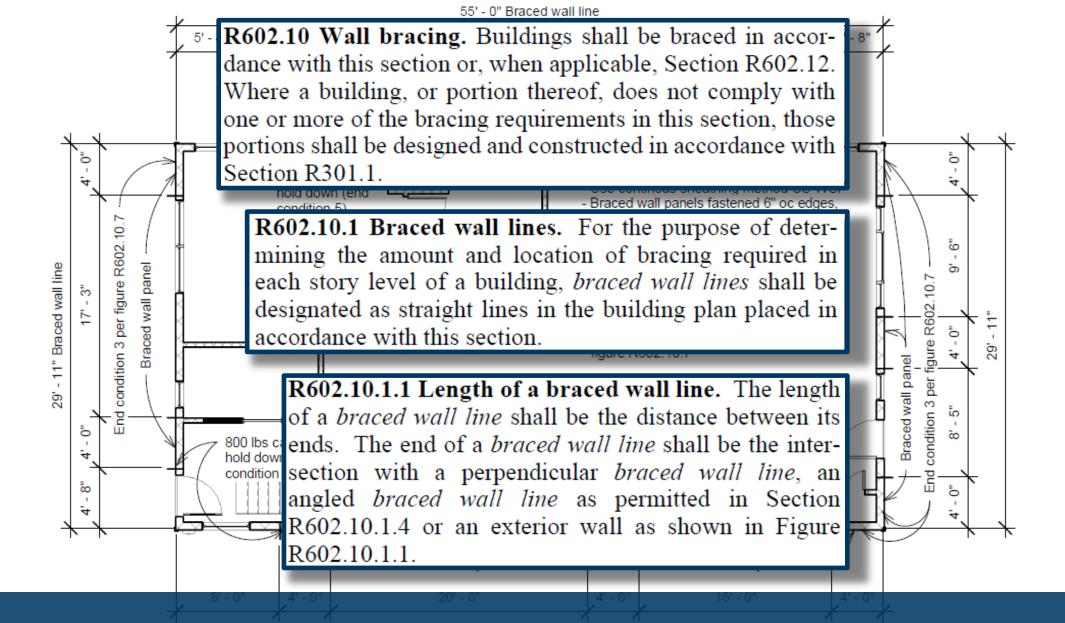


now-to/maning/more-wall-bracing-options-using-osb o

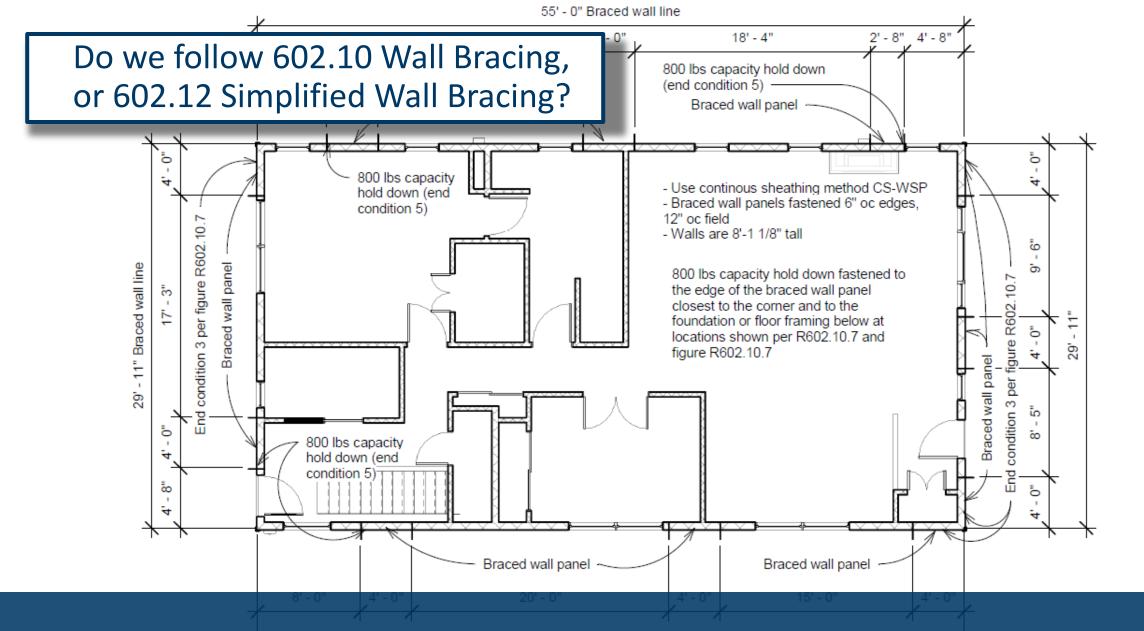


https://www.prosalesmagazine.com/news/industry-trends/build-it-right-bracing-walls-for-wind\_o

Walls - Bracing

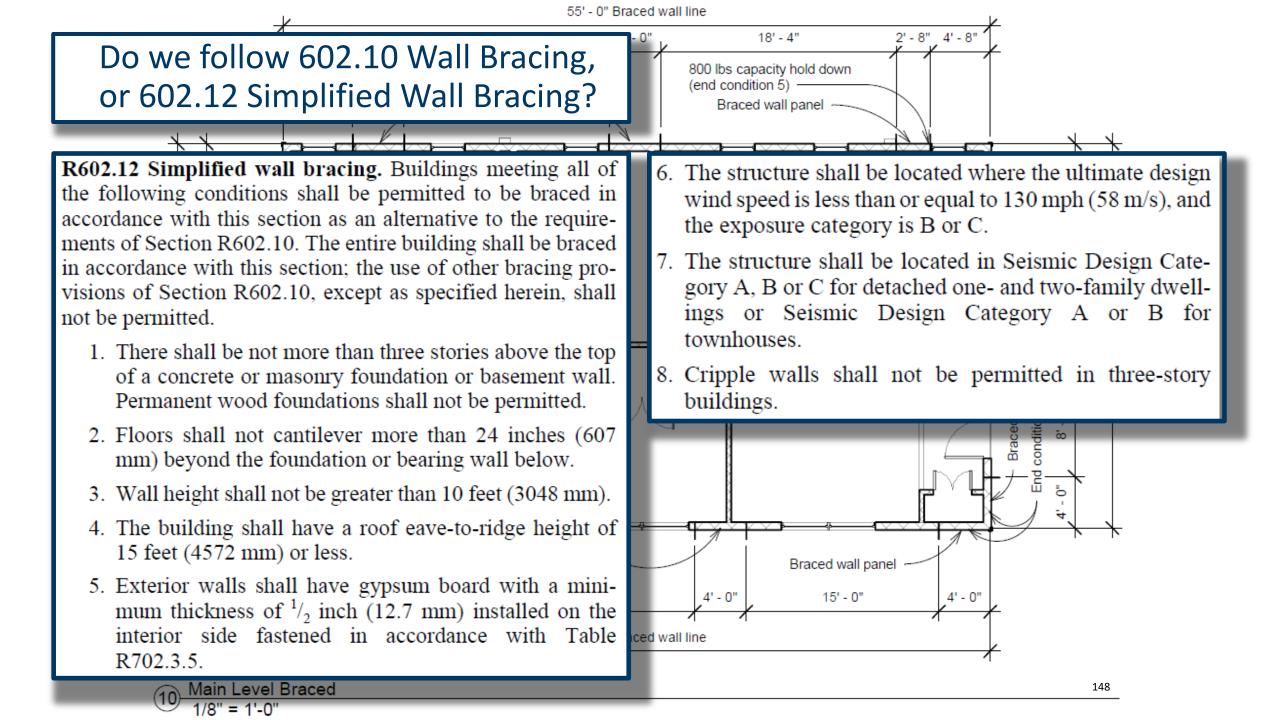


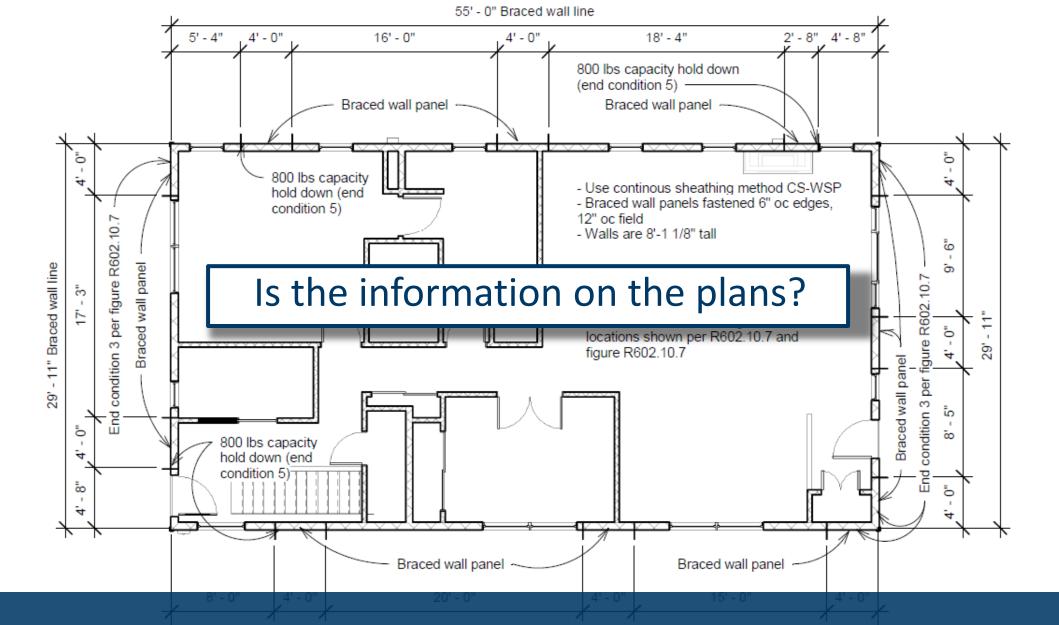
#### Wall Bracing – 602.10



#### Wall Bracing – 602.10

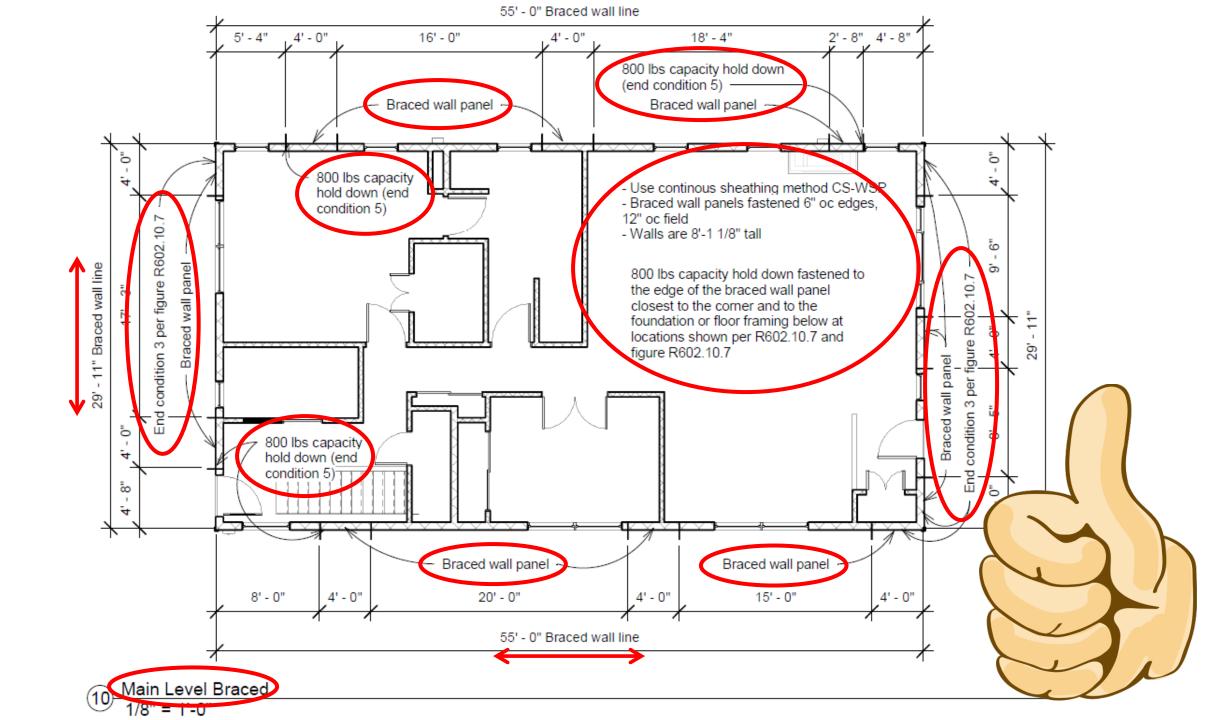






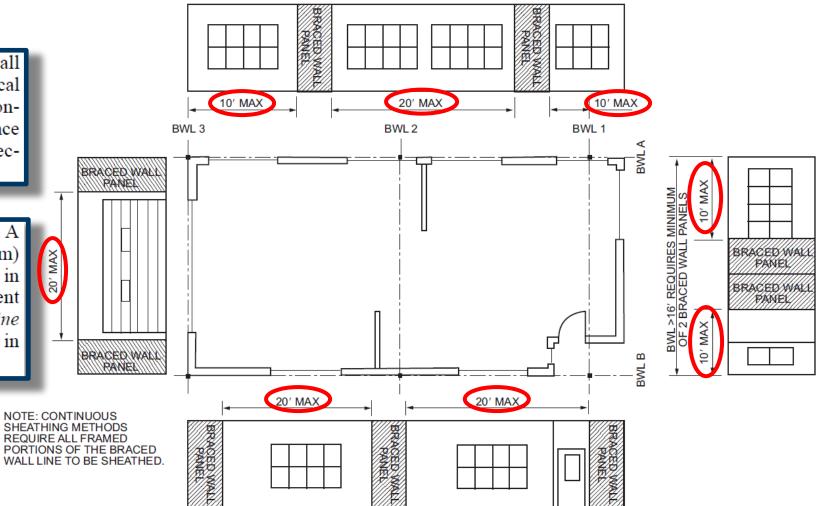
## Wall Bracing – 602.10





**R602.10.2 Braced wall panels.** *Braced wall panels* shall be full-height sections of wall that shall not have vertical or horizontal offsets. *Braced wall panels* shall be constructed and placed along a *braced wall line* in accordance with this section and the bracing methods specified in Section R602.10.4.

**R602.10.2.2 Locations of braced wall panels.** A *braced wall panel* shall begin within 10 feet (3810 mm) from each end of a *braced wall line* as determined in Section R602.10.1.1. The distance between adjacent edges of *braced wall panels* along a *braced wall line* shall be not greater than 20 feet (6096 mm) as shown in Figure R602.10.2.2.



For SI: 1 foot = 304.8 mm.

FIGURE R602.10.2.2 LOCATION OF BRACED WALL PANELS

#### TABLE R602.10.4 BRACING METHODS

			5101105	CONNECTION CRITER	RIA*					
M	THODS, MATERIAL	MINIMUM THICKNESS	FIGURE	Fasteners	Spacing					
	LIB Let-in-bracing	$1 \times 4$ wood or approved metal straps at 45° to 60° angles for		Wood: 2-8d common nails or 3-8d (2 <sup>1</sup> / <sub>2</sub> " long x 0.113" dia.) nails	Wood: per stud and top and bottom plates					
	Let-m-bracing	maximum 16" stud spacing		Metal strap: per manufacturer	Metal: per manufacturer					
	DWB Diagonal wood boards	<sup>3</sup> / <sub>4</sub> " (1" nominal) for maximum 24" stud spacing		2-8d $(2^{1}/_{2}" \log \times 0.113" \text{ dia.})$ nails or 2 - $1^{3}/_{4}" \log \text{ staples}$	Per stud					
	WSP Wood	3/ <sub>8</sub> ″		Exterior sheathing per Table R602.3(3)	6" edges 12" field					
	structural panel (See Section R604)	18		Interior sheathing per Table R602.3(1) or R602.3(2)	Varies by fastener					
Intermittent Bracing Methods	BV-WSP <sup>e</sup> Wood structural panels with stone or masonry veneer (See Section R602.10.6.5)	7/ <sub>16</sub> ″	See Figure R602.10.6.5	8d common (2 <sup>1</sup> / <sub>2</sub> " × 0.131) nails	4" at panel edges 12" at intermediate supports 4" at braced wall panel end posts					
	SFB Structural fiberboard sheathing	<sup>1</sup> / <sub>2</sub> " or <sup>25</sup> / <sub>32</sub> " for maximum 16" stud spacing		$\begin{array}{l} 1^{1} / _{2} " \log \times 0.12 " \mbox{ dia. (for } ^{1} / _{2} " \mbox{ thick sheathing) } 1^{3} / _{4} " \log \times 0.12 " \mbox{ dia. (for } ^{23} / _{32} " \mbox{ thick sheathing) } \\ \mbox{ galvanized roofing nails } \end{array}$	3" edges 6" field					
termitte	GB	<sup>1</sup> / <sub>2</sub> ″		Nails or screws per Table R602.3(1) for exterior locations	For all braced wall panel locations: 7"					
II	Gypsum board	12		Nails or screws per Table R702.3.5 for interior locations	edges (including top and bottom plates) 7" field					
	PBS Particleboard sheathing (See Section R605)	<sup>3</sup> / <sub>s</sub> " or <sup>1</sup> / <sub>2</sub> " for maximum 16" stud spacing		For ${}^{3}\!\!/_{g}$ ", 6d common (2" long × 0.113" dia.) nails For ${}^{1}\!/_{2}$ ", 8d common (2 ${}^{1}\!/_{2}$ " long × 0.131" dia.) nails	3" edges 6" field					
	PCP Portland cement plaster	See Section R703.7 for maximum 16" stud spacing		$1^{1/2}$ " long, 11 gage, $7/16$ " dia. head nails or $7/6$ " long, 16 gage staples	6" o.c. on all framing members					
	HPS Hardboard panel siding	<sup>7/</sup> 16" for maximum 16" stud spacing		$0.092^{\prime\prime}$ dia., $0.225^{\prime\prime}$ dia. head nails with length to accommodate $1^{1}\!/_{2}^{\prime\prime}$ penetration into studs	4" edges 8" field					
	ABW Alternate braced wall	3/ <sub>8</sub> ″		See Section R602.10.6.1	See Section R602.10.6.1					
			(continued)							

(continued)

#### TABLE R602.10.4—continued BRACING METHODS

			FIGURE	CONNECTION	I CRITERIA*
	METHODS, MATERIAL	MINIMUM THICKNESS	FIGURE	Fasteners	Spacing
g Methods	PFH Portal frame with hold-downs	3/ <sub>8</sub> ″		See Section R602.10.6.2	See Section R602.10.6.2
Interm	PFG Portal frame at garage	<sup>7</sup> / <sub>16</sub> ″		See Section R602.10.6.3	See Section R602.10.6.3
	CS-WSP Continuously sheathed wood structural panel	3/ <sub>8</sub> ″		Exterior sheathing per Table R602.3(3) Interior sheathing per	6" edges 12" field
sp	wood siruciurar paner			Table R602.3(1) or R602.3(2)	Varies by fastener
Continuous Sheathing	CS-G <sup>b, c</sup> Continuously sheathed wood structural panel adjacent to garage openings			See Method CS-WSP	See Method CS-WSP
	CS-PF Continuously sheathed portal frame	<sup>7</sup> / <sub>16</sub> ″		See Section R602.10.6.4	See Section R602.10.6.4
	CS-SFB <sup>4</sup> Continuously sheathed structural fiberboard $1/2$ " or $25/32$ " for maximum 16" stud spacing $1/2$ " for max		$1^{1}/_{2}$ " long × 0.12" dia. (for $1^{1}/_{2}$ " thick sheathing) $1^{3}/_{4}$ " long × 0.12" dia. (for $2^{25}/_{32}$ " thick sheathing) galvanized roofing nails	3" edges 6" field	

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.0175 rad, 1 pound per square foot = 47.8 N/m<sup>2</sup>, 1 mile per hour = 0.447 m/s.

a. Adhesive attachment of wall sheathing, including Method GB, shall not be permitted in Seismic Design Categories C, D<sub>0</sub>, D<sub>1</sub> and D<sub>2</sub>.

b. Applies to panels next to garage door opening where supporting gable end wall or roof load only. Shall only be used on one wall of the garage. In Seismic Design Categories D<sub>0</sub>, D<sub>1</sub> and D<sub>2</sub>, roof covering dead load shall not exceed 3 psf.

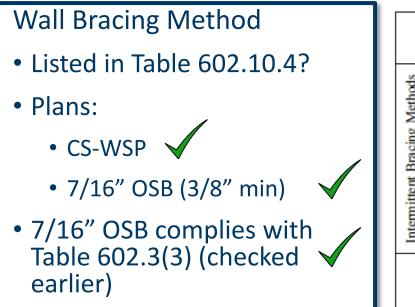
c. Garage openings adjacent to a Method CS-G panel shall be provided with a header in accordance with Table R602.7(1). A full-height clear opening shall not be permitted adjacent to a Method CS-G panel.

d. Method CS-SFB does not apply in Seismic Design Categories D<sub>10</sub>, D<sub>1</sub> and D<sub>2</sub>.

e. Method applies to detached one- and two-family dwellings in Seismic Design Categories D<sub>0</sub> through D<sub>2</sub> only.

### ★ Most commonly seen methods

#### TABLE R602.10.4—continued BRACING METHODS



METHODS, MATERIAL			FIGURE	CONNECTION	I CRITERIA*
	METHODS, MATERIAL	MINIMUM THICKNESS	FIGURE	Fasteners	Spacing
g Methods	PFH Portal frame with hold-downs	<sup>3</sup> / <sub>8</sub> ″		See Section R602.10.6.2	See Section R602.10.6.2
Interm	PFG Portal frame at garage	<sup>7</sup> / <sub>16</sub> ″		See Section R602.10.6.3	See Section R602.10.6.3
	CS-WSP Continuously sheathed	3/ <sub>8</sub> ″		Exterior sheathing per Table R602.3(3)	6" edges 12" field
sp	wood structural panel	18		Interior sheathing per Table R602.3(1) or R602.3(2)	Varies by fastener
Sheathing Methods	CS-G <sup>b, c</sup> Continuously sheathed wood structural panel adjacent to garage openings	<sup>3</sup> / <sub>8</sub> ″		See Method CS-WSP	See Method CS-WSP
Continuous SI	CS-PF Continuously sheathed portal frame	<sup>7</sup> / <sub>16</sub> ″		See Section R602.10.6.4	See Section R602.10.6.4
Contin	CS-SFB <sup>4</sup> Continuously sheathed structural fiberboard	<sup>1</sup> / <sub>2</sub> " or <sup>25</sup> / <sub>32</sub> " for maximum 16" stud spacing		$1^{1/2}$ " long × 0.12" dia. (for $1/2$ " thick sheathing) $1^{3/4}$ " long × 0.12" dia. (for $2^{5/32}$ " thick sheathing) galvanized roofing nails	3" edges 6" field

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.0175 rad, 1 pound per square foot = 47.8 N/m<sup>2</sup>, 1 mile per hour = 0.447 m/s.

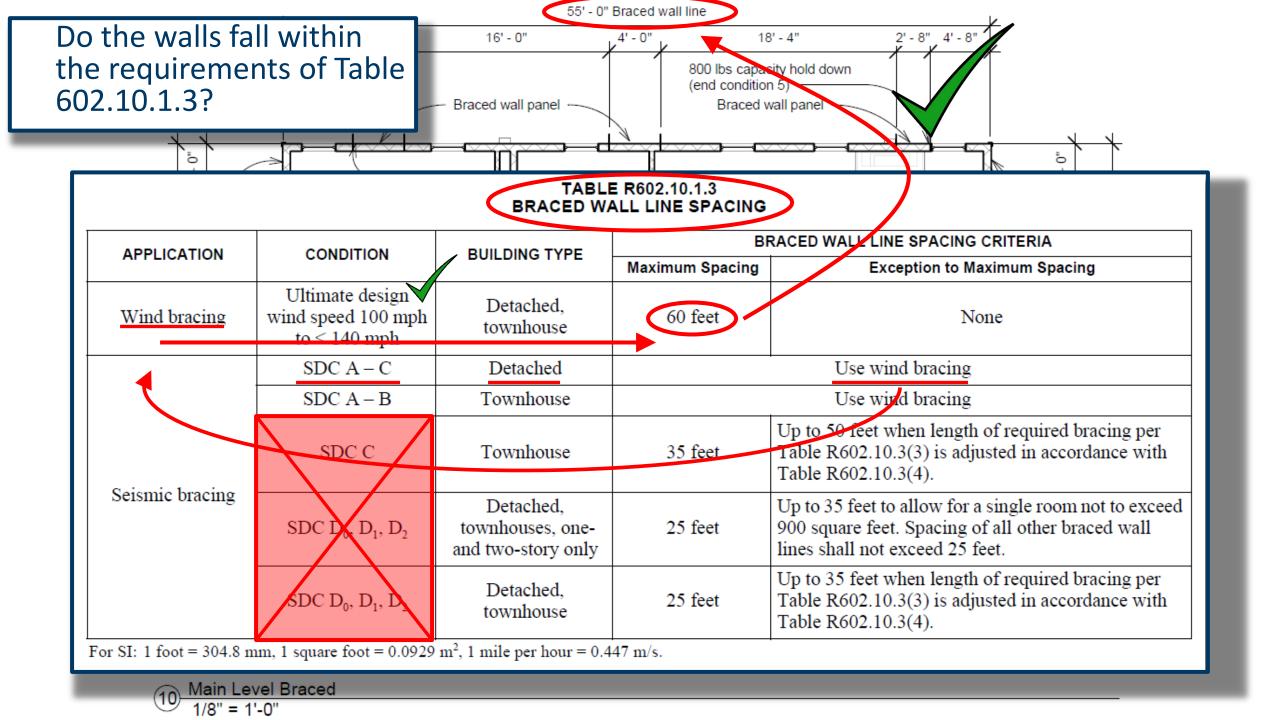
a. Adhesive attachment of wall sheathing, including Method GB, shall not be permitted in Seismic Design Categories C, D<sub>0</sub>, D<sub>1</sub> and D<sub>2</sub>.

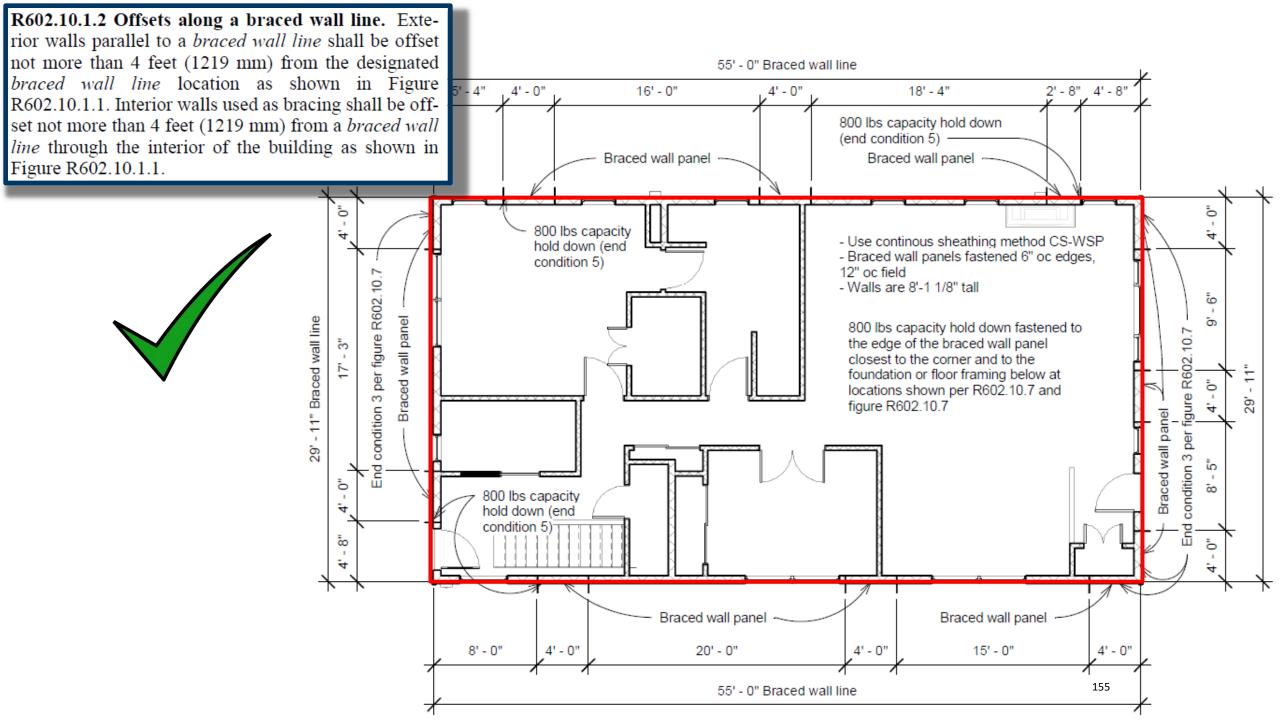
b. Applies to panels next to garage door opening where supporting gable end wall or roof load only. Shall only be used on one wall of the garage. In Seismic Design Categories D<sub>0</sub>, D<sub>1</sub> and D<sub>2</sub>, roof covering dead load shall not exceed 3 psf.

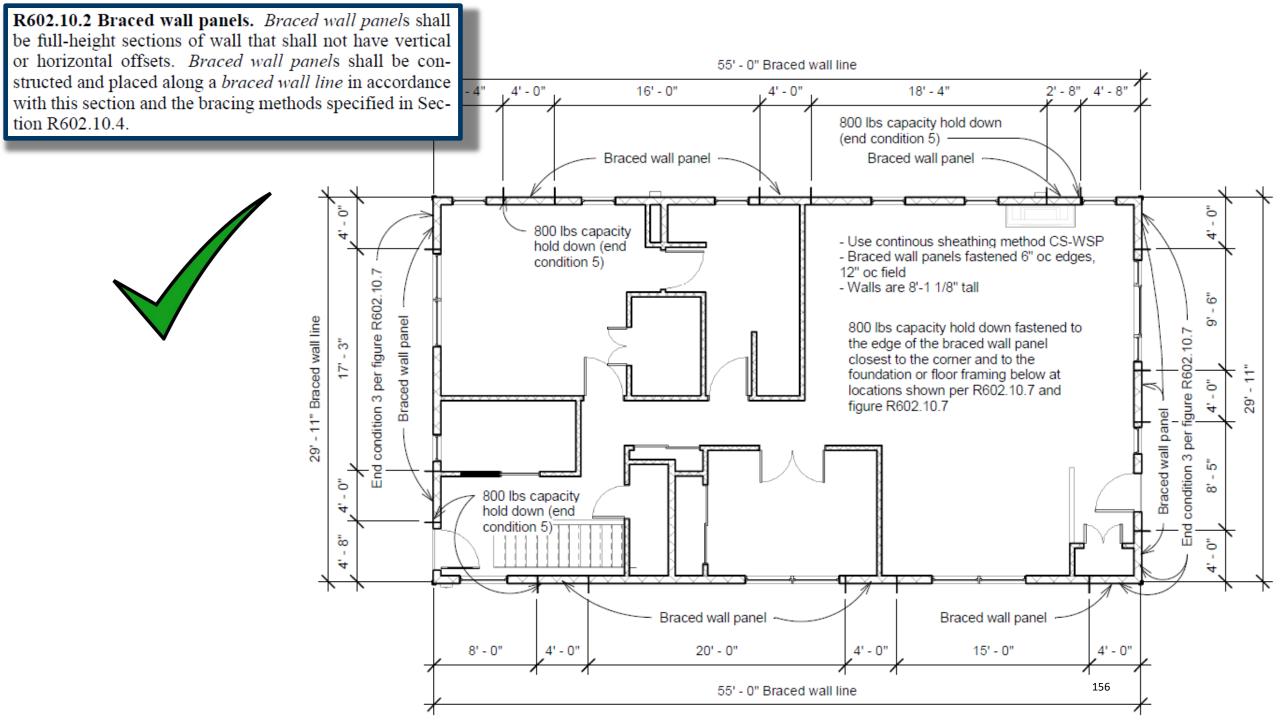
c. Garage openings adjacent to a Method CS-G panel shall be provided with a header in accordance with Table R602.7(1). A full-height clear opening shall not be permitted adjacent to a Method CS-G panel.

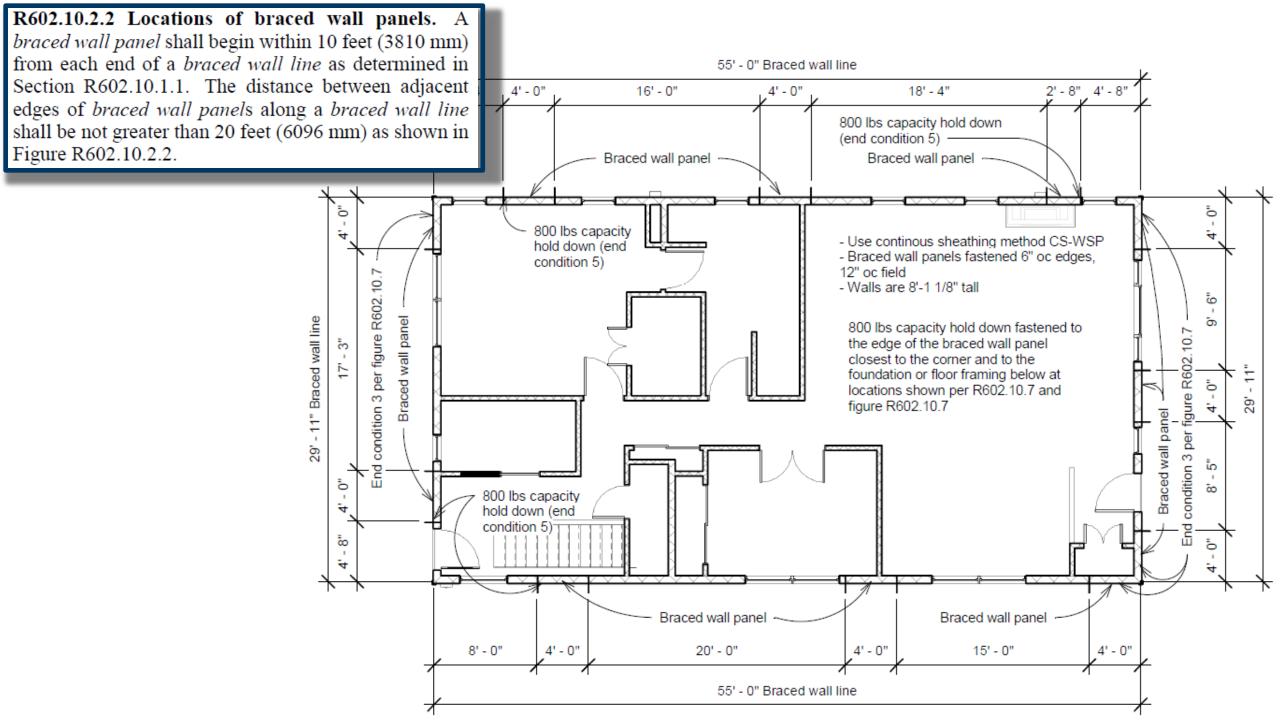
d. Method CS-SFB does not apply in Seismic Design Categories D<sub>0</sub>, D<sub>1</sub> and D<sub>2</sub>.

e. Method applies to detached one- and two-family dwellings in Seismic Design Categories D<sub>0</sub> through D<sub>2</sub> only.





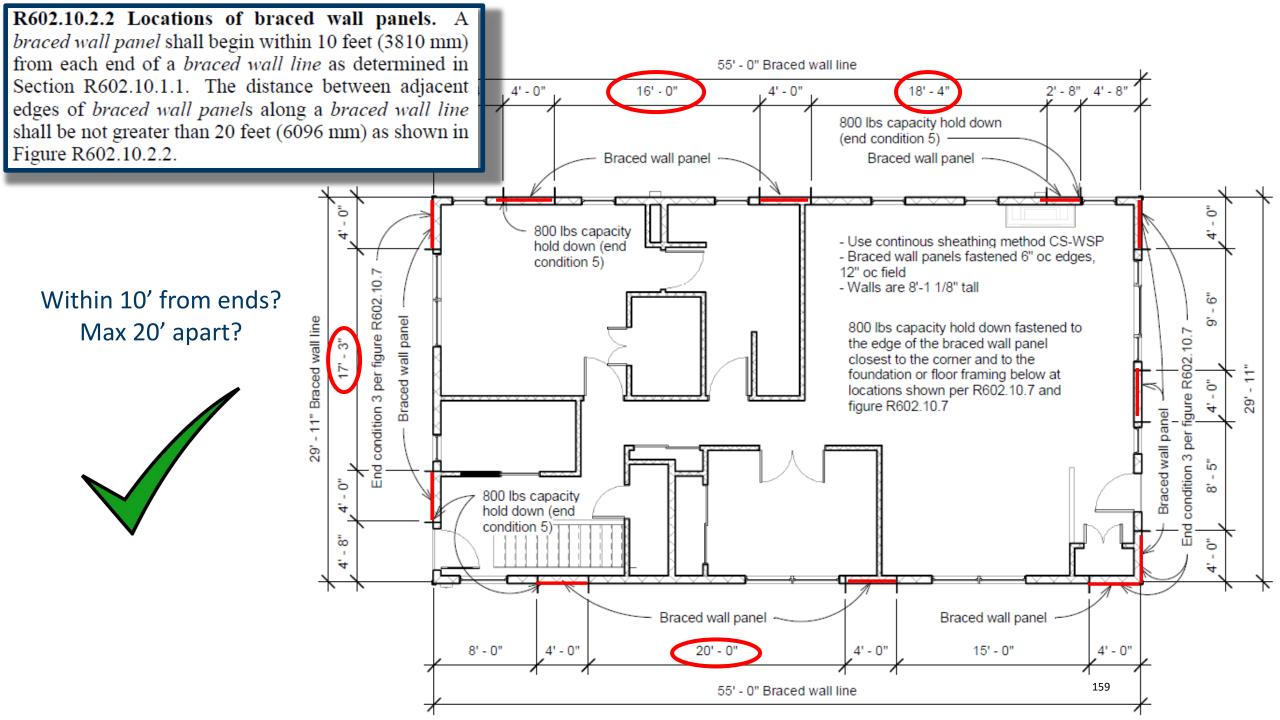


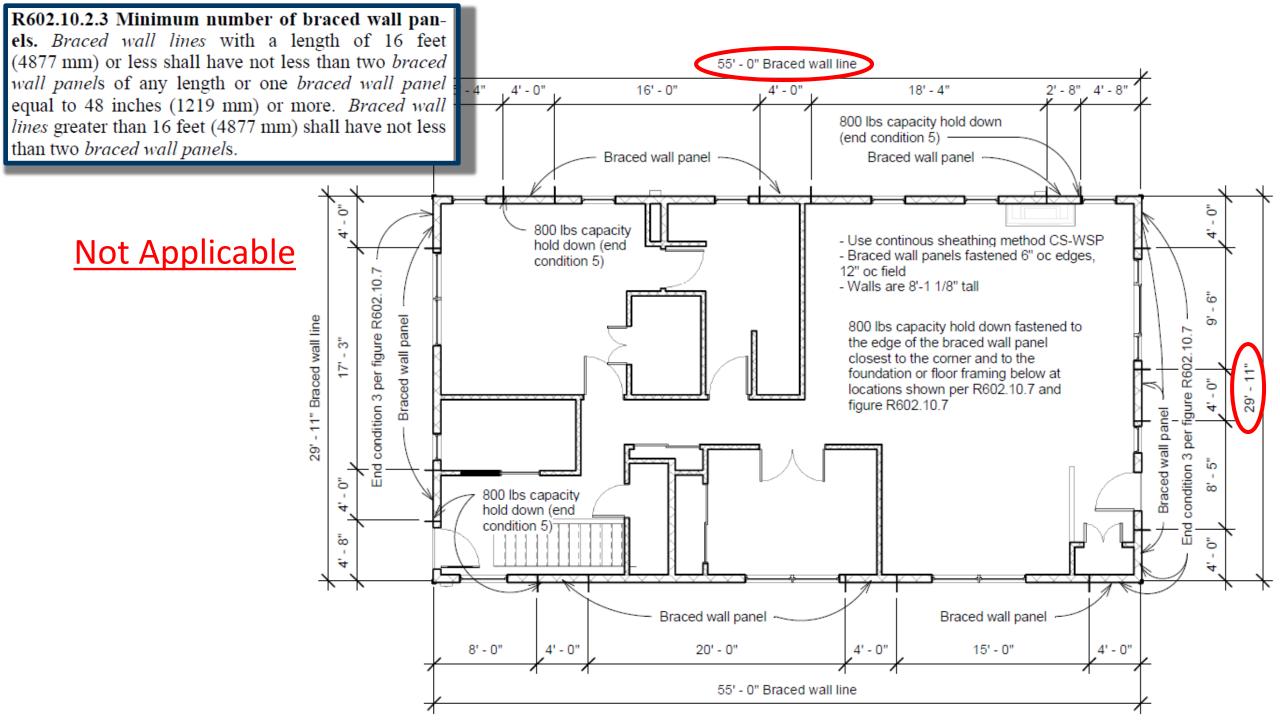


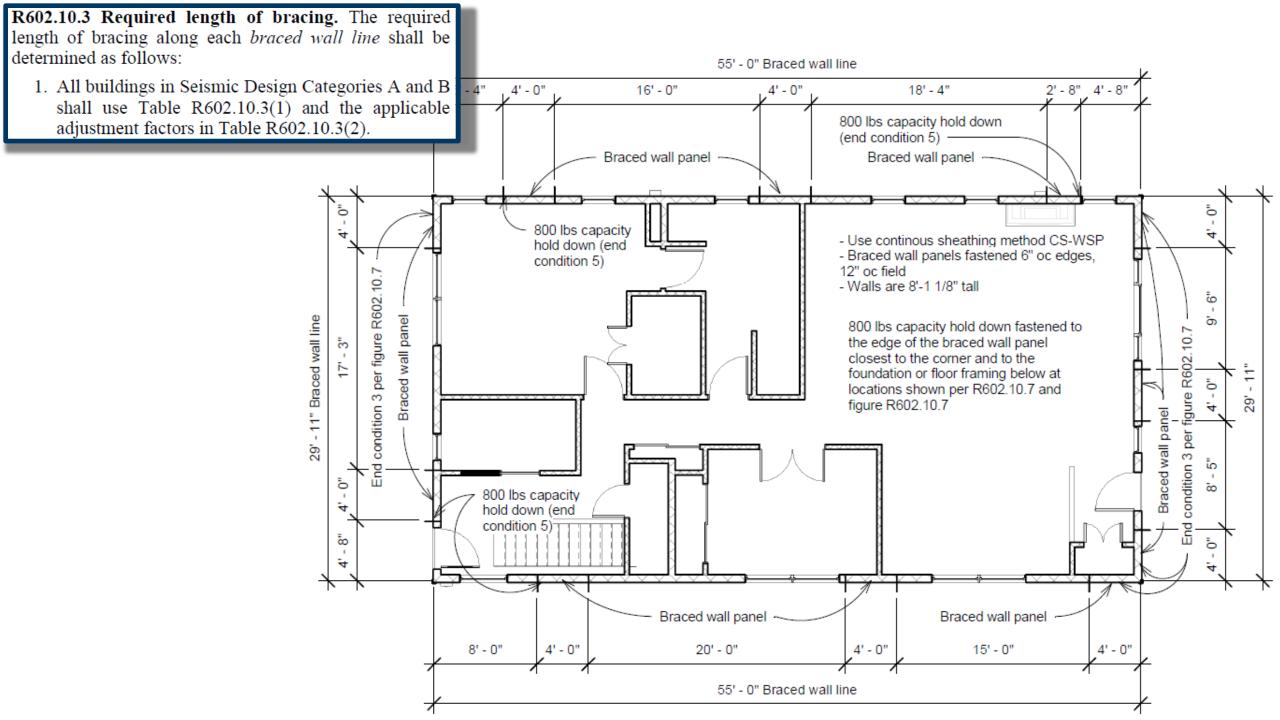
R602.10.2.2 Locations of braced wall panels. A BRACED WALL PANEL braced wall panel shall begin within 10 feet (3810 mm) PANEL W from each end of a braced wall line as determined in Section R602.10.1.1. The distance between adjacent edges of braced wall panels along a braced wall line shall be not greater than 20 feet (6096 mm) as shown in 10' MAX 10' MAX 20' MAX Figure R602.10.2.2. BWL 2 BWL 3 BWL 1 BWL BRACED WALL PANEL 10' MAX BWL >16' REQUIRES MINIMUM OF 2 BRACED WALL PANELS BRACEDWALL 20' MAX PANEL MAX BRACED WALL PANEL , 10 BWL B 20' MAX 20' MAX NOTE: CONTINUOUS SHEATHING METHODS BRACED WALL PANEL **REQUIRE ALL FRAMED** PORTIONS OF THE BRACED PANEL PANEL WALL LINE TO BE SHEATHED.

For SI: 1 foot = 304.8 mm.

FIGURE R602.10.2.2 LOCATION OF BRACED WALL PANELS







1. All buildings in Seismic Design Categories A and B shall use Table R602.10.3(1) and the applicable adjustment factors in Table R602.10.3(2).

TABLE R602.10.3(1) BRACING REQUIREMENTS BASED ON WIND SPEED

EXPOSURE C.     30-FOOT MEA     10-FOOT WAL     2 BRACED WA	N ROOF HEIGHT L HEIGHT		MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE*					
Ultimate Design Wind Speed (mph)	Sto	Braced Wall Line Spacing <sup>e</sup> (feet)	Method LIB <sup>b</sup>	Method GB	Methods DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP, ABW, PFH, PFC, CS-SFB	Methods CS-WSP, CS-G, CS-PF		
		10	3.5	3.5	2.0	1.5		
		20	6.0	6.0	3.5	3.0		
		30	8.5	8.5	5.0	4.5		
		40	11.5	11.5	6.5	5.5		
		50	14.0	14.0	8.0	7.0		
		60	16.5	16.5	9.5	8.0		
		10	6.5	6.5	2.5	2.0		

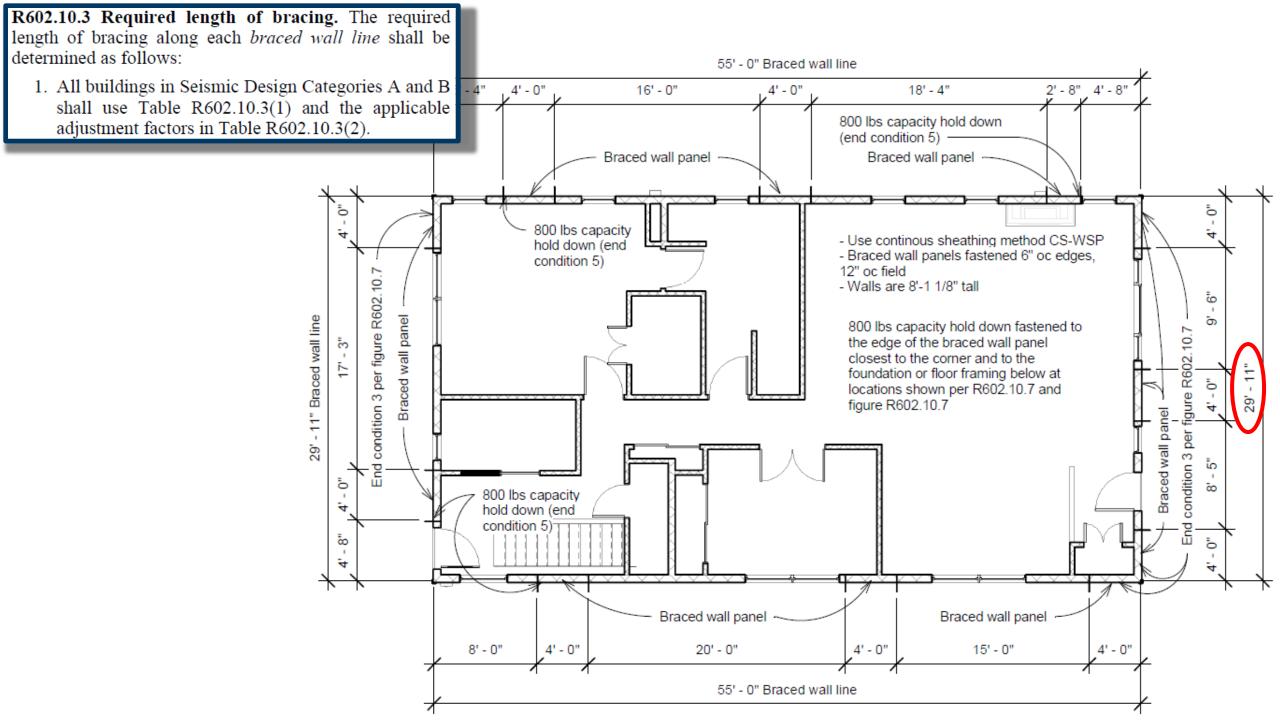
TABLE R602.10.3(1) BRACING REQUIREMENTS BASED ON WIND SPEED EXPOSURE CATEGORY B 30-FOOT MEAN ROOF HEIGHT MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS 10-FOOT WALL HEIGHT REQUIRED ALONG EACH BRACED WALL LINE\* 2 BRACED WALL LINES Methods Ultimate Braced Wall Line DWB, WSP, SFB, Methods Design Wind Story Location Spacing Method LIB<sup>b</sup> Method GB PBS, PCP, HPS, CS-WSP, CS-G, Speed BV-WSP, ABW, CS-PF (feet) (mph) PFH, PFC, CS-SFB 60 18.0 18.0 10.5 9.0 10 7.0 7.0 4.0 3.5 20 12.5 12.5 7.5 6.5 30 18.0 10.5 9.0 18.0 ≤115 40 23.5 13.5 23.5 11.5 50 29.0 29.0 16.5 14.0 60 34.5 34.5 20.0 17.0 NP 5.0 10 10.0 6.0 20 NP 18.5 11.0 9.0 30 NP 27.0 15.5 13.0 40 NP 35.0 20.0 17.0 50 NP 21.0 43.0 24.5 60 NP 51.0 29.0 25.0

(continued)

1. All buildings in Seismic Design Categories A and B shall use Table R602.10.3(1) and the applicable adjustment factors in Table R602.10.3(2).

#### TABLE R602.10.3(1) BRACING REQUIREMENTS BASED ON WIND SPEED

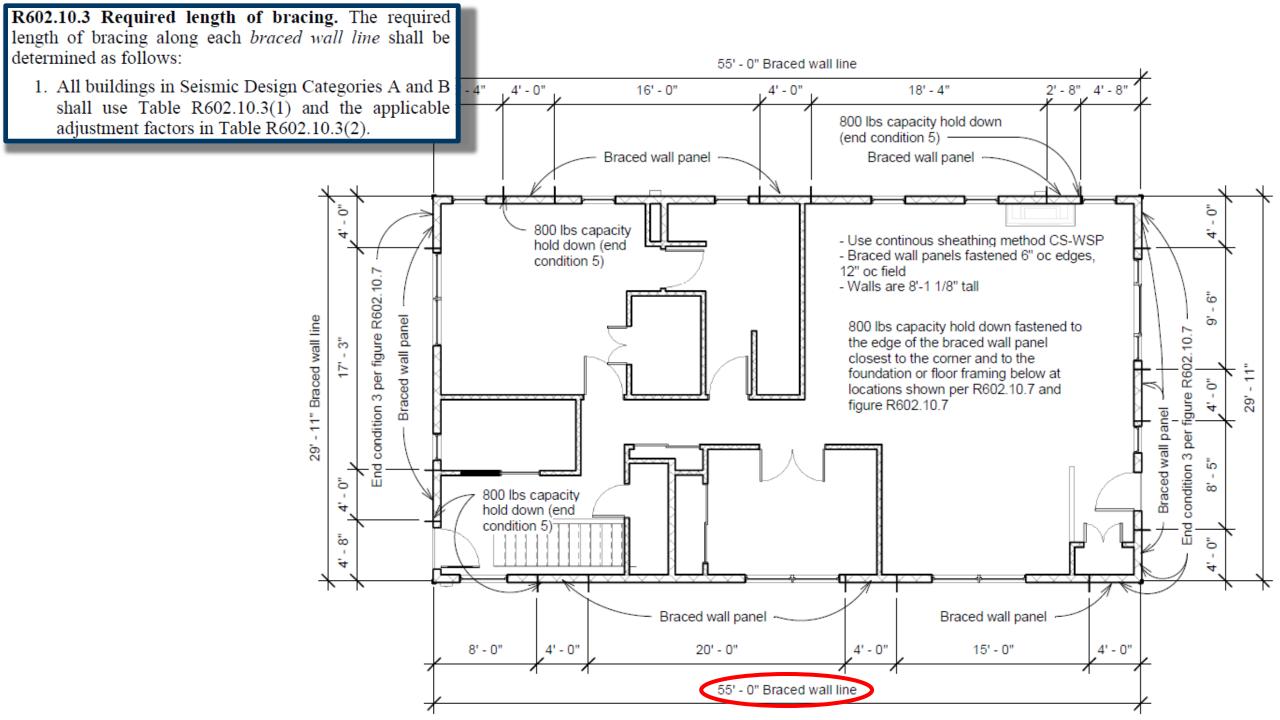
	EXPOSURE CATEGORY B     30-FOOT MEAN ROOF HEIGHT     10-FOOT WALL HEIGHT     2 BRACED WALL LINES			MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE*						
	Ultimate Design Wind Speed (mph)	Story Location	Braced Wall Line Spacing <sup>e</sup> (feet)	Method LIB⁵	Method GB	Methods DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP, ABW, PFH, PFC, CS-SFB	Methods <u>CS-WSP.</u> CS-G, CS-PF			
			10	3.5	3.5	2.0	2.0			
		$\land$	20	6.5	6.5	3.5	3.5			
		$\land$ $\square$	30	9.5	9.5	5.5	4.5			
			40	12.5	12.5	7.0	6.0			
			50	15.0	15.0	9.0	7.5			
			60	18.0	18.0	10.5	9.0			
			10	7.0	7.0	4.0	3.5			
			20	12.5	12.5	7.5	6.5			
	≤ 115		30	18.0	18.0	10.5	9.0			
	3115		40	23.5	23.5	13.5	11.5			
			50	29.0	29.0	16.5	14.0			
			60	34.5	34.5	20.0	17.0			
			10	NP	10.0	6.0	5.0			
		$\wedge$	20	NP	18.5	11.0	9.0			
			30	NP	27.0	15.5	13.0			
			40	NP	35.0	20.0	17.0			
			50	NP	43.0	24.5	21.0			
			60	NP	51.0	29.0	25.0			



1. All buildings in Seismic Design Categories A and B shall use Table R602.10.3(1) and the applicable adjustment factors in Table R602.10.3(2).

EXPOSURE CA     30-FOOT MEAN     10-FOOT WALL     2 BRACED WA	IN ROOF HEIGHT LL HEIGHT		MINIMUM Rf	TOTAL LENGTH (FEE EQUIRED ALONG EA(	MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE"						
Ultimate Design Wind Speed (mph)	Story Location	Braced Wall Line Spacing <sup>e</sup> (feet)	Method LIB <sup>b</sup>	Method GB	Methods DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP, ABW, PFH, PFC, CS-SFB	Methods <u>CS-WSP,</u> CS-G, CS-PF					
F 1	[	10	3.5	3.5	2.0	2.0					
		20	6.5	6.5	3.5	3.5					
	$\land \blacksquare$	30	9.5	9.5	5.5	4.5					
		40	12.5	12.5	7.0	6.0					
/		50	15.0	15.0	9.0	7.5					
		60	18.0	18.0	10.5	9.0					
1		10	7.0	7.0	4.0	3.5					
	$\land \land$	20	12.5	12.5	7.5	6.5					
≤ 115		30	18.0	18.0	10.5	9.0					
		40	23.5	23.5	13.5	11.5					
	🔲 🗌 '	50	29.0	29.0	16.5	14.0					
		60	34.5	34.5	20.0	17.0					
I		10	NP	10.0	6.0	5.0					
	$ \land $	20	NP	18.5	11.0	9.0					
	I [] '	30	NP	27.0	15.5	13.0					
	1 🗌 '	40	NP	35.0	20.0	17.0					
		50	NP	43.0	24.5	21.0					
	'	60	NP	51.0	29.0	25.0					

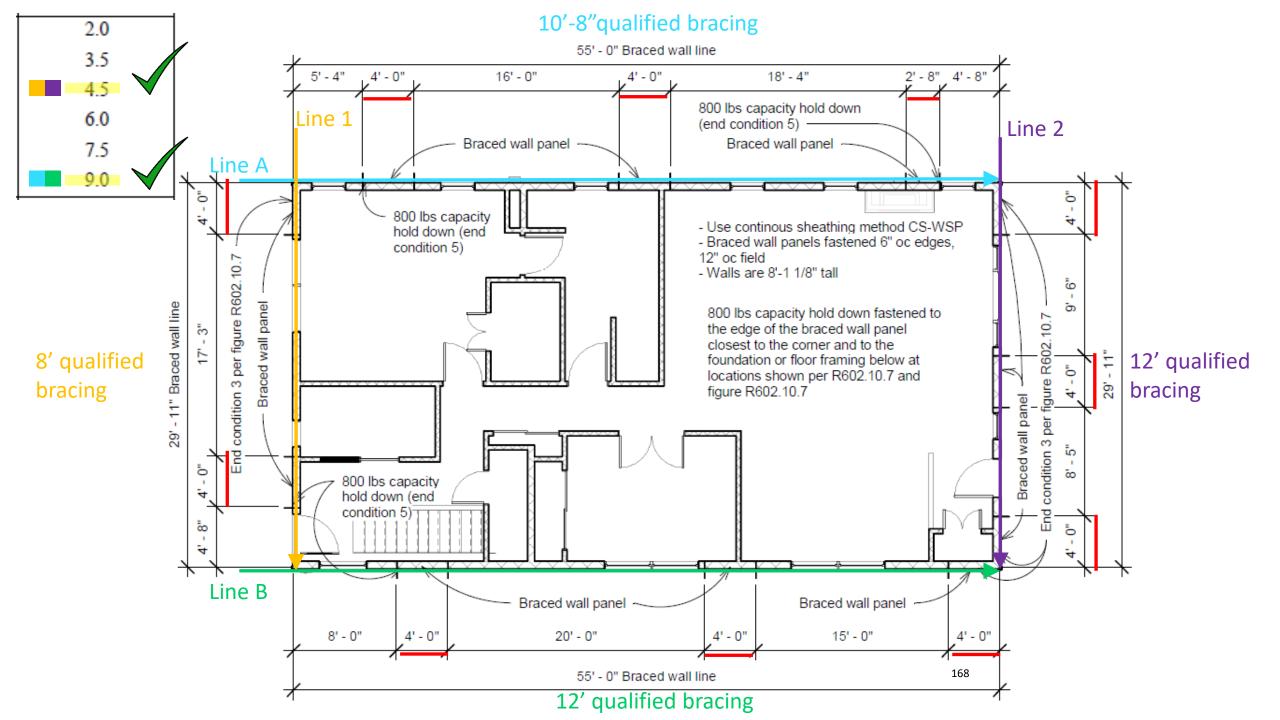
#### TABLE R602.10.3(1) BRACING REQUIREMENTS BASED ON WIND SPEED



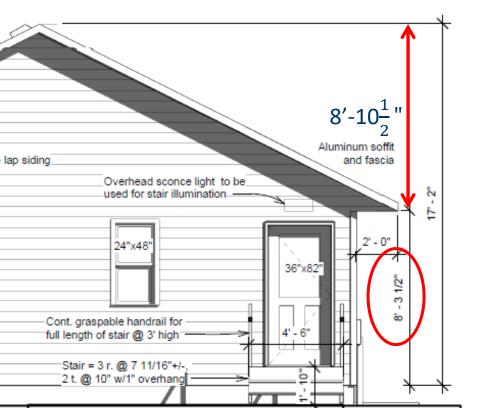
1. All buildings in Seismic Design Categories A and B shall use Table R602.10.3(1) and the applicable adjustment factors in Table R602.10.3(2).

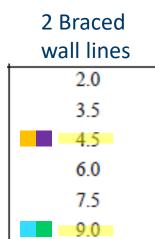
EXPOSURE C/     30-FOOT MEAI     10-FOOT WAL     2 BRACED WA	IN ROOF HEIGHT		MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE*						
Ultimate Design Wind Speed (mph)	Story Location	Braced Wall Line Spacing <sup>e</sup> (feet)	Method LIB <sup>b</sup>	Method GB	Methods DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP, ABW, PFH, PFC, CS-SFB	Methods <u>CS-WSP.</u> CS-G, CS-PF			
	/	10	3.5	3.5	2.0	2.0			
		20	6.5	6.5	3.5	3.5			
		30	9.5	9.5	5.5	4.5			
		40	12.5	12.5	7.0	6.0			
		50	15.0	15.0	9.0	7.5			
		60	18.0	18.0	10.5	9.0			
		10	7.0	7.0	4.0	3.5			
	$\wedge$	20	12.5	12.5	7.5	6.5			
≤115	$  \land \Pi  $	30	18.0	18.0	10.5	9.0			
2115		40	23.5	23.5	13.5	11.5			
	1 🔲 🗌 '	50	29.0	29.0	16.5	14.0			
		60	34.5	34.5	20.0	17.0			
		10	NP	10.0	6.0	5.0			
	$\land$	20	NP	18.5	11.0	9.0			
		30	NP	27.0	15.5	13.0			
		40	NP	35.0	20.0	17.0			
		50	NP	43.0	24.5	21.0			
	'	60	NP	51.0	29.0	25.0			

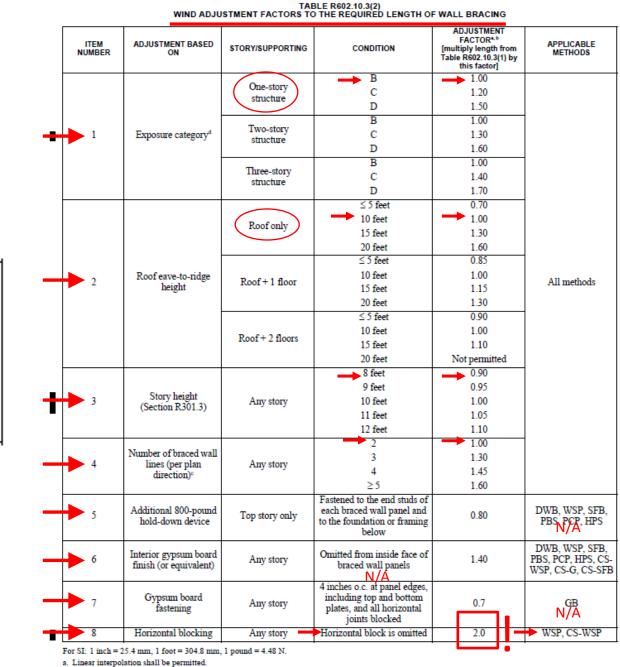
#### TABLE R602.10.3(1) BRACING REQUIREMENTS BASED ON WIND SPEED



1. All buildings in Seismic Design Categories A and B shall use Table R602.10.3(1) and the applicable adjustment factors in Table R602.10.3(2).



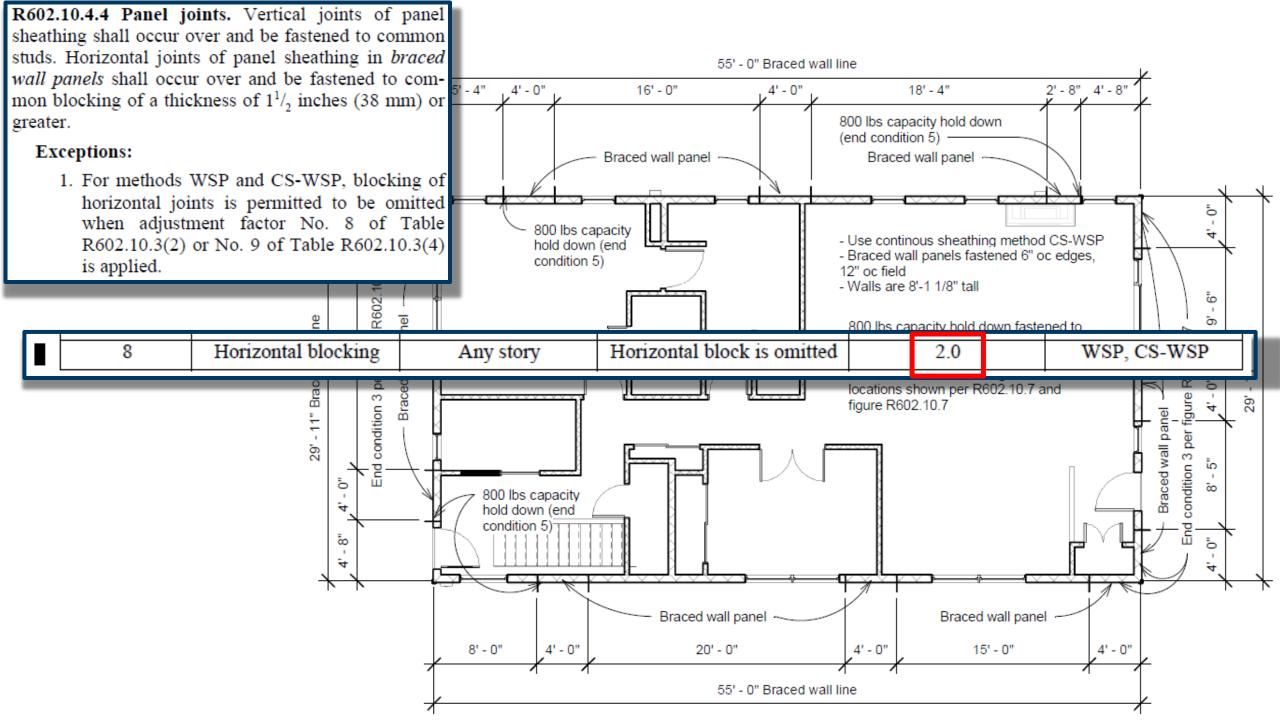


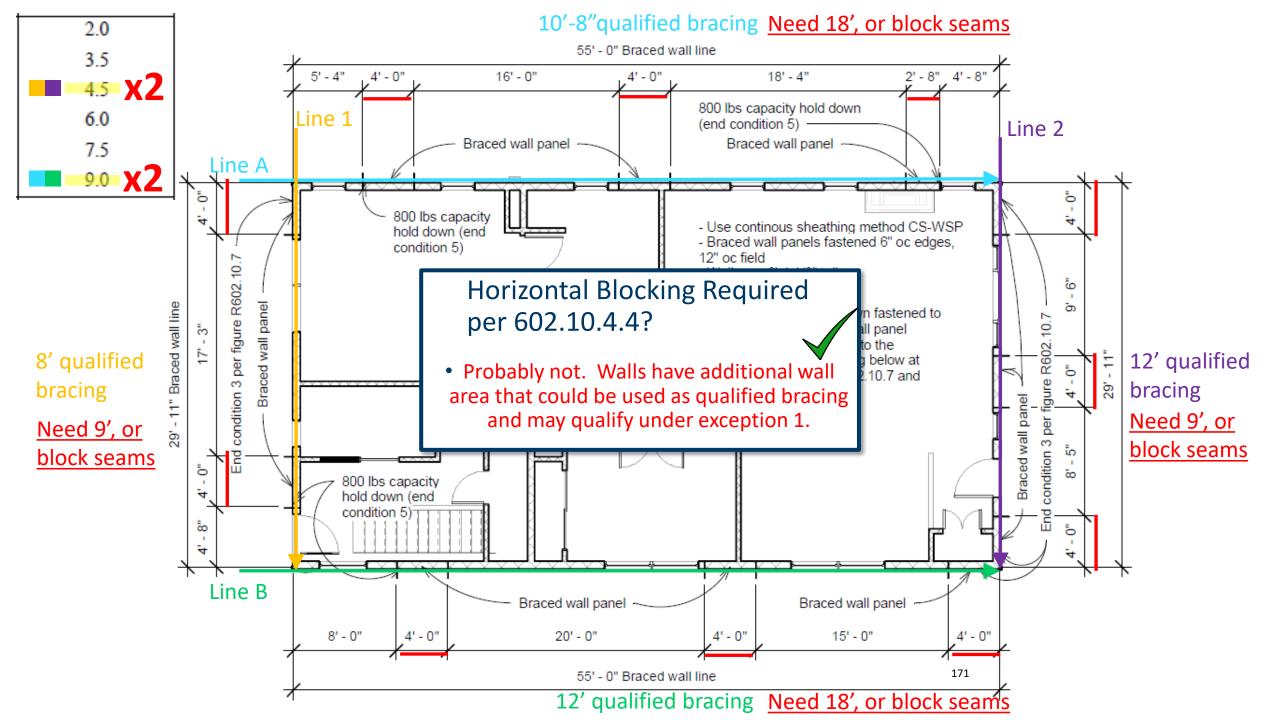


b. The total adjustment factor is the product of all applicable adjustment factors.

c. The adjustment factor is permitted to be 1.0 when determining bracing amounts for intermediate braced wall lines provided the bracing amounts on adjacent braced wall lines are based on a spacing and number that neglects the intermediate braced wall line.

d. The same adjustment factor shall be applied to all braced wall lines on all floors of the structure, based on the worst-case exposure category.





**R602.10.5 Minimum length of a braced wall panel.** The minimum length of a *braced wall panel* shall comply with Table R602.10.5. For Methods CS-WSP and CS-SFB, the minimum panel length shall be based on the adjacent clear opening height in accordance with Table R602.10.5 and Figure R602.10.5. Where a panel has an opening on either side of differing heights, the taller opening height shall be used to determine the panel length.

**R602.10.5.1 Contributing length.** For purposes of computing the required length of bracing in Tables R602.10.3(1) and R602.10.3(3), the contributing length of each *braced wall panel* shall be as specified in Table R602.10.5.

#### TABLE R602.10.5 MINIMUM LENGTH OF BRACED WALL PANELS

		М	NIMUM LEN (inches)			CONTRIBUTING LENGTH		
	METHOD able R602.10.4)			Wall Heig	ht		(inches)	
		8 feet	9 feet	10 feet	11 feet	12 feet		
DWB, WSP, SFB,	PBS, PCP, HPS, BV-WSP	48	48	48	53	58	Actual <sup>b</sup>	
GB		48	48	48	53	58	Double sided = Actual Single sided = 0.5 × Actua	
	LIB	55	62	69	NP	NP	Actual <sup>b</sup>	
ABW	SDC A, B and C, ultimate design wind speed < 140 mph	28	32	34	38	42	48	
	SDC D <sub>0</sub> , D <sub>1</sub> and D <sub>2</sub> , ultimate design wind speed < 140 mph	32	32	34	NP	NP		
	ĆS-G		27	30	33	36	Actual <sup>b</sup>	
	Adjacent clear opening height (inches)							
	≤ 64	24	27	30	33	36		
	68	26	27	30	33	36	1	
	72	27	27	30	33	36	1	
	76	30	29	30	33	36		
	80	32	30	30	33	36		
	84	35	32	32	33	36		
	88	38	35	33	33	36		
	92	43	37	35	35	36	- - -	
	96	48	41	38	36	36		
CS-WSP, CS-SFB	100		44	40	38	38		
	104	_	49	43	40	39	Actual <sup>b</sup>	
	108	_	54	46	43	41		
	112	_	—	50	45	43		
	116	_	—	55	48	45		
	120	_	—	60	52	48		
	124	-	—	—	56	51		
	128		—	—	61	54		
	132	_	—	—	66	58		
	136	_	—	—	—	62		
	140	_	_	_	_	66		
	144	_	_	_	_	72		
-	IETHOD			rtal header l	<u> </u>			
(See Ta	ble R602.10.4)	8 feet	9 feet	10 feet	11 feet	12 feet		
PFH	Supporting roof only	16	16	16	Note c	Note c	48	
	Supporting one story and roof	24	24	24	Note c	Note c	15 A th	
	PFG	24	27	30	Note d	Note d	1.5 × Actual <sup>b</sup>	
CS-PF	SDC A, B and C	16	18	20	Note e	Note e	1.5 × Actual <sup>b</sup>	
	SDC $D_0$ , $D_1$ and $D_2$	16	18	20	Note e	Note e	Actual <sup>b</sup>	

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s.

NP = Not Permitted.

a. Linear interpolation shall be permitted.

b. Use the actual length where it is greater than or equal to the minimum length.

c. Maximum header height for PFH is 10 feet in accordance with Figure R602.10.6.2, but wall height shall be permitted to be increased to 12 feet with pony wall.

d. Maximum header height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height shall be permitted to be increased to 12 feet with pony wall.

e. Maximum header height for CS-PF is 10 feet in accordance with Figure R602.10.6.4, but wall height shall be permitted to be increased to 12 feet with pony wall.

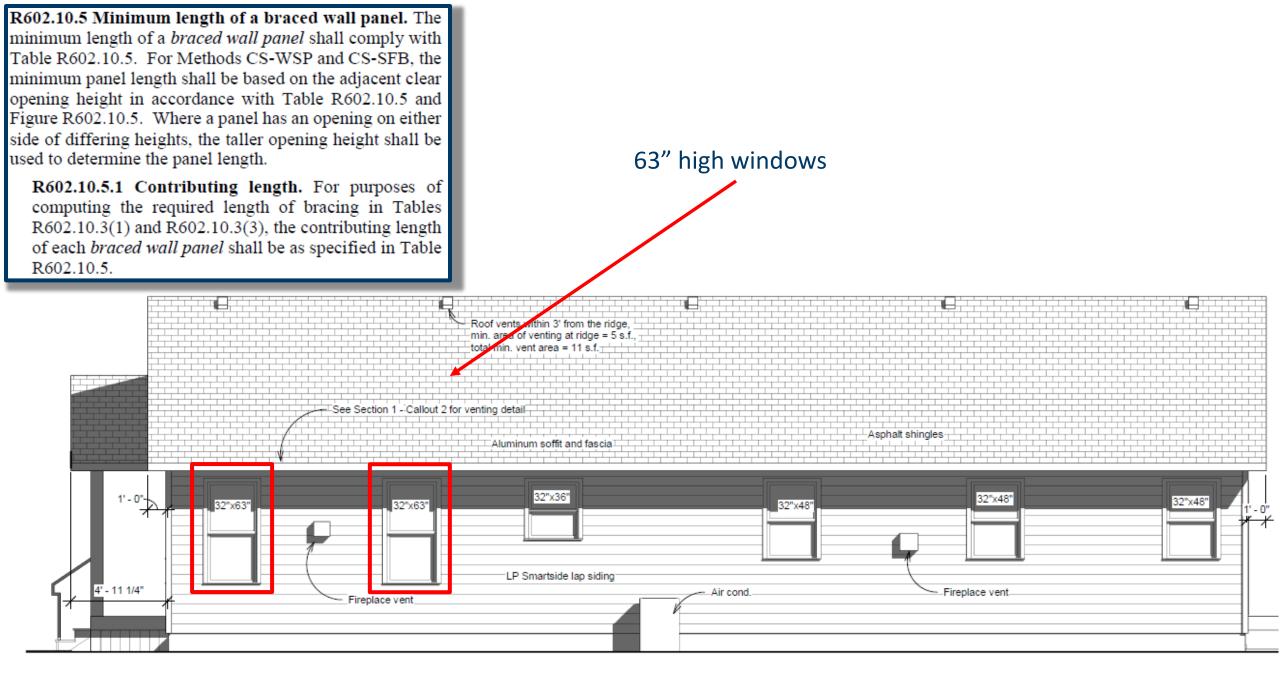
**R602.10.5 Minimum length of a braced wall panel.** The minimum length of a *braced wall panel* shall comply with Table R602.10.5. For Methods CS-WSP and CS-SFB, the minimum panel length shall be based on the adjacent clear opening height in accordance with Table R602.10.5 and Figure R602.10.5. Where a panel has an opening on either side of differing heights, the taller opening height shall be used to determine the panel length.

**R602.10.5.1 Contributing length.** For purposes of computing the required length of bracing in Tables R602.10.3(1) and R602.10.3(3), the contributing length of each *braced wall panel* shall be as specified in Table R602.10.5.

CS-W

MINIMUM LE	TABLE R602.10.5 ENGTH OF BRACED WALL PANELS	
		1

clear and	METHOD (See Table R602.10.4)			MI	NIMUM LEN (inches)		- CONTRIBUTING LENGTH	
ither					Wall Heigh	nt		(inches)
all be				9 feet	10 feet	11 feet	12 feet	-
es of ables		Adjacent clear opening height (inches)						
ength		≤ 64	24	27	30	33	36	
[able		68	26	27	30	33	36	
		72	27	27	30	33	36	
		76	30	29	30	33	36	
		80	32	30	30	33	36	
		84	35	32	32	33	36	
		88	38	35	33	33	36	
		92	43	37	35	35	36	
		96	48	41	38	36	36	
SP, CS	-SFB	100	—	44	40	38	38	
		104	—	49	43	40	39	Actual <sup>b</sup>
		108	—	54	46	43	41	
		112	_	_	50	45	43	
		116	—	_	55	48	45	
		120	—	_	60	52	48	
		124	—	_	—	56	51	
		128	—	_	—	61	54	
		132	—	_	—	66	58	
		136	—	_	—	_	62	
		140	—	_	—	_	66	
		144	—	_	—	_	72	

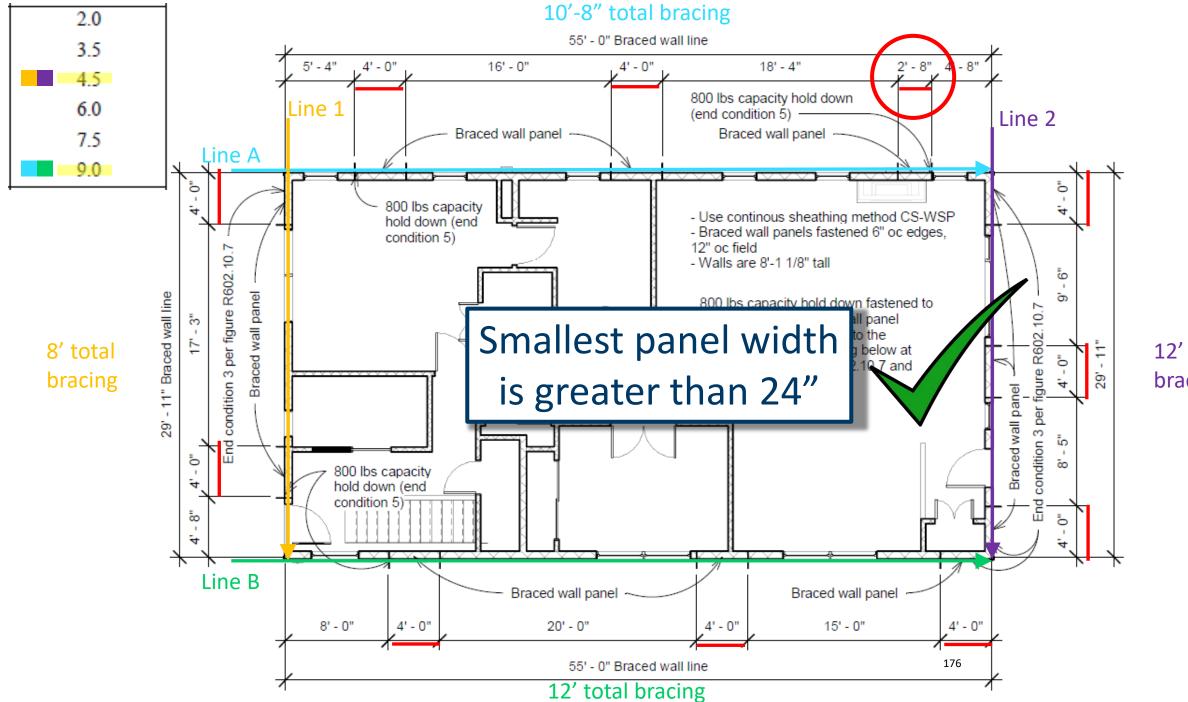


5 North 1/4" = 1'-0" R602.10.5 Minimum length of a braced wall panel. The minimum length of a braced wall panel shall comply with Table R602.10.5. For Methods CS-WSP and CS-SFB, the minimum panel length shall be based on the adjacent clear opening height in accordance with Table R602.10.5 and Figure R602.10.5. Where a panel has an opening on either (See Table side of differing heights, the taller opening height shall used to determine the panel length.

R602.10.5.1 Contributing length. For purposes computing the required length of bracing in Tabl R602.10.3(1) and R602.10.3(3), the contributing leng of each braced wall panel shall be as specified in Tab R602.10.5.

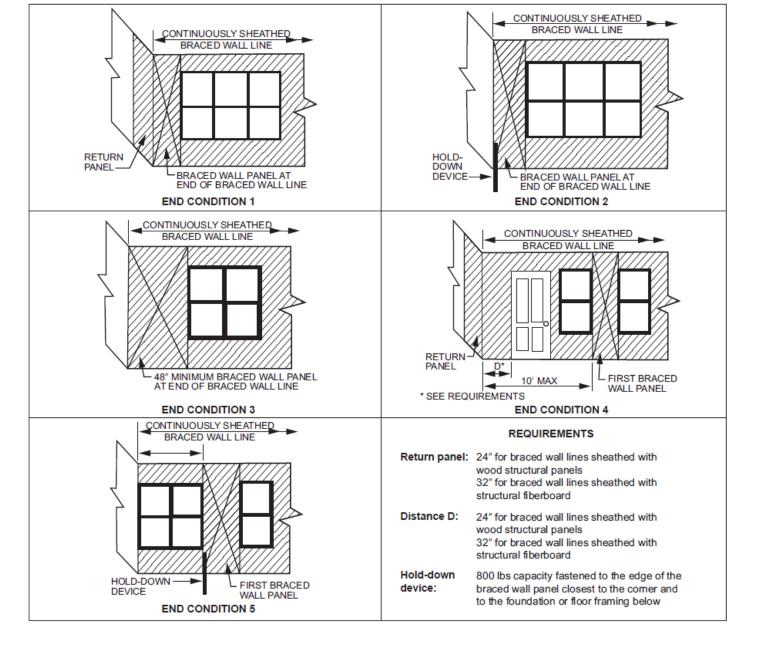
	MINIMUM LI		R602.10. BRACED		NELS		
MF	THOD		МІ	CONTRIBUTING LENGTH			
	le R602.10.4)			(inches)			
		8 feet	9 feet	10 feet	11 feet	12 feet	
	Adjacent clear opening height (inches)						
	≤ 64	24	27	30	33	36	
	68	26	27	30	33	36	
	72	27	27	30	33	36	
	76	30	29	30	33	36	
	80	32	30	30	33	36	
	84	35	32	32	33	36	
	<u> </u>	38	35	33	33	36	•

ing on enner			_						
eight shall be		8 feet	9 feet	10 feet	11 feet	12 feet			
								-1	
purposes of	Adjacent clear opening height (inches)								
ng in Tables		24	27	20	22	26			
buting length	≤ 64	24	27	30	33	36			
fied in Table	68	26	27	30	33	36			
	72	27	27	30	33	36			
	76	30	29	30	33	36			
	80	32	30	30	33	36			
	84	35	32	32	33	36	ĺ		
	88	38	35	33	33	36	ĺ		
	92	43	37	35	35	36	ĺ		
	96	48	41	38	36	36			
CS-WSP, CS-SFB	100	—	44	40	38	38			
	104	_	49	43	40	39		Actual <sup>b</sup>	
	108	_	54	46	43	41	Í		
	112	_	—	50	45	43	[		
	116	_	_	55	48	45	[		
	120	_	_	60	52	48			
	124	_	_	—	56	51			
	128	_	_	_	61	54	[		
	132	_	_	_	66	58	[		
	136	_	_	_	_	62	[		
	140	_	_	_	_	66			
	144	_	—	—	—	72			
				-					

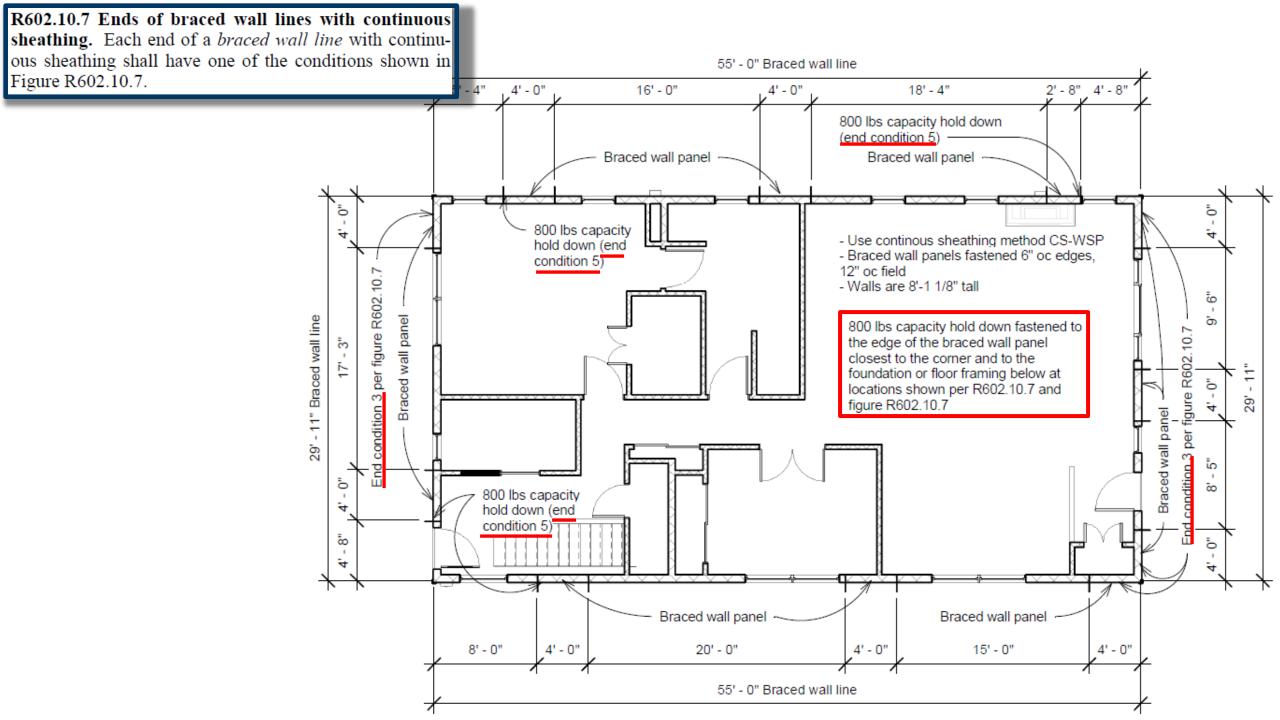


12' total bracing

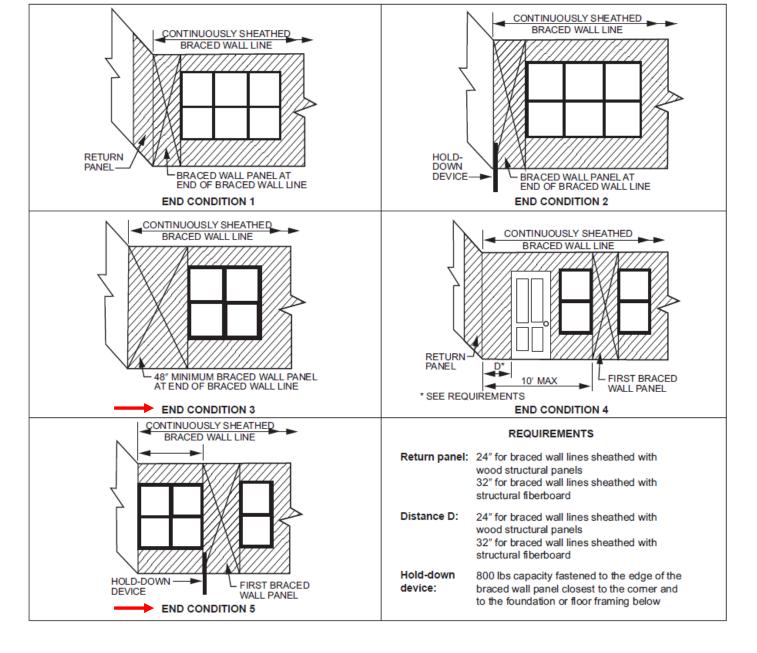
**R602.10.7 Ends of braced wall lines with continuous sheathing.** Each end of a *braced wall line* with continuous sheathing shall have one of the conditions shown in Figure R602.10.7.



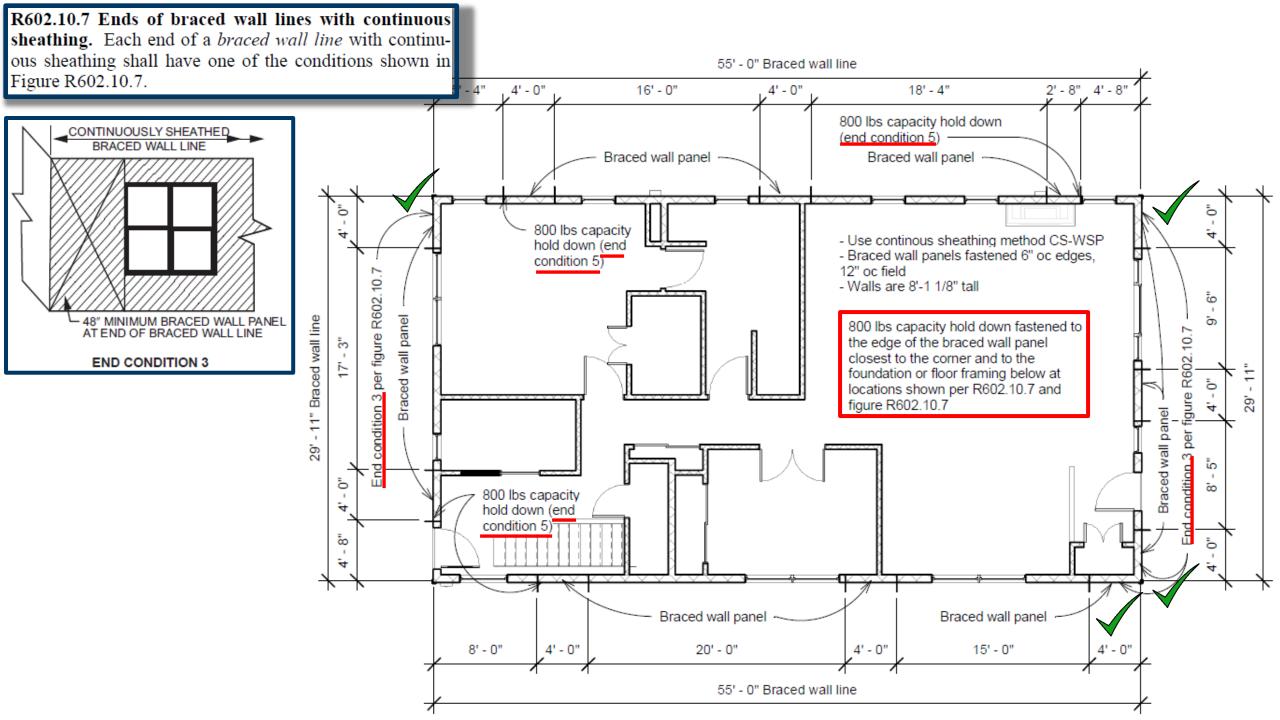
For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound = 4.45 N.

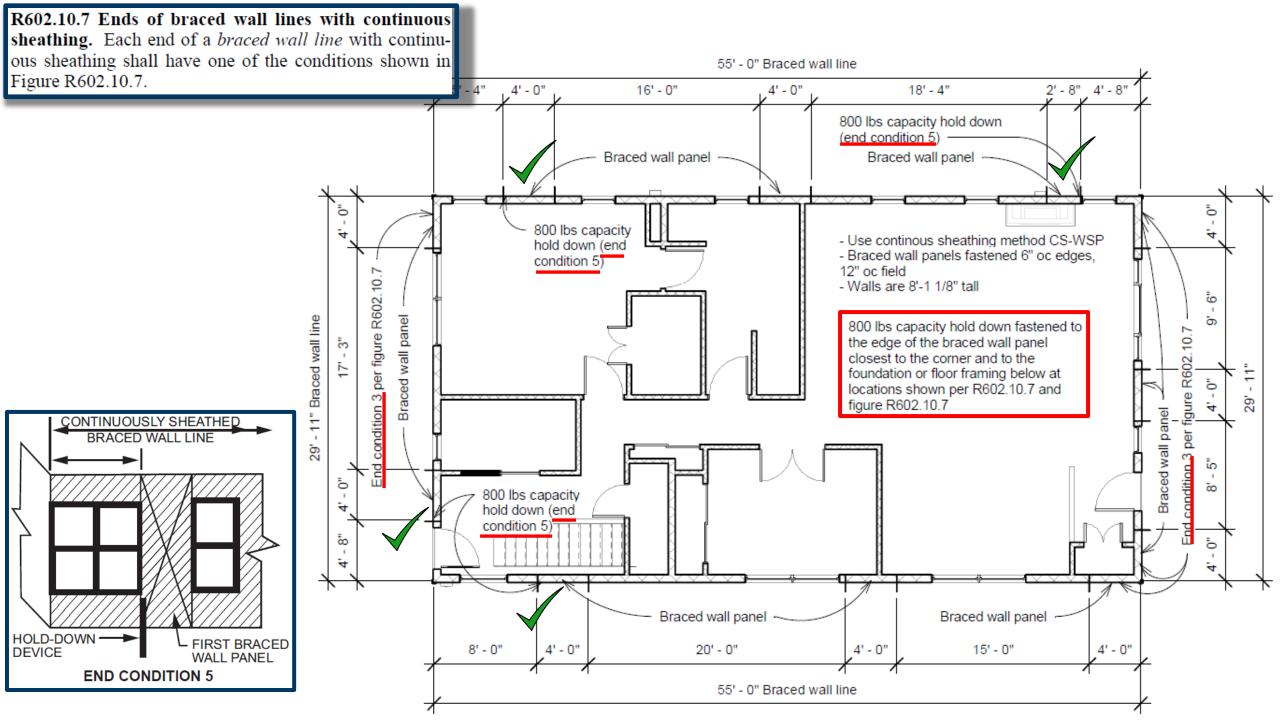


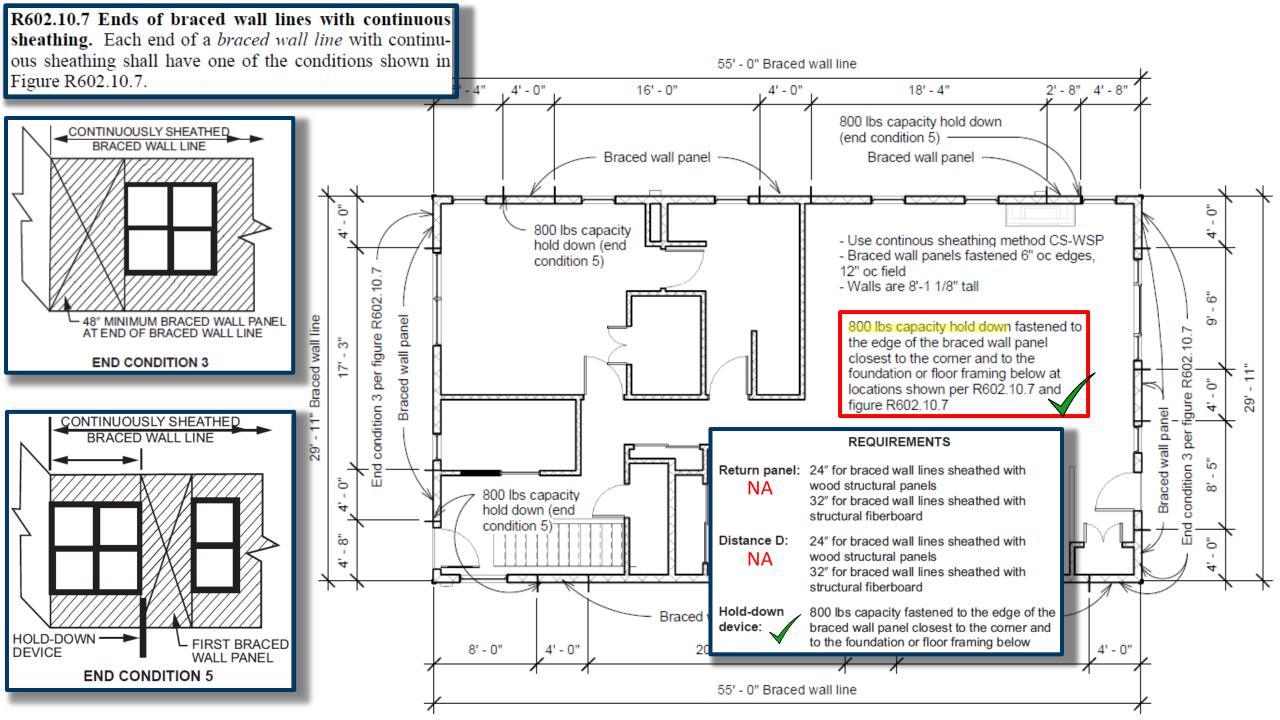
**R602.10.7 Ends of braced wall lines with continuous sheathing.** Each end of a *braced wall line* with continuous sheathing shall have one of the conditions shown in Figure R602.10.7.



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound = 4.45 N.



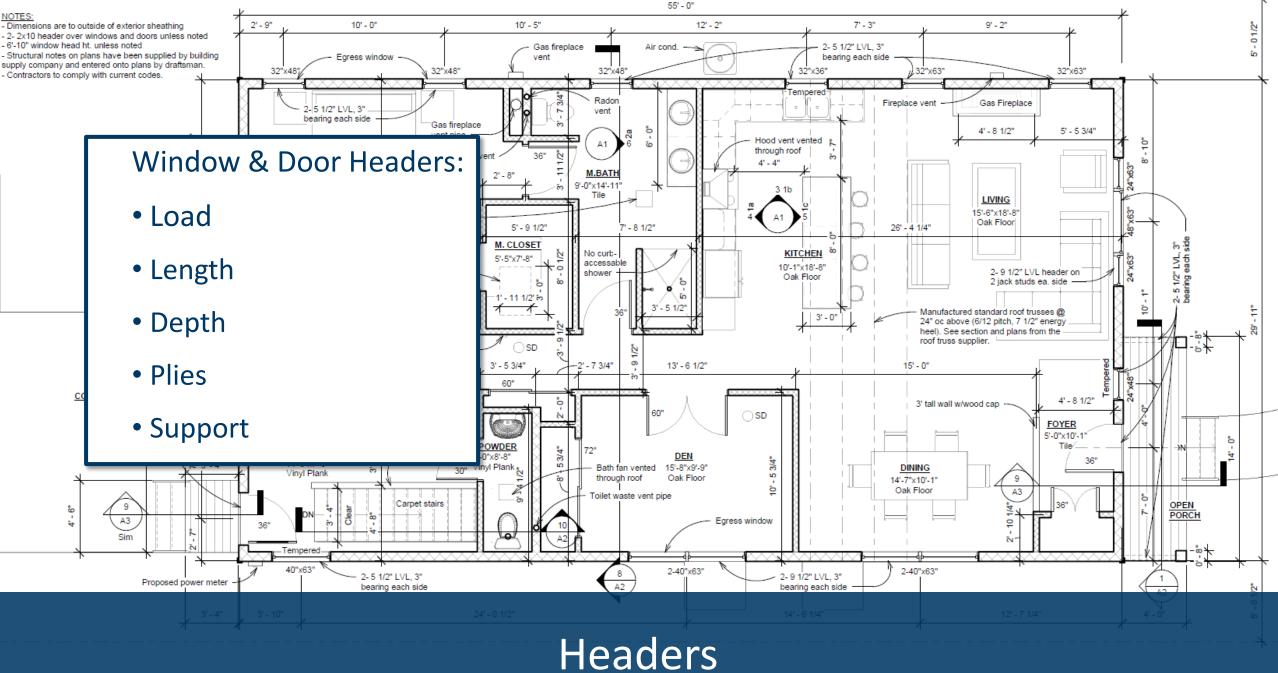




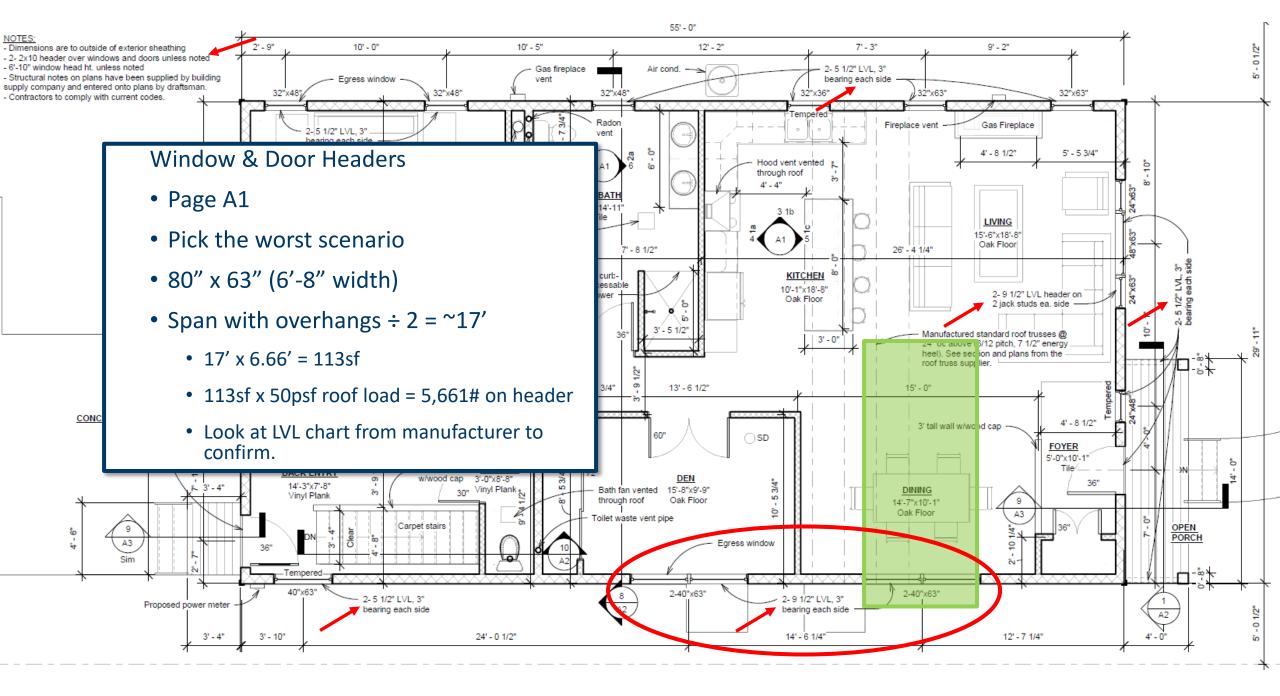


# Headers, Beams, Girders – Chapter 6

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2 Main Level 1/4" = 1'-0'



<u>Main Level</u> 1/4" = 1'-0"

### Window & Door Headers:

- Load
- Length
- Depth
- Plies
- Support

		L							GROUI	ND SNO		vn (psi)							
GIRDERS AND				3	0					-	0					1	70		
HEADERS SUPPORTING	SIZE	L	2	1 1						uilding width <sup>e</sup> ( 24			~	12				· ·	
		Span	2	2 Span	4	Span	6	Span	2	2 Span		3 Span	6	Span		∠ Span	4	Span	6
		apan 1	NJª	apan 1	NJ <sup>d</sup>	apan 1	NJª	apan 1	NJ <sup>d</sup>	apan 1	NJ <sup>d</sup>	apan 1	NJ <sup>d</sup>	apan 1	NJª	apan 1	NJ <sup>d</sup>	apan 1	NJ
	1-2 × 6	4-0	1	3-1	2	2-7	2	3-5	1	2-8	2	2-3	2	3-0	2	2-4	2	2-0	2
	1-2 × 8	5-1	2	3-11	2	3-3	2	4-4	2	3-4	2	2-10	2	3-10	2	3-0	2	2-6	3
	1-2 × 10	6-0	2	4-8	2	3-11	2	5-2	2	4-0	2	3-4	3	4-7	2	3-6	3	3-0	3
	$1-2 \times 12$	7-1	2	5-5	2	4-7	3	6-1	2	4-8	3	3-11	3	5-5	2	4-2	3	3-6	3
	2-2 × 4	4-0	1	3-1	1	2-7	1	3-5	1	2-7	1	2-2	1	3-0	1	2-4	1	2-0	1
	2-2 × 6	6-0	1	4-7	1	3-10	1	5-1	1	3-11	1	3-3	2	4-6	1	3-6	2	2-11	2
HEADER,	2-2 × 8	7-7	1	5-9	1	4-10	2	6-5	1	5-0	2	4-2	2	5-9	1	4-5	2	3-9	2
	2-2 × 10	9-0	1	6-10	2	5-9	2	7-8	2	5-11	2	4-11	2	6-9	2	5-3	2	4-5	2
	2-2 × 12	10-7	2	8-1	2	6-10	2	9-0	2	6-11	2	5-10	2	8-0	2	6-2	2	5-2	3
ROOF AND CELING	3-2 × 8	9-5	1	7-3	1	6-1	1	8-1	1	6-3	1	5-3	2	7-2	1	5-6	2	4-8	2
	3-2 × 10	11-3	1	8-7	1	7-3	2	9-7	1	7-4	2	6-2	2	8-6	1	6-7	2	5-6	2
	3-2 × 12	13-2	1	10-1	2	8-6	2	11-3	2	8-8	2	7-4	2	10-0	2	7-9	2	6-6	2
	4-2 × 8	10- 11	1	8-4	1	7-0	1	9-4	1	7-2	1	6-0	1	8-3	1	6-4	1	5-4	2
	4-2 × 10	12- 11	1	9-11	1	8-4	1	11-1	1	8-6	1	7-2	2	9-10	1	7-7	2	6-4	2
	4-2 × 12	15-3	1	11-8	1	9-10	2	13-0	1	10-0	2	8-5	2	11-7	1	8-11	2	7-6	2
	1-2 × 6	3-3	1	2-7	2	2-2	2	3-0	2	2-4	2	2-0	2	2-9	2	2-2	2	1-10	2
	1-2 × 8	4-1	2	3-3	2	2-9	2	3-9	2	3-0	2	2-6	3	3-6	2	2-9	2	2-4	3
	$1-2 \times 10$	4-11	2	3-10	2	3-3	3	4-6	2	3-6	3	3-0	3	4-1	2	3-3	3	2-9	3
	$1-2 \times 12$	5-9	2	4-6	3	3-10	3	5-3	2	4-2	3	3-6	3	4-10	3	3-10	3	3-3	4
	2-2 × 4	3-3	1	2-6	1	2-2	1	3-0	1	2-4	1	2-0	1	2-8	1	2-2	1	1-10	1
	2-2 × 6	4-10	1	3-9	1	3-3	2	4-5	1	3-6	2	3-0	2	4-1	1	3-3	2	2-9	2
Poof coiling	2-2 × 8	6-1	1	4-10	2	4-1	2	5-7	2	4-5	2	3-9	2	5-2	2	4-1	2	3-6	2
$\frown$	2-2 × 10	7-3	2	5-8	2	4-10	2	6-8	2	5-3	2	4-5	2	6-1	2	4-10	2	4-1	2
ſ ]	2-2 × 12	8-6	2	6-8	2	5-8	2	7-10	2	6-2	2	5-3	3	7-2	2	5-8	2	4-10	3
	3-2 × 8	7-8	1	6-0	1	5-1	2	7-0	1	5-6	2	4-8	2	6-5	1	5-1	2	4-4	2
	3-2 × 10	9-1	1	7-2	2	6-1	2	8-4	1	6-7	2	5-7	2	7-8	2	6-1	2	5-2	2
ROOF, CELING AND ONE FLOOR	3-2 × 12	10-8	2	8-5	2	7-2	2	9-10	2	7-8	2	6-7	2	9-0	2	7-1	2	6-1	2
(CENTER BEARING)	4-2 × 8	8-10	1	6-11	1	5-11	1	8-1	1	6-4	1	5-5	2	7-5	1	5-11	1	5-0	2
	4-2 × 10	10-6	1	8-3	2	7-0	2	9-8	1	7-7	2	6-5	2	8-10	1	7-0	2	6-0	2
	4-2 × 12	12-4	1	9-8	2	8-3	2	11-4	2	8-11	2	7-7	2	10-4	2	8-3	2	7-0	2
	1-2 × 6	2-11	2	2-3	2	1-11	2	2-9	2	2-1	2	1-9	2	2-7	2	2-0	2	1-8	2
	1-2 × 8	3-9	2	2-10	2	2-5	3	3-6	2	2-8	2	2-3	3	3-3	2	2-6	3	2 -2	3
	1-2 × 10	4-5	2	3-5	3	2-10	3	4-2	2	3-2	3	2-8	3	3-11	2	3-0	3	2-6	3
	$1-2 \times 12$	5-2	2	4-0	3	3-4	3	4-10	3	3-9	3	3-2	4	4-7	3	3-6	3	3-0	4
	2-2 × 4	2-11	1	2-3	1	1-10	1	2-9	1	2-1	1	1-9	1	2-7	1	2-0	1	1-8	1
	2-2 × 6	4-4	1	3-4	2	2-10	2	4-1	1	3-2	2	2-8	2	3-10	1	3-0	2	2-6	2
	$2-2 \times 8$	5-6	2	4-3	2	3-7	2	5-2	2	4-0	2	3-4	2	4-10	2	3-0	2	3-2	2

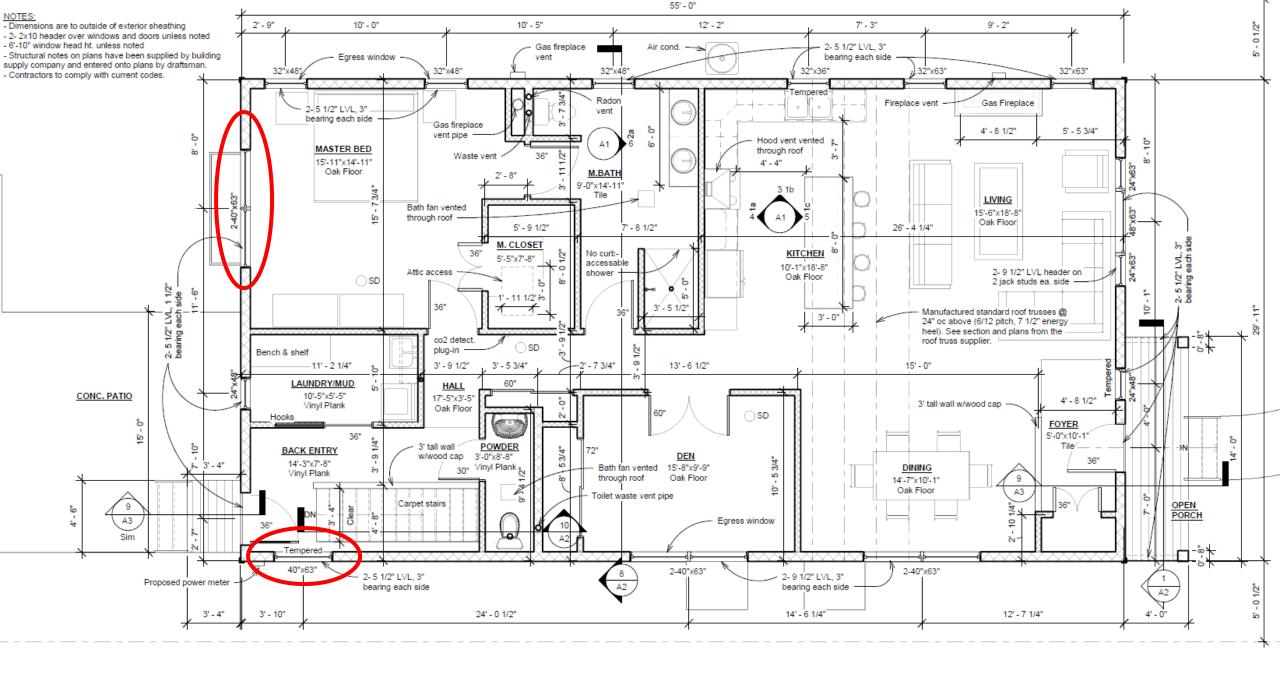
TABLE R602.7(1) GIRDER SPANS\* AND HEADER SPANS\* FOR EXTERIOR BEARING WALLS

# Headers – Dimensional Lumber

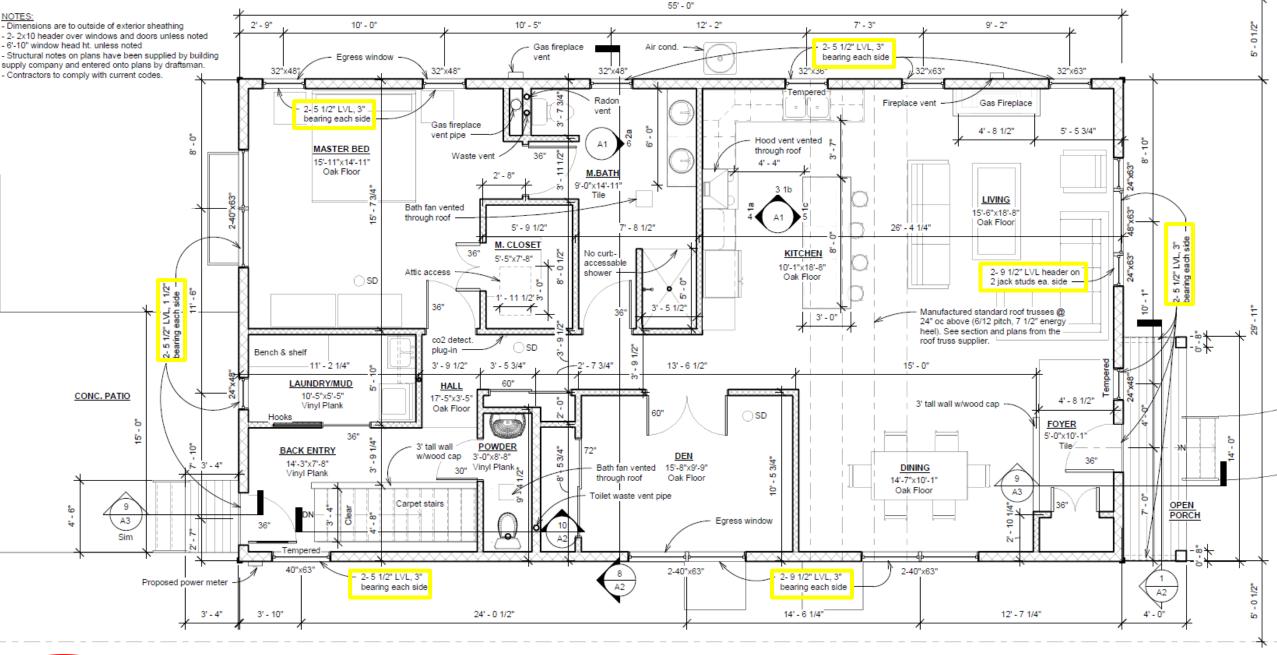
#### TABLE R602.7(1) GIRDER SPANS\* AND HEADER SPANS\* FOR EXTERIOR BEARING WALLS (Maximum spans for Douglas fir-larch, hem-fir, Southern pine and spruce-pine-fir<sup>b</sup> and required number of jack studs)

									GROUM	ID SNO	W LOA	D (psf)	•							
GIRDERS AND		30					50						Window & Door Headers:							
HEADERS	SIZE						Building width <sup>e</sup> (feet)													
SUPPORTING		12 24				1	12		24		36									
		Span	NJª	Span r	NJ⁴	Span	NJ₫	Span f	NJ₫	Span	NJ <sup>a</sup>	Span f	NJª		• 3(	b' IS	clos	e to	32'	
	1-2 × 6	4-0	1	3-1	2	2-7	2	3-5	1	2-8	2	2-3	2		(1	nter	pola	ntion	i pei	rmitted)
	1-2 × 8	5-1	2	3-11	2	3-3	2	4-4	2	3-4	2	2-10	2							
	$1-2 \times 10$	6-0	2	4-8	2	3-11	2	5-2	2	4-0	2	3-4	3	1	• 3-	-2x1	0 or	3-2	x12	
	$1-2 \times 12$	7-1	2	5-5	2	4-7	3	6-1	2	4-8	3	3-11	3	1						
	2-2 × 4	4-0	1	3-1	1	2-7	1	3-5	1	2-7	1	2-2	1		<ul> <li>Min 2 jack studs required</li> </ul>					
	2-2 × 6	6-0	1	4-7	1	3-10	1	5-1	1	3-11	1	3-3	$\frac{2}{-}$ - EACH SIDE					•		
HEADER,	2-2 × 8	7-7	1	5-9	1	4-10	2	6-5	1	5-0	2	4-2	2		_	LAC	11 31	DL		
	2-2 × 10	9-0	1	6-10	2	5-9	2	7-8	2	5-11	2	4-11	2							
	$2 - 2 \times 12$	10-7	2	8-1	2	6-10	2	9-0	2	6-11	2	5-10	2	8-0	2	6-2	2	5-2	3	
ROOF AND CELING	3-2 × 8	9-5	1	7-3	1	6-1	1	8-1	1	6-3	1	5-3	2	7-2	1	5-6	2	4-8	2	
<b></b>	3-2 × 10	11-3	1	8-7	1	7-3	2	9-7	1	7-4	2	6-2	2	8-6	1	6-7	2	5-6	2	
	3-2 × 12	13-2	1	10-1	2	8-6	2	11-3	2	8-8	2	7-4	2	10-0	2	7-9	2	6-6	2	
	4-2 × 8	10- 11	1	8-4	1	7-0	1	9-4	1	7-2	1	6-0	1	8-3	1	6-4	1	5-4	2	
	4-2 × 10	12- 11	1	9-11	1	8-4	1	11-1	1	8-6	1	7-2	2	9-10	1	7-7	2	6-4	2	
	4-2 × 12	15-3	1	11-8	1	9-10	2	13-0	1	10-0	2	8-5	2	11-7	1	8-11	2	7-6	2	

# Headers – Dimensional Lumber

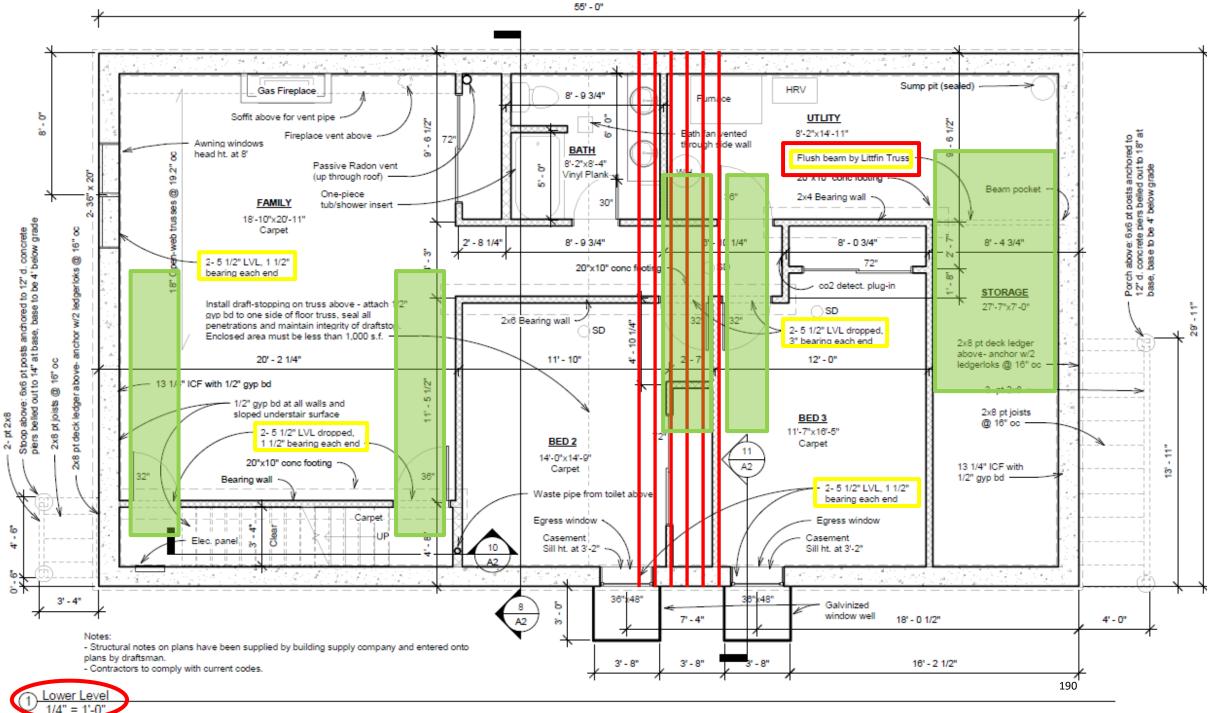


2 Main Level 1/4" = 1'-0"



2 Main Level 1/4" = 1'-0"

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NOTES: - Dimensions are to outside of exterior sheathing - 2- 2x10 header over windows and doors unless no

- 6'-10" w

Structura

supply cor

Contract

### SECTION R407 COLUMNS

10' - 0"

**R407.1 Wood column protection.** Wood columns shall be protected against decay as set forth in Section R317.

**R407.2 Steel column protection.** All surfaces (inside and outside) of steel columns shall be given a shop coat of rust-inhibitive paint, except for corrosion-resistant steel and steel treated with coatings to provide corrosion resistance.

**R407.3 Structural requirements.** The columns shall be restrained to prevent lateral displacement at the bottom end. Wood columns shall be not less in nominal size than 4 inches by 4 inches (102 mm by 102 mm). Steel columns shall be not less than 3-inch-diameter (76 mm) Schedule 40 pipe manufactured in accordance with ASTM A53 Grade B or *approved* equivalent.

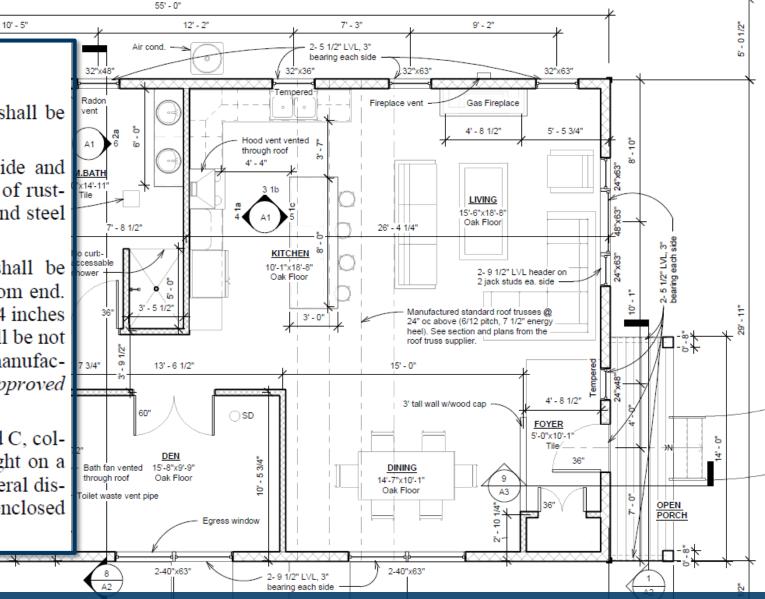
**Exception:** In Seismic Design Categories A, B and C, columns not more than 48 inches (1219 mm) in height on a pier or footing are exempt from the bottom end lateral displacement requirement within under-floor areas enclosed by a continuous foundation.

2-5 1/2" LVL, 3"

bearing each side

40"x63

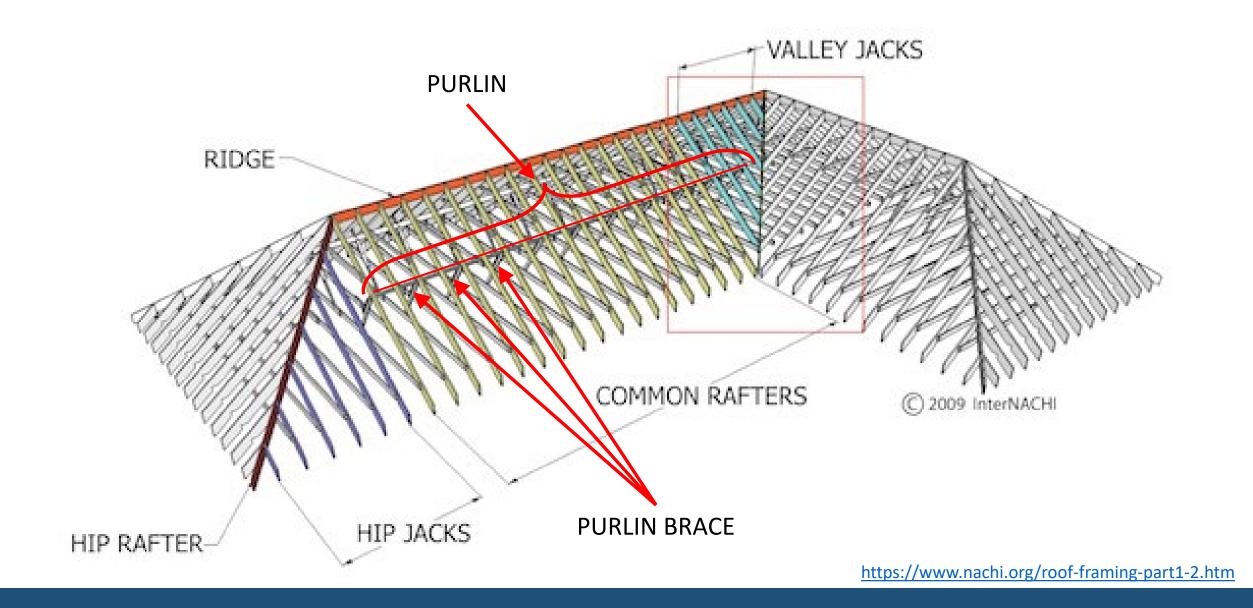
Proposed power met



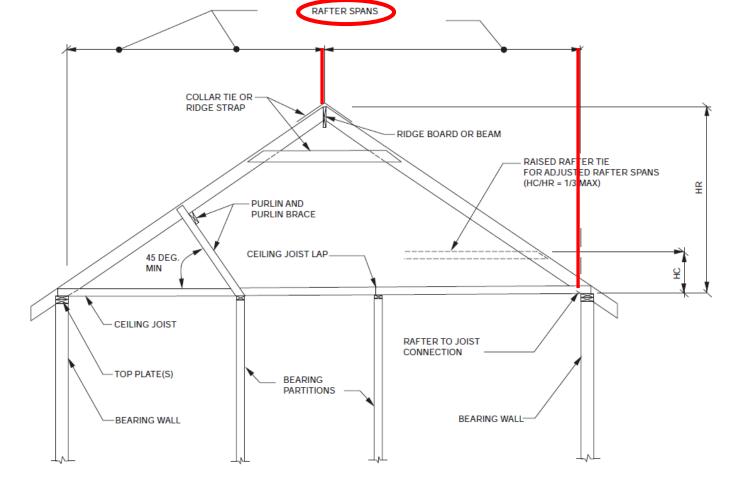
# Columns – Chapter 4



# Roof/Ceiling Construction – Chapter 8



# Terminology



For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 degree = 0.018 rad.

 $H_c$  = Height of ceiling joists or rafter ties measured vertically above the top of rafter support walls.

 $H_R$  = Height of roof ridge measured vertically above the top of the rafter support walls.

#### FIGURE R802.4.5 BRACED RAFTER CONSTRUCTION

# Terminology

R802.4 Rafters. Rafters shall be in accordance with this section.

**R802.4.1 Rafter size.** Rafters shall be sized based on the rafter spans in Tables R802.4.1(1) through R802.4.1(8). Rafter spans shall be measured along the horizontal projection of the rafter. For other grades and species and for other loading conditions, refer to the AWC STJR.

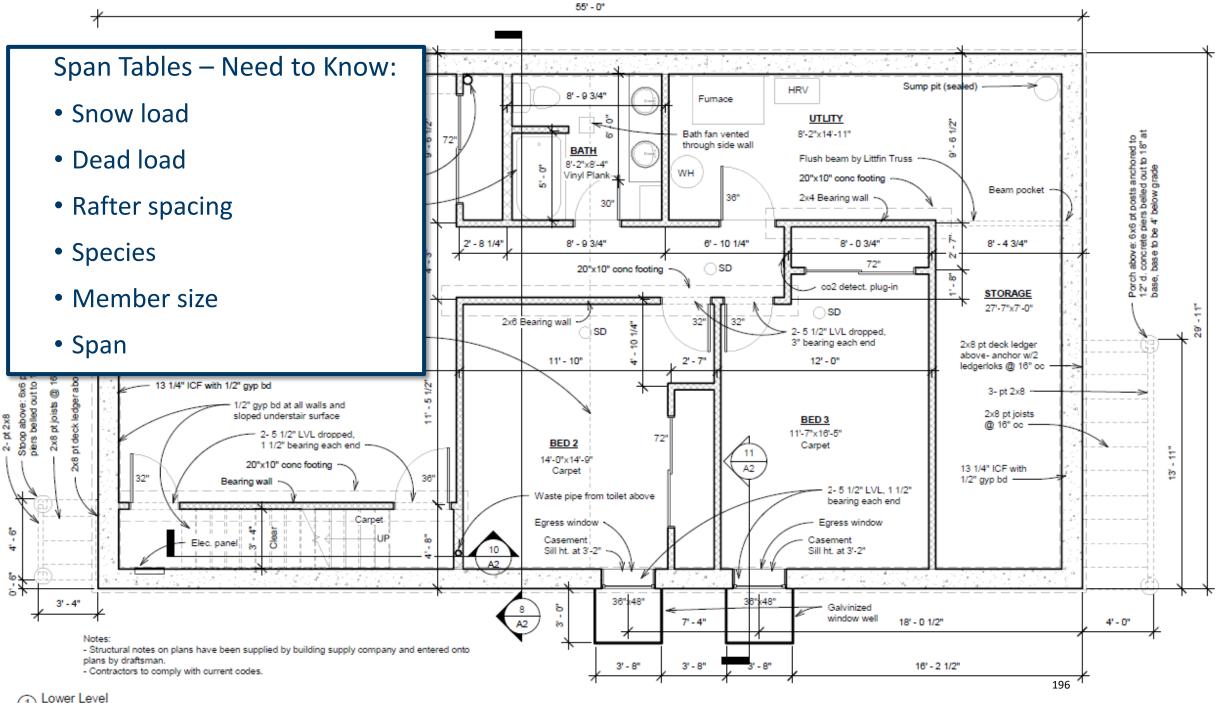
**R802.4.2 Framing details.** Rafters shall be framed not more than  $1^{1/2}$  inches (38 mm) offset from each other to a ridge board or directly opposite from each other with a collar tie, gusset plate or ridge strap in accordance with Table R602.3(1). Rafters shall be nailed to the top wall plates in accordance with Table R602.3(1) unless the roof assembly is required to comply with the uplift requirements of Section R802.11.

### Span Tables – Need to Know:

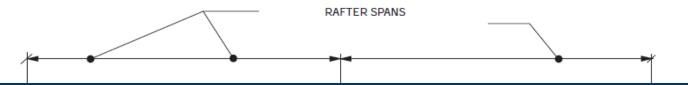
- Snow load
- Dead load
- Rafter spacing
- Species
- Member size
- Span

#### TABLE R802.4.1(6) RAFTER SPANS FOR COMMON LUMBER SPECIES (Ground snow load = 50 psf, ceiling attached to rafters, $L/\Delta$ = 240)

		1	10			D LOAD = 1	-	neu to ran			D LOAD = 2	0 psf	,
	RAFTER			2×4	2×6	2×8	2 × 10	2 × 12	2×4	2×6	2×8	2 × 10	2 × 12
	SPACING	SPECIES AND GR	ADE					Maximum r					
	(inches)			(feet- inches)									
		Douglas fir-larch	SS	7-8	12-1	15-11	20-3	24-8	7-8	12-1	15-11	20-3	24-5
		Douglas fir-larch	#1	7-5	11-7	15-3	18-7	21-7	7-5	11-2	14-1	17-3	20-0
		Douglas fir-larch	#2	7-3	11-5	14-5	17-8	20-5	7-3	10-7	13-4	16-4	18-11
		Douglas fir-larch	#3	6-0	8-9	11-0	13-6	15-7	5-6	8-1	10-3	12-6	14-6
		Hem-fir	SS	7-3	11-5	15-0	19-2	23-4	7-3	11-5	15-0	19-2	23-4
		Hem-fir	#1	7-1	11-2	14-8	18-4	21-3	7-1	11-0	13-11	17-0	19-9
		Hem-fir	#2	6-9	10-8	14-0	17-2	19-11	6-9	10-3	13-0	15-10	18-5
	12	Hem-fir	#3	5-10	8-6	10-9	13-2	15-3	5-5	7-10	10-0	12-2	14-1
	12	Southern pine	SS	7-6	11-10	15-7	19-11	24-3	7-6	11-10	15-7	19-11	24-3
		Southern pine	#1	7-3	11-5	15-0	18-2	21-7	7-3	11-4	14-5	16-10	20-0
		Southern pine	#2	6-11	10-6	13-4	15-10	18-8	6-6	9-9	12-4	14-8	17-3
		Southern pine	#3	5-5	8-0	10-1	12-3	14-6	5-0	7-5	9-4	11-4	13-5
		Spruce-pine-fir	SS	7-1	11-2	14-8	18-9	22-10	7-1	11-2	14-8	18-9	22-4
		Spruce-pine-fir	#1	6-11	10-11	14-3	17-5	20-2	6-11	10-5	13-2	16-1	18-8
		Spruce-pine-fir	#2	6-11	10-11	14-3	17-5	20-2	6-11	10-5	13-2	16-1	18-8
		Spruce-pine-fir	#3	5-10	8-6	10-9	13-2	15-3	5-5	7-10	10-0	12-2	14-1
		Douglas fir-larch	SS	7-0	11-0	14-5	18-5	22-5	7-0	11-0	14-5	18-3	21-2
		Douglas fir-larch	#1	6-9	10-5	13-2	16-1	18-8	6-7	9-8	12-2	14-11	17-3
		Douglas fir-larch	#2	6-7	9-10	12-6	15-3	17-9	6-3	9-2	11-7	14-2	16-5
		Douglas fir-larch	#3	5-2	7-7	9-7	11-8	13-6	4-9	7-0	8-10	10-10	12-6
		Hem-fir	SS	6-7	10-4	13-8	17-5	21-2	6-7	10-4	13-8	17-5	20-5
		Hem-fir	#1	6-5	10-2	13-0	15-11	18-5	6-5	9-6	12-1	14-9	17-1
		Hem-fir	#2	6-2	9-7	12-2	14-10	17-3	6-1	8-11	11-3	13-9	15-11
		Hem-fir	#3	5-0	7-4	9-4	11-5	13-2	4-8	6-10	8-8	10-6	12-3
	16	Southern pine	SS	6-10	10-9	14-2	18-1	22-0	6-10	10-9	14-2	18-1	21-10
		Southern pine	#1	6-7	10-4	13-5	15-9	18-8	6-7	9-10	12-5	14-7	17-3
		Southern pine	#2	6-1	9-2	11-7	13-9	16-2	5-8	8-5	10-9	12-9	15-0
		Southern pine	#3	4-8	6-11	8-9	10-7	12-6	4-4	6-5	8-1	9-10	11-7
		Spruce-pine-fir	SS	6-5	10-2	13-4	17-0	20-9	6-5	10-2	13-4	16-8	19-4
		Spruce-pine-fir	#1	6-4	9.9	12-4	15-1	17-6	6-2	9-0	11-5	13-11	16-2
		Spruce-pine-fir	#1	6-4	9-9	12-4	15-1	17-6	6-2	9-0	11-5	13-11	16-2
		Spruce-pine-fir	#3	5-0	7-4	9-4	11-5	13-2	4-8	6-10	8-8	10-6	12-3
		Douglas fir-larch	SS	6-7	10-4	13-7	17-4	20-11	6-7	10-4	13-7	16-8	12-5
		Douglas III-Ialeli	33	6-1	0.4	12.0	1/-4	17-1	6.0	2.10	11-7	10-0	15-0
		Soughas III-Iaich	00										
Rafter Spa	n	lah		C									
Tarter <u>Jp</u>			<b>IC</b>	5									



1/4" = 1'-0"



**R802.4.3** Hips and valleys. Hip and valley rafters shall be not less than 2 inches (51 mm) nominal in thickness and not less in depth than the cut end of the rafter. Hip and valley rafters shall be supported at the ridge by a brace to a bearing partition or be designed to carry and distribute the specific load at that point.

**R802.4.4 <u>Rafter supports</u>**. Where the roof pitch is less than 3:12 (25-percent slope), structural members that support rafters, such as ridges, hips and valleys, shall be designed as beams, and bearing shall be provided for rafters in accordance with Section R802.6.

**R802.4.5 Purlins.** Installation of purlins to reduce the span of rafters is permitted as shown in Figure R802.4.5. Purlins shall be sized not less than the required size of the rafters that they support. Purlins shall be continuous and shall be supported by 2-inch by 4-inch (51 mm by 102 mm) braces installed to bearing walls at a slope not less than 45 degrees (0.79 rad) from the horizontal. The braces shall be spaced not more than 4 feet (1219 mm) on center and the unbraced length of braces shall not exceed 8 feet (2438 mm).

**R802.4.6** Collar ties. Where collar ties are used to connect opposing rafters, they shall be located in the upper third of the *attic* space and fastened in accordance with Table R602.3(1). Collar ties shall be not less than 1 inch by 4 inches ( $25 \text{ mm} \times 102 \text{ mm}$ ) nominal, spaced not more

than 4 feet (1220 mm) on center. Ridge straps in accordance with Table R602.3(1) shall be permitted to replace collar ties.

**R802.5** Ceiling joists. Ceiling joists shall be continuous across the sururure or securely joined where they meet over interior partitions in accordance with Table R802.5.2.

**R802.5.1** Ceiling joist size. Ceiling joists shall be sized based on the joist spans in Tables R802.5.1(1) and R802.5.1(2). For other grades and species and for other loading conditions, refer to the AWC STJR.

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**R802.5.2** Ceiling joist and rafter connections. Where ceiling joists run parallel to rafters, they shall be connected to rafters at the top wall plate in accordance with Table R802.5.2. Where ceiling joists are not connected to the rafters at the top wall plate, they shall be installed in the bottom third of the rafter height in accordance with Figure R802.4.5 and Table R802.5.2. Where the ceiling joists are installed above the bottom third of the rafter height, the ridge shall be designed as a beam. Where ceiling joists do not run parallel to rafters, the ceiling joists shall be connected to top plates in accordance with Table R602.3(1). Each rafter shall be tied across the structure with a rafter tie or a 2-inch by 4-inch (51 mm  $\times$  102 mm) kicker connected to the ceiling diaphragm with nails equivalent in capacity to Table R802.5.2.



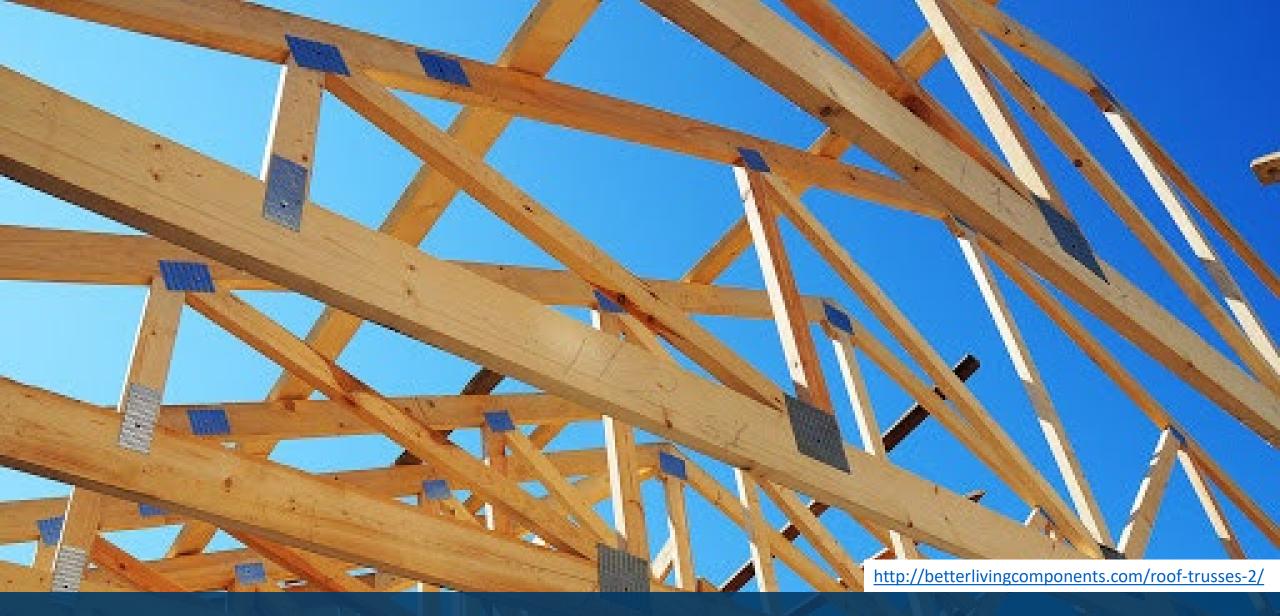
 $H_c$  = Height of ceiling joists or rafter ties measured vertically above the top of rafter support walls

Hand Framed Roof/Ceiling

FIGURE R802.4.5 BRACED RAFTER CONSTRUCTION

		TABLE R802.5.1(1)         CEILING JOIST SPANS FOR COMMON LUMBER SPECIES         (Uninhabitable attics without storage, live load = 10 psf, L/∆ = 240)								
		SPECIES AND GRADE		DEAD LOAD = 5 psf						
	CEILING JOIST			2 × 4	2×6	2 × 8	2 × 10			
	SPACING (inches)				Maximum ceiling joist spans					
				(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)			
		Douglas fir-larch	SS	13-2	20-8	Note a	Note a			
		Douglas fir-larch	#1	12-8	19-11	Note a	Note a			
		Douglas fir-larch	#2	12-5	19-6	25-8	Note a			
		Douglas fir-larch	#3	11-1	16-3	20-7	25-2			
		Hem-fir	SS	12-5	19-6	25-8	Note a			
		Hem-fir	#1	12-2	19-1	25-2	Note a			
		Hem-fir	#2	11-7	18-2	24-0	Note a			
	12	Hem-fir	#3	10-10	15-10	20-1	24-6			
TABLE R802.5.1(1)		Southern pine	SS	12-11	20-3	Note a	Note a			
CEILING JOIST SPANS FOR COMMON LUMBER SPECIES		Southern pine	#1	12-5	19-6	25-8	Note a			
(Uninhabitable attics without storage, live load = 10 psf, $L/\Delta$ = 24	ທາ	Southern pine	#2	11-10	18-8	24-7	Note a			
(ommabiliable attacs without storage, invertous interpat, EB 24	~/	Southern pine	#3	10-1	14-11	18-9	22-9			
		Spruce-pine-fir	SS	12-2	19-1	25-2	Note a			
		Spruce-pine-fir	#1	11-10	18-8	24-7	Note a			
		Spruce-pine-fir	#2	11-10	18-8	24-7	Note a			
TABLE R802.5.1(2)		Spruce-pine-fir	#3	10-10	15-10	20-1	24-6			
		Douglas fir-larch	SS	11-11	18-9	24-8	Note a			
CEILING JOIST SPANS FOR COMMON LUMBER SPECIES		Douglas fir-larch	#1	11-6	18-1	23-10	Note a			
(Uninhabitable attics with limited storage, live load = 20 psf, $L/\Delta$	= 240)	Douglas fir-larch	#2	11-3	17-8	23-4	Note a			
(	,	Douglas fir-larch	#3	9-7	14-1	17-10	21-9			
		Hem-fir	SS	11-3	17-8	23-4	Note a			
		Hem-fir	#1	11-0	17-4	22-10	Note a			
		Hem-fir	#2	10-6	16-6	21-9	Note a			
	16	Hem-fir	#3	9-5	13-9	17-5	21-3			
		Southern pine	SS	11-9	18-5	24-3	Note a			
		Southern pine	#1	11-3	17-8	23-10	Note a			
		Southern pine	#2	10-9	16-11	21-7	25-7			
		Southern pine	#3	8-9	12-11	16-3	19-9			
		Spruce-pine-fir	SS	11-0	17-4	22-10	Note a			
		Spruce-pine-fir	#1	10-9	16-11	22-4	Note a			
		Spruce-pine-fir	#2	10-9	16-11	22-4	Note a			
	L	Spruce-pine-fir	#3	9-5	13-9	17-5	21-3			

# Ceiling Joist Span Tables



# Roof Truss – Chapter 8

#### R802.10 Wood trusses.

**R802.10.1** <u>Truss design drawings.</u> Truss design drawings, prepared in conformance to Section R802.10.1, shall be provided to the *building official* and *approved* prior to installation. Truss design drawings shall be provided with the shipment of trusses delivered to the job site. Truss design drawings shall include, at a minimum, the following information:

- 1. Slope or depth, span and spacing.
- 2. Location of all joints.
- 3. Required bearing widths.
- 4. Design loads as applicable.
  - 4.1. Top chord live load (as determined from Section R301.6).
  - 4.2. Top chord dead load.
  - 4.3. Bottom chord live load.
  - 4.4. Bottom chord dead load.
  - 4.5. Concentrated loads and their points of application.
  - 4.6. Controlling wind and earthquake loads.
- 5. Adjustments to lumber and joint connector design values for conditions of use.
- 6. Each reaction force and direction.
- 7. Joint connector type and description such as size, thickness or gage and the dimensioned location of each joint connector except where symmetrically located relative to the joint interface.
- 8. Lumber size, species and grade for each member.
- 9. Connection requirements for:
  - 9.1. Truss to girder-truss.
  - 9.2. Truss ply to ply.
  - 9.3. Field splices.

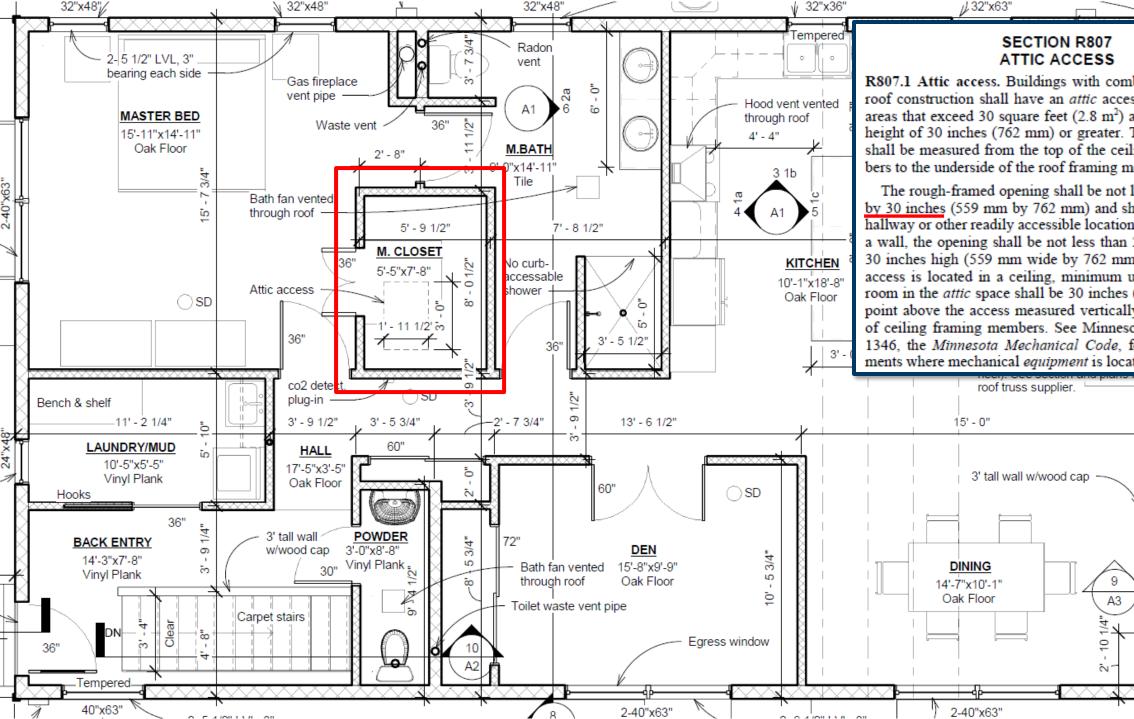
- Calculated deflection ratio or maximum description for live and total load.
- 11. Maximum axial compression forces in the truss members to enable the building designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces shall be shown on the truss design drawing or on supplemental documents.
- 12. Required permanent truss member bracing location.

**R802.10.2 Design.** Wood trusses shall be designed in accordance with accepted engineering practice. The design and manufacture of metal-plate-connected wood trusses shall comply with ANSI/TPI 1. The truss design drawings shall be prepared by a registered professional where required by the statutes of the *jurisdiction* in which the project is to be constructed in accordance with Section R106.1.

**R802.10.2.1** Applicability limits. The provisions of this section shall control the design of truss roof framing where snow controls for buildings that are not greater than 60 feet (18 288 mm) in length perpendicular to the joist, rafter or truss span, not greater than 36 feet (10 973 mm) in width parallel to the joist, rafter or truss span, not more than three stories above grade plane in height, and have roof slopes not smaller than 3:12 (25-percent slope) or greater than 12:12 (100-percent slope). Truss roof framing constructed in accordance with the provisions of this section shall be limited to sites subjected to a maximum design wind speed of 140 miles per hour (63 m/s), Exposure B or C, and a maximum ground snow load of 70 psf (3352 Pa). For consistent loading of all truss types, roof snow load is to be computed as:  $0.7 p_e$ .

**R802.10.3 Bracing.** Trusses shall be braced to prevent rotation and provide lateral stability in accordance with the requirements specified in the *construction documents* for the building and on the individual truss design drawings. In the absence of specific bracing requirements, trusses shall be braced in accordance with accepted industry practice such as the SBCA *Building Component Safety Information (BCSI) Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.* 

**R802.10.4** Alterations to trusses. Truss members shall not be cut, notched, drilled, spliced or otherwise altered in any way without the approval of a registered *design professional*. Alterations resulting in the addition of load such as HVAC equipment water heater that exceeds the design load for the truss shall not be permitted without verification that the truss is capable of supporting such additional loading.



R807.1 Attic access. Buildings with combustible ceiling or roof construction shall have an attic access opening to attic areas that exceed 30 square feet (2.8 m<sup>2</sup>) and have a vertical height of 30 inches (762 mm) or greater. The vertical height shall be measured from the top of the ceiling framing members to the underside of the roof framing members.

32"x63"

4' - 8 1/2"

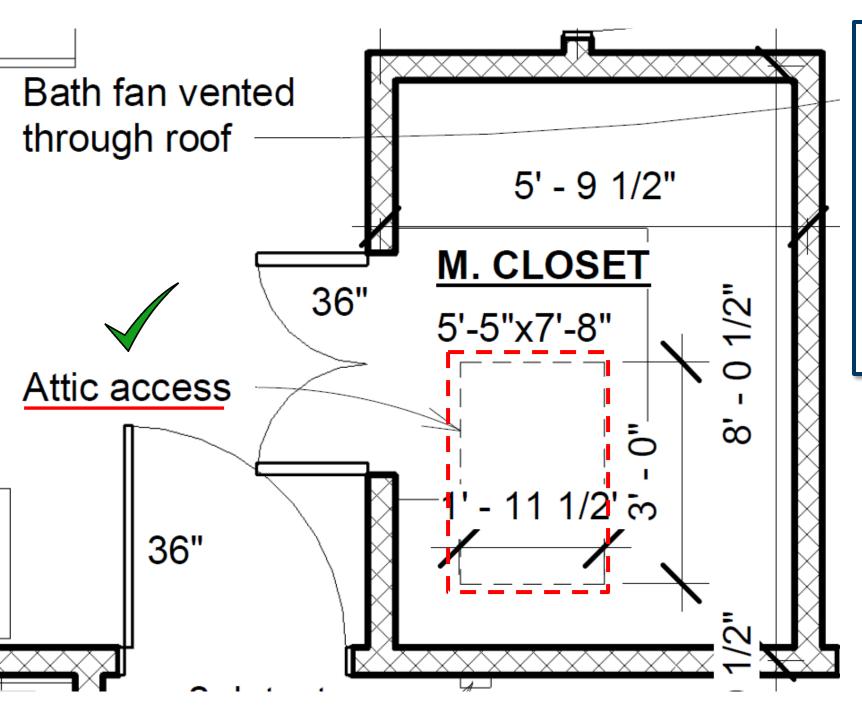
36"

FOYER 5'-0"x10'-1"

36"

Tile

The rough-framed opening shall be not less than 22 inches by 30 inches (559 mm by 762 mm) and shall be located in a hallway or other readily accessible location. Where located in a wall, the opening shall be not less than 22 inches wide by 30 inches high (559 mm wide by 762 mm high). Where the access is located in a ceiling, minimum unobstructed headroom in the attic space shall be 30 inches (762 mm) at some point above the access measured vertically from the bottom of ceiling framing members. See Minnesota Rules, Chapter 1346, the Minnesota Mechanical Code, for access requirements where mechanical equipment is located in attics.



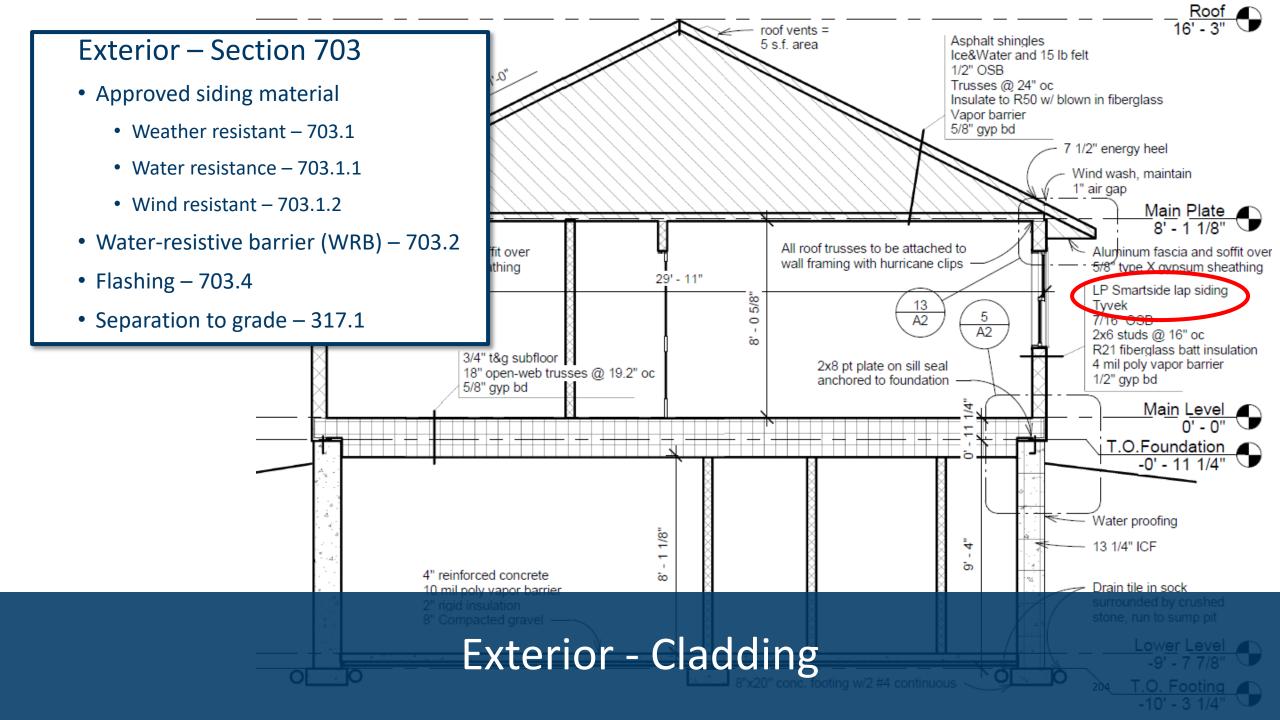
#### SECTION R807 ATTIC ACCESS

**R807.1** Attic access. Buildings with combustible ceiling or roof construction shall have an *attic* access opening to *attic* areas that exceed 30 square feet  $(2.8 \text{ m}^2)$  and have a vertical height of 30 inches (762 mm) or greater. The vertical height shall be measured from the top of the ceiling framing members to the underside of the roof framing members.

The rough-framed opening shall be not less than 22 inches by 30 inches (559 mm by 762 mm) and shall be located in a hallway or other readily accessible location. Where located in a wall, the opening shall be not less than 22 inches wide by 30 inches high (559 mm wide by 762 mm high). Where the access is located in a ceiling, minimum unobstructed headroom in the *attic* space shall be 30 inches (762 mm) at some point above the access measured vertically from the bottom of ceiling framing members. See Minnesota Rules, Chapter 1346, the *Minnesota Mechanical Code*, for access requirements where mechanical *equipment* is located in *attics*.

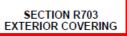


# Exterior – Chapters 7 & 9



clear airspaces. Other openings with the equivalent vent area shall be permitted.

- 1. Vinyl polypropylene or horizontal aluminum siding applied over a weather-resistive barrier as specified in Table R703.3(1).
- 2. Brick veneer with a clear airspace as specified in Table R703.8.4.
- 3. Other approved vented claddings.



R703.1 General. Exterior walls shall provide the building with a weather-resistant exterior wall envelope. The exterior wall envelope shall include flashing as described in Section R703.4.

Exception: Log walls designed and constructed in accordance with the provisions of ICC 400.

R703.1.1 Water resistance. The exterior wall envelope shall be designed and constructed in a manner that prevents the accumulation of water within the wall assembly by pro- R703.2 Water-resistive barrier. One layer of No. 15 asphalt viding a water-resistant barrier behind the exterior cladding as required by Section R703.2 and a means of draining to the exterior water that penetrates the exterior cladding.

#### Exceptions:

- 1. A weather-resistant exterior wall envelope shall not be required over concrete or masonry walls designed in accordance with Chapter 6 and flashed in accordance with Section R703.4 or R703.8.
- 2. Compliance with the requirements for a means of drainage, and the requirements of Sections R703.2 and R703.4, shall not be required for an exterior wall envelope that has been demonstrated to resist wind-driven rain through testing of the exterior wall envelope, including joints, penetrations and intersections with dissimilar materials, in accordance with ASTM E331 under the following con- R703.3 Wall covering nominal thickness and attachments. ditions:
  - 2.1. Exterior wall envelope test assemblies shall include at least one opening, one control joint, one wall/eave interface and one wall sill. All tested openings and penetrations shall be representative of the intended end-use configuration.
  - 2.2. Exterior wall envelope test assemblies shall be at least 4 feet by 8 feet (1219 mm by 2438 mm) in size.
  - 2.3. Exterior wall assemblies shall be tested at a minimum differential pressure of 6.24 pounds per square foot (299 Pa).
  - 2.4. Exterior wall envelope assemblies shall be subjected to the minimum test exposure for a minimum of 2 hours.

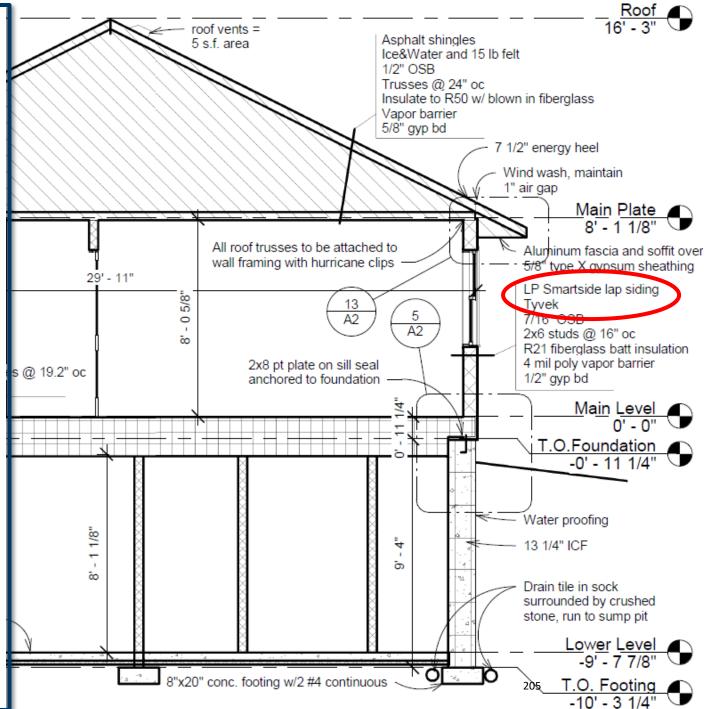
The exterior wall envelope design shall be considered to resist wind-driven rain where the

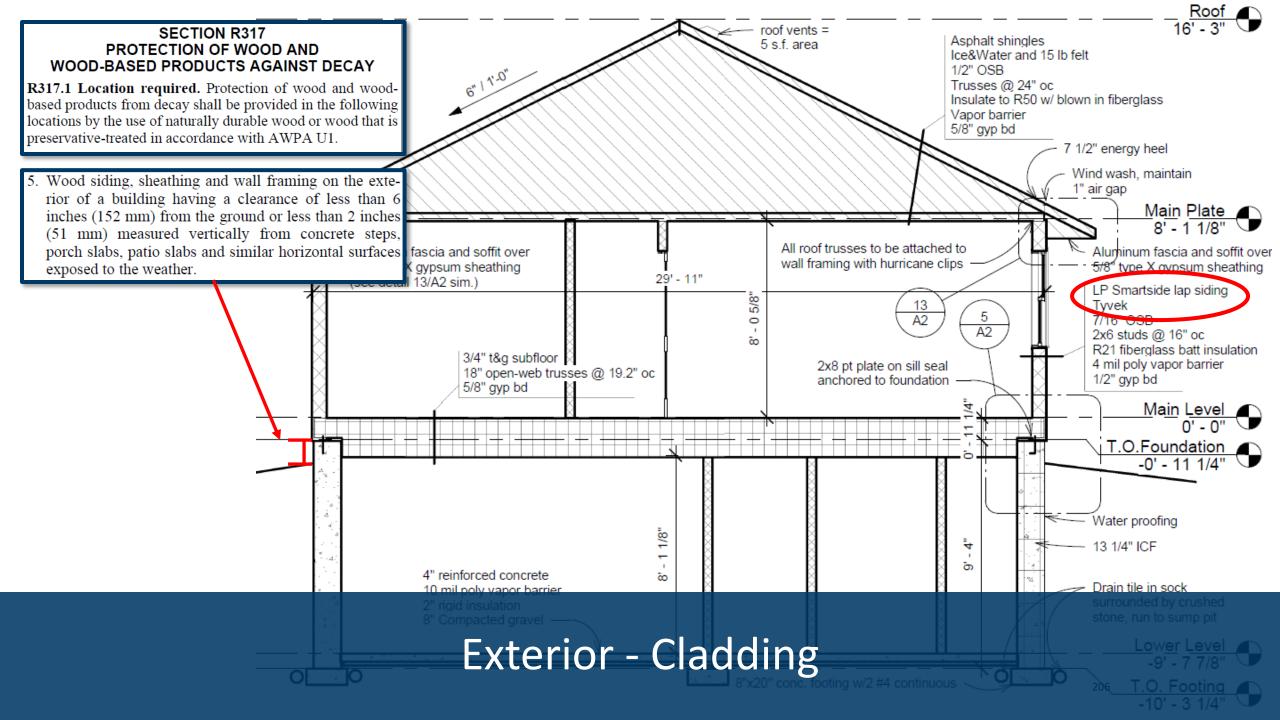
results of testing indicate that water did not penetrate control joints in the exterior wall envelope, joints at the perimeter of openings penetration or intersections of terminations with dissimilar materials.

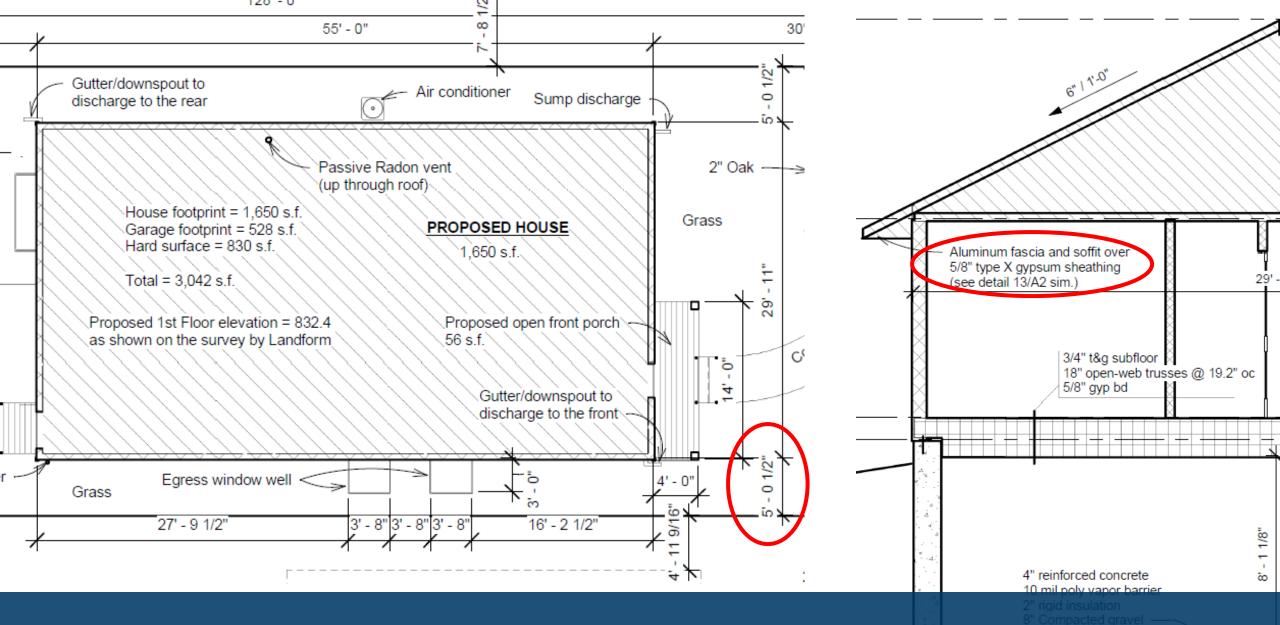
R703.1.2 Wind resistance. Wall coverings, backing materials and their attachments shall be capable of resisting wind loads in accordance with Tables R301.2(2) and R301.2(3). Wind-pressure resistance of the siding, soffit and backing materials shall be determined by ASTM E330 or other applicable standard test methods. Where windpressure resistance is determined by design analysis, data from approved design standards and analysis conforming to generally accepted engineering practice shall be used to evaluate the siding, soffit and backing material and its fastening. All applicable failure modes including bending rupture of siding, fastener withdrawal and fastener head pull-through shall be considered in the testing or design analysis. Where the wall covering, soffit and backing material resist wind load as an assembly, use of the design capacity of the assembly shall be permitted.

felt, free from holes and breaks, complying with ASTM D226 for Type 1 felt or other approved water-resistive barrier shall be applied over studs or sheathing of all exterior walls. No.15 asphalt felt shall be applied horizontally, with the upper layer lapped over the lower layer not less than 2 inches (51 mm). Where joints occur, felt shall be lapped not less than 6 inches (152 mm). Other approved materials shall be installed in accordance with the water-resistive barrier manufacturer's installation instructions. The No. 15 asphalt felt or other approved water-resistive barrier material shall overlap the flashings required in Section R703.4 not less than 2 inches (51 mm). The No. 15 asphalt felt or other approved waterresistive barrier material shall be continuous up to the underside of the rafter or truss top chord and terminated at penetrations and building appendages in a manner to meet the requirements of the exterior wall envelope as described in Section R703.1.

The nominal thickness and attachment of exterior wall coverings shall be in accordance with Table R703.3(1), the wall covering material requirements of this section, and the wall covering manufacturer's installation instructions. Cladding attachment over foam sheathing shall comply with the additional requirements and limitations of Sections R703.15 through R703.17. Nominal material thicknesses in Table R703.3(1) are based on a maximum stud spacing of 16 inches (406 mm) on center. Where specified by the siding manufacturer's instructions and supported by a test report or other documentation, attachment to studs with greater spacing is permitted. Fasteners for exterior wall coverings attached to wood framing shall be in accordance with Section R703.3.3 and Table R703.3(1). Exterior wall coverings shall be attached to cold-formed steel light frame construction in accordance with the cladding manufacturer's installation instructions, the requirements of Table R703.3(1) using screw fasteners substituted for the nails specified in accordance with Table R703.3(2), or an approved design.







Exterior – Fire Separation Distance – Chapter 3

		balbubau iba	
EXTERI	OR WALL ELEMENT	MINIMUM FIRE-RESISTANCE RATING	MINIMUM FIRE SEPARATION DISTANCE
Walls	Fire-resistance rated	1 hour—tested in accordance with ASTM E119, UL 263, or Section 703.3 of the <i>International</i> <i>Building Code</i> with exposure from both sides	0 feet
	Not fire-resistance rated	0 hours	$\geq 5$ feet
Projections	Not allowed	NA	< 2 feet
	Fire-resistance rated	1 hour on the underside, or heavy timber, or fire- retardant-treated wood <sup>a, b, c</sup>	$\ge 2$ feet to $< 5$ feet
	Not fire-resistance rated	0 hours	$\geq 5$ feet
	Not allowed	NA	< 3 feet
Openings in walls	25% maximum of wall area	0 hours	3 feet
	Unlimited	0 hours	5 feet
Depatrations	A 11	Comply with Section R302.4	< 3 feet
Penetrations	All	None required	3 feet

NA = Not Applicable.

a. The fire-resistance rating shall be permitted to be reduced to 0 hours on the underside of the eave overhang if fireblocking is provided from the wall top plate to the underside of the roof sheathing.

b. The fire-resistance rating shall be permitted to be reduced to 0 hours on the underside of the rake overhang where gable vent openings are not installed. c. One hour on the underside equates to one layer of  $\frac{5}{8}$ -inch type X gypsum sheathing. Openings are not allowed.

### 4" reinforced concrete 10 mil poly vapor barrier 2" rigid insulation

8" Compacted gravel —

3/4" t&g subfloor 🖗

5/8" gyp bd

18" open-web trusses @ 19.2" oc

6" | 1'.D"

29'

- 1 1/8"

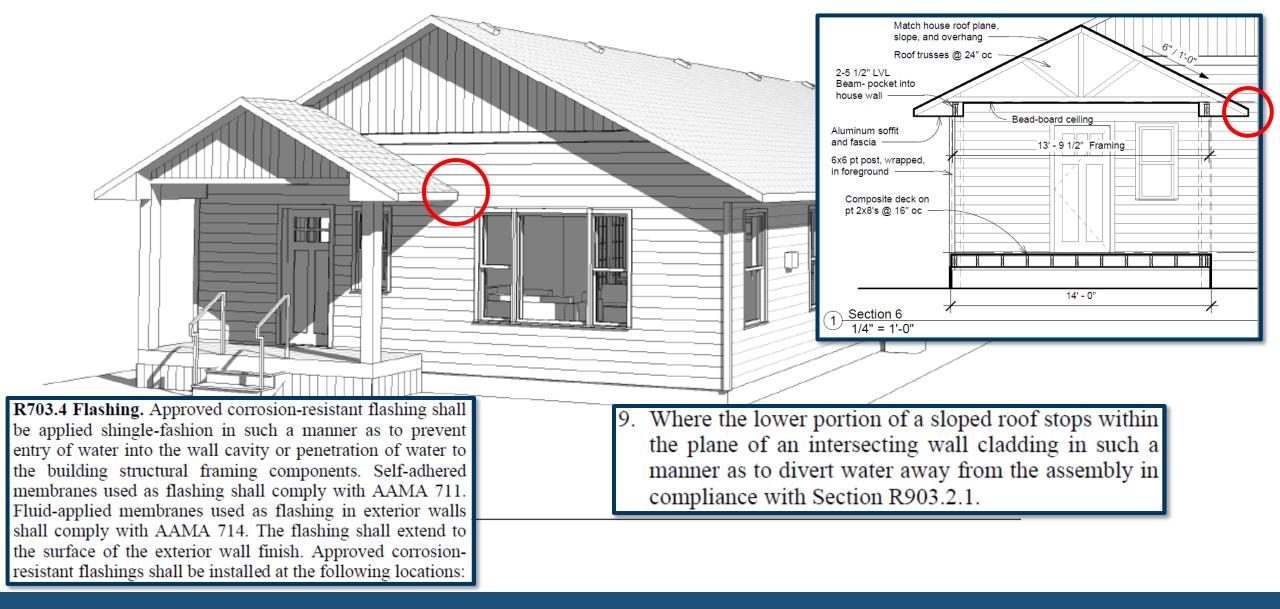
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Aluminum fascia and soffit over 5/8" type X gypsum sheathing

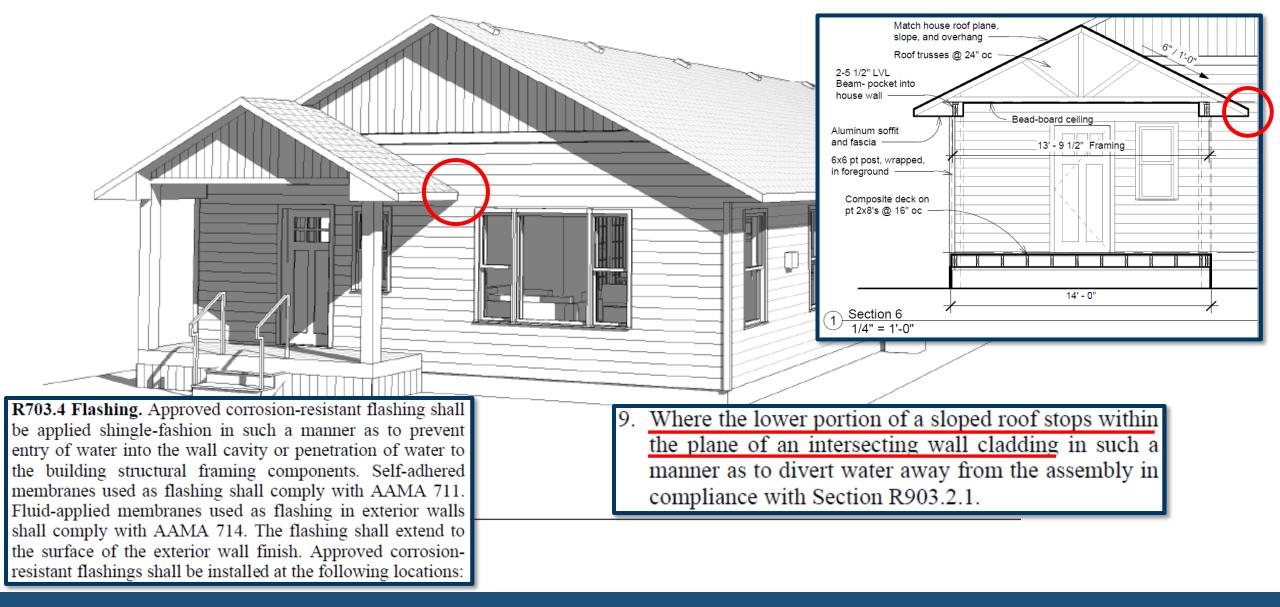
(see detail 13/A2 sim.)

## **Exterior – Fire Separation Distance**

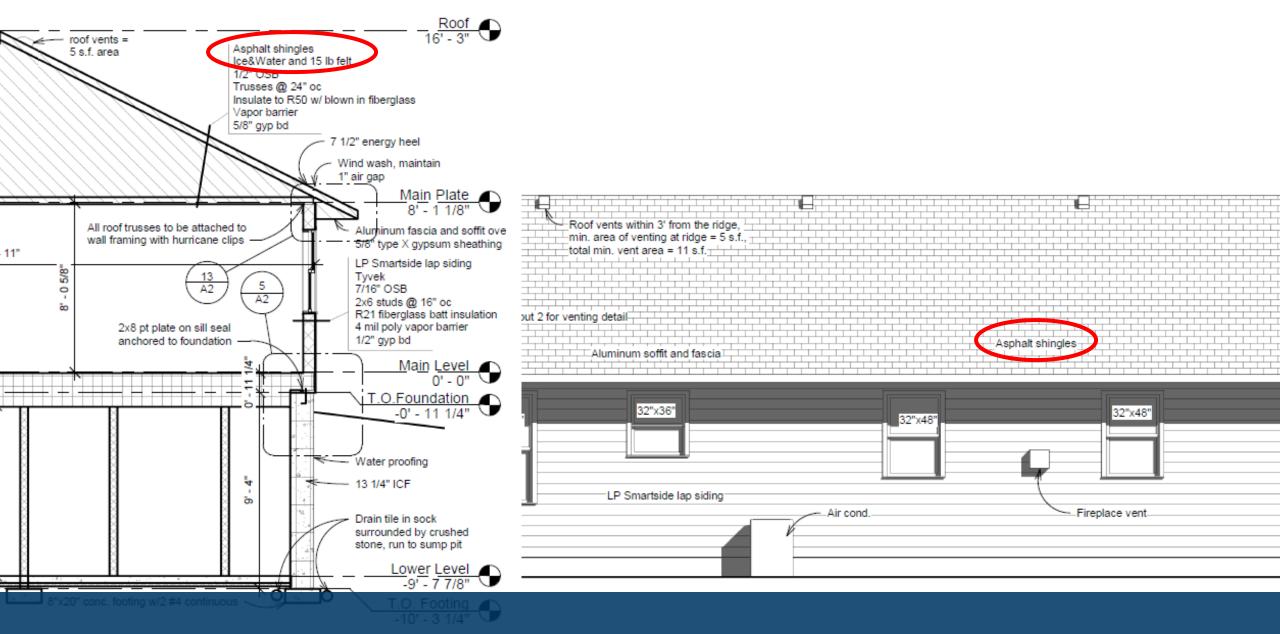
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# Exterior – Flashing



# Exterior – Flashing



## Exterior – Roof Covering – Chapter 9

**R905.2 Asphalt shingles.** The installation of asphalt shingles shall comply with the provisions of this section.

**R905.2.1 Sheathing requirements.** Asphalt shingles shall be fastened to solidly sheathed decks or 1-inch thick nominal wood boards.

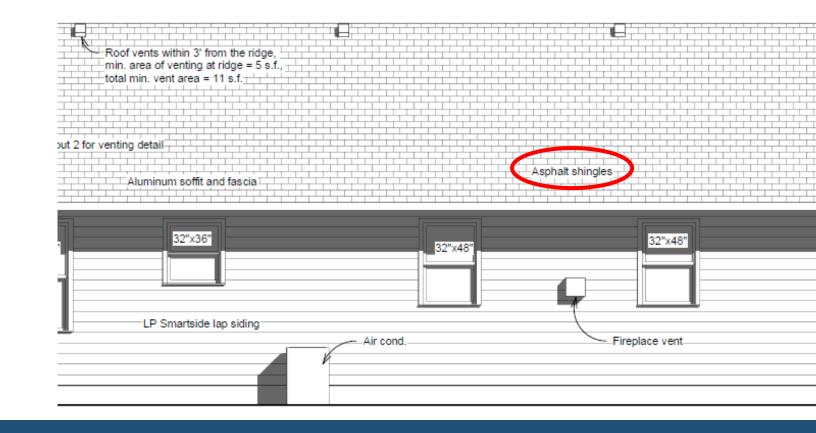
**R905.2.2 Slope.** Asphalt shingles shall be used only on roof slopes of two units vertical in 12 units horizontal (17-percent slope) or greater. For roof slopes from two units vertical in 12 units horizontal (17-percent slope) up to four units vertical in 12 units horizontal (33-percent slope), double *underlayment* application is required in accordance with Section R905.1.1.

**R905.2.3 Underlayment.** *Underlayment* shall comply with Section R905.1.1.

R905.2.4 Asphalt shingles. Asphalt shingles shall comply with ASTM D3462.

**R905.2.4.1 Wind resistance of asphalt shingles.** Asphalt shingles shall be tested in accordance with ASTM D7158. Asphalt shingles shall meet the classification requirements of Table R905.2.4.1 for the appropriate ultimate design wind speed. Asphalt shingle packaging shall bear a label to indicate compliance with ASTM D7158 and the required classification in Table R905.2.4.1.

**Exception:** Asphalt shingles not included in the scope of ASTM D7158 shall be tested and labeled in accordance with ASTM D3161. Asphalt shingle packaging shall bear a label to indicate compliance with ASTM D3161 and the required classification in Table R905.2.4.1.



**R905.2 Asphalt shingles.** The installation of asphalt shingles shall comply with the provisions of this section.

**R905.2.1 Sheathing requirements.** Asphalt shingles shall be fastened to solidly sheathed decks or 1-inch thick nominal wood boards.

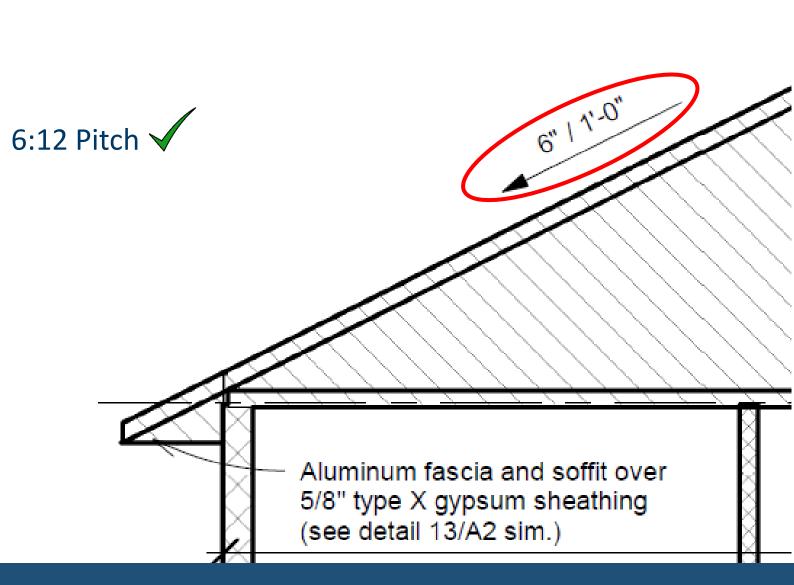
**R905.2.2 Slope.** Asphalt shingles shall be used only on roof slopes of two units vertical in 12 units horizontal (17-percent slope) or greater. For roof slopes from two units vertical in 12 units horizontal (17-percent slope) up to four units vertical in 12 units horizontal (33-percent slope), double *underlayment* application is required in accordance with Section R905.1.1.

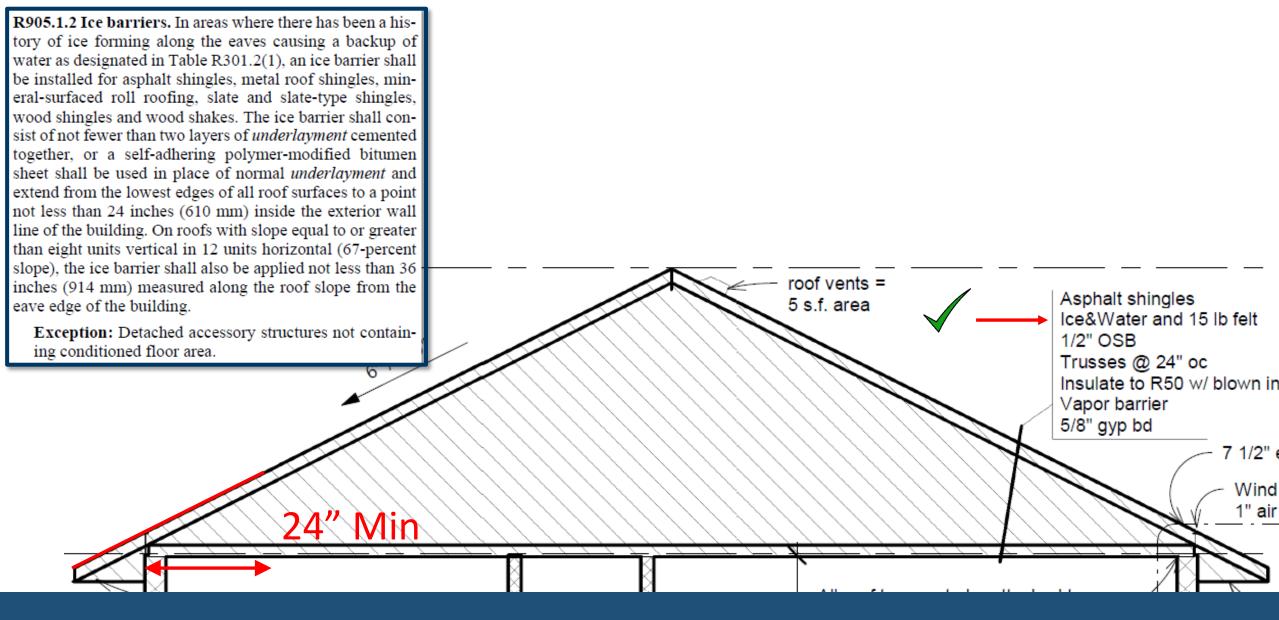
**R905.2.3 Underlayment.** Underlayment shall comply with Section R905.1.1.

R905.2.4 Asphalt shingles. Asphalt shingles shall comply with ASTM D3462.

**R905.2.4.1 Wind resistance of asphalt shingles.** Asphalt shingles shall be tested in accordance with ASTM D7158. Asphalt shingles shall meet the classification requirements of Table R905.2.4.1 for the appropriate ultimate design wind speed. Asphalt shingle packaging shall bear a label to indicate compliance with ASTM D7158 and the required classification in Table R905.2.4.1.

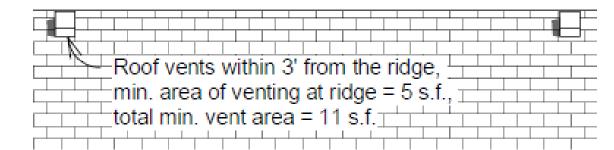
**Exception:** Asphalt shingles not included in the scope of ASTM D7158 shall be tested and labeled in accordance with ASTM D3161. Asphalt shingle packaging shall bear a label to indicate compliance with ASTM D3161 and the required classification in Table R905.2.4.1.





### SECTION R806 ROOF VENTILATION

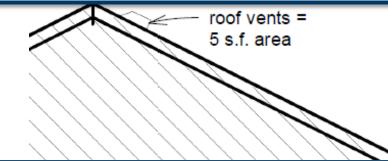
**R806.1 Ventilation required.** Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of  $\frac{1}{16}$  inch (1.6 mm) minimum and  $\frac{1}{4}$  inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than  $\frac{1}{4}$  inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, perforated vinyl or similar material with openings having a least dimension of  $\frac{1}{16}$  inch (1.6 mm) minimum and  $\frac{1}{4}$  inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7. Required ventilation openings shall open directly to the outside air and shall be protected to prevent the entry of birds, rodents, snakes and other similar creatures.

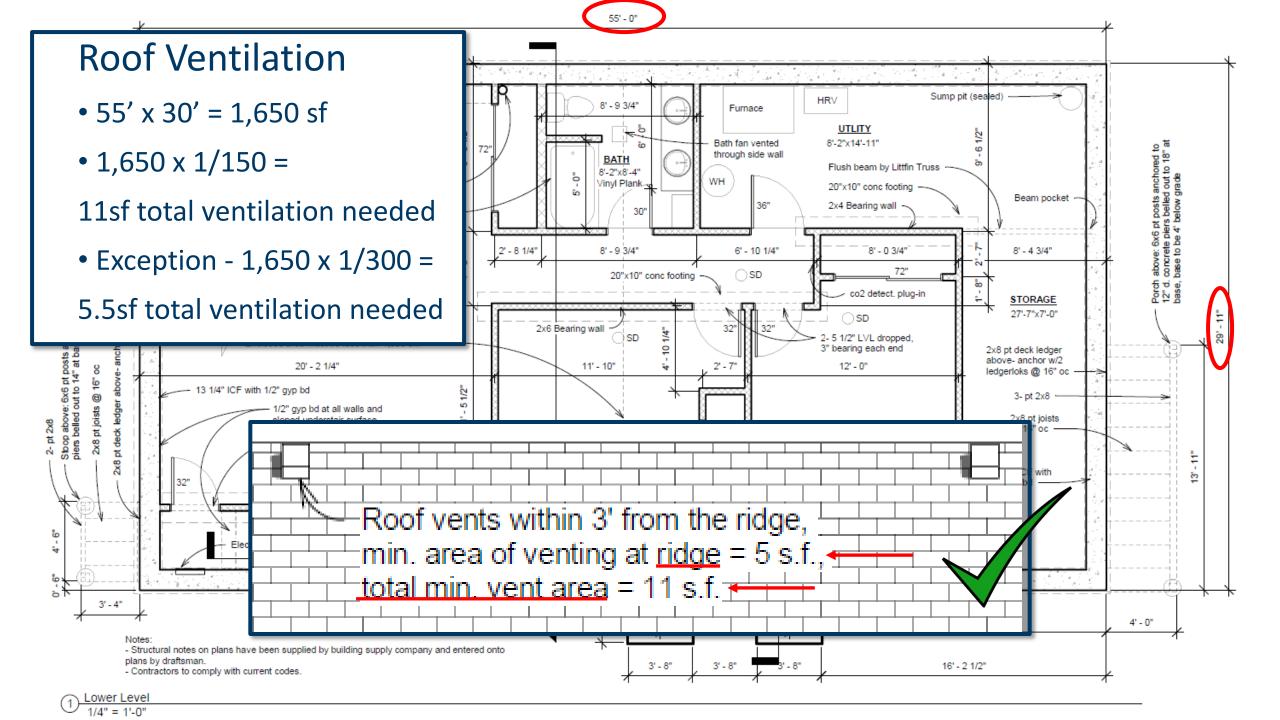


**R806.2 Minimum vent area.** The minimum net free ventilating area shall be  $\frac{1}{150}$  of the area of the vented space.

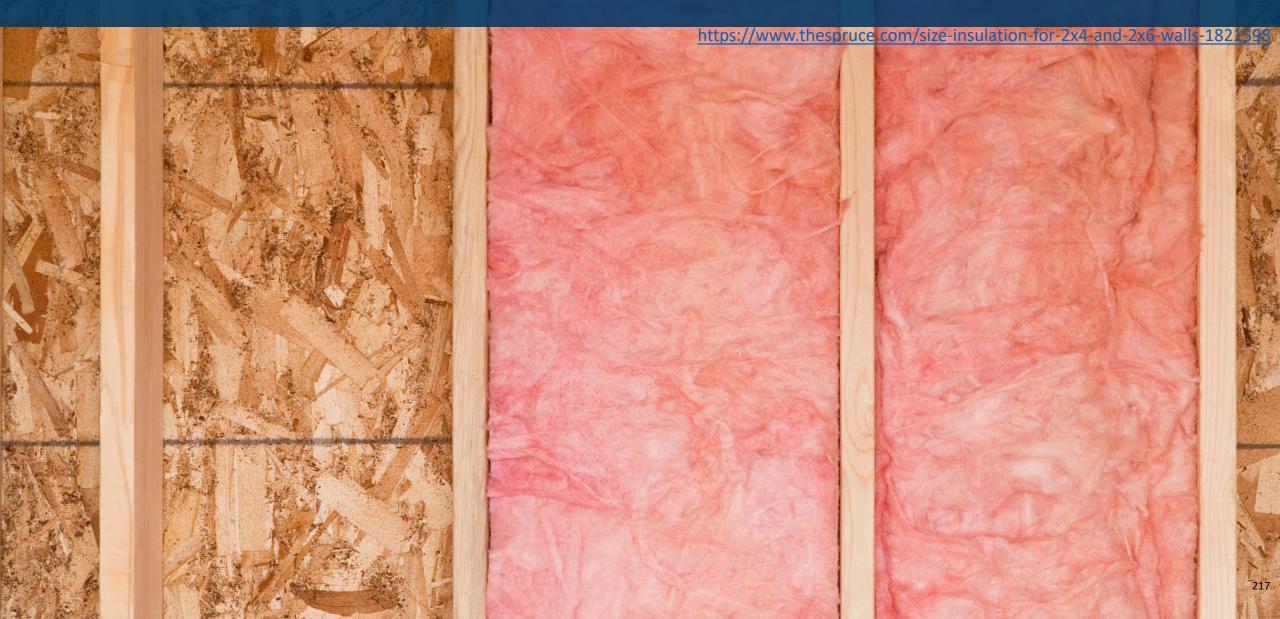
**Exception:** The minimum net free ventilation area shall be  $1/_{300}$  of the vented space provided both of the following conditions are met:

- In Climate Zones 6, 7 and 8, a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.
- 2. Not less than 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space. Upper ventilators shall be located not more than 3 feet (914 mm) below the ridge or highest point of the space, measured vertically. The balance of the required ventilation provided shall be located in the bottom one-third of the *attic* space. Where the location of wall or roof framing members conflicts with the installation of upper ventilators, installation more than 3 feet (914 mm) below the ridge or highest point of the space shall be permitted.





# Insulation – MNRE Chapter 4



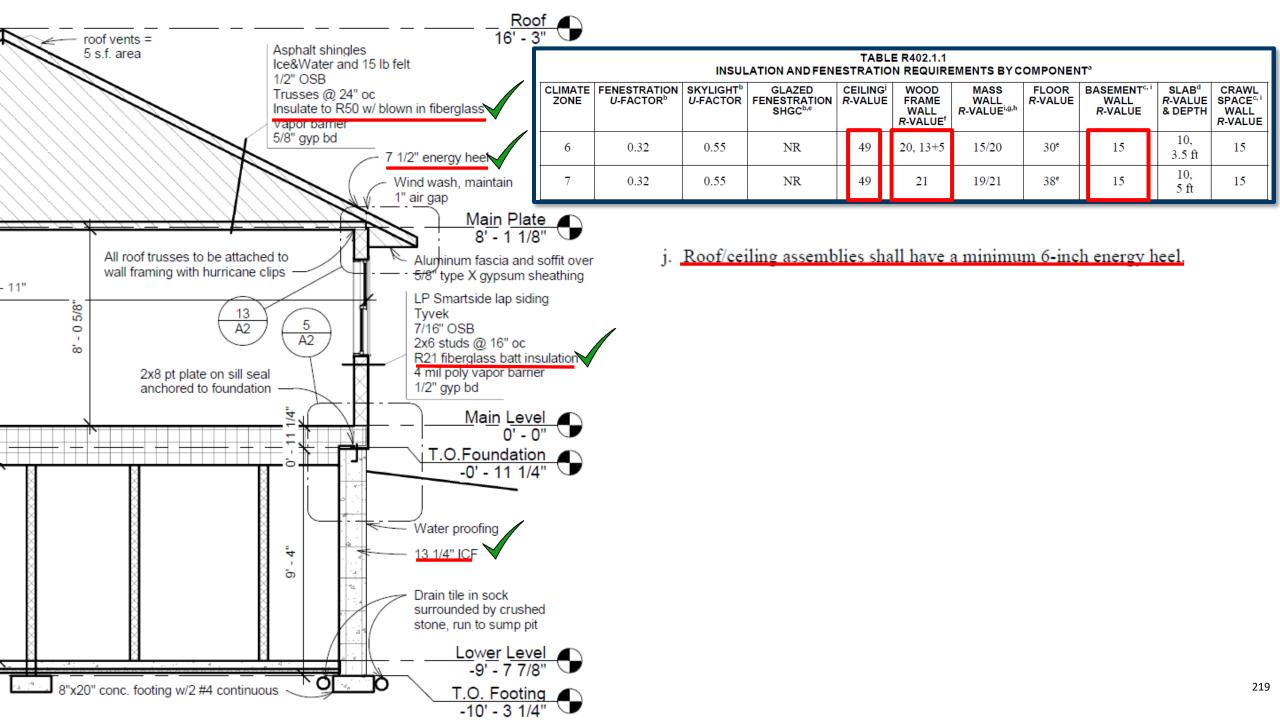
# Insulation – RE402

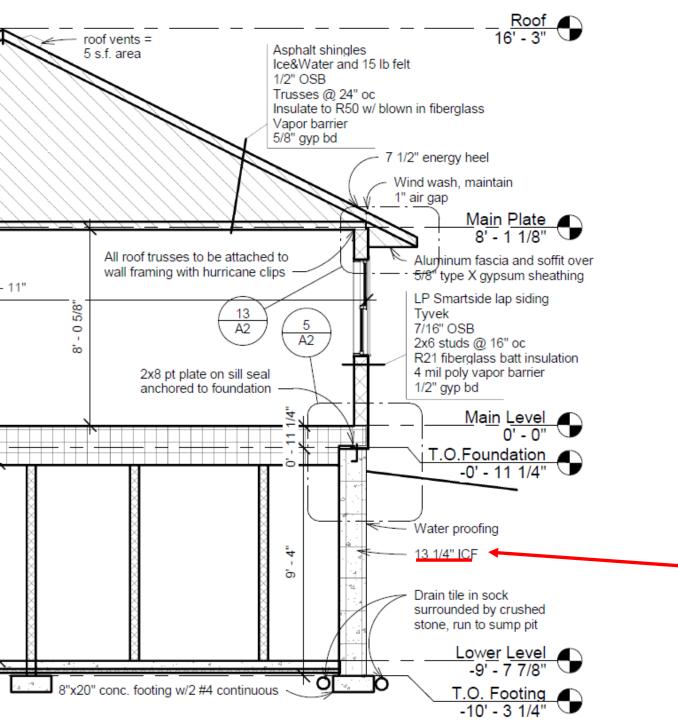
### TABLE R402.1.1 INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT<sup>a</sup>

CLIMATE ZONE	FENESTRATION U-FACTOR <sup>b</sup>	SKYLIGHT <sup>♭</sup> <i>U</i> -FACTOR	GLAZED FENESTRATION SHGC <sup>b,e</sup>	CEILING <sup>i</sup> R-VALUE	WOOD FRAME WALL <i>R</i> -VALUE <sup>f</sup>	MASS WALL <i>R</i> -VALUE <sup>i,g,h</sup>	FLOOR <i>R</i> -VALUE	BASEMENT <sup>c, i</sup> WALL <i>R</i> -VALUE	SLAB <sup>d</sup> <i>R</i> -VALUE & DEPTH	CRAWL SPACE <sup>c, i</sup> WALL <i>R</i> -VALUE
6	0.32	0.55	NR	49	20, 13+5	15/20	30 <sup>e</sup>	15	10, 3.5 ft	15
7	0.32	0.55	NR	49	21	19/21	38°	15	10, 5 ft	15

For SI: 1 foot = 304.8 mm.

- a. R-values are minimums. U-factors and SHGC are maximums. When insulation is installed in a cavity that is less than the label or design thickness of the insulation, the installed R-value of the insulation shall not be less than the R-value specified in the table.
- b. The fenestration U-factor column excludes skylights. The SHGC column applies to all glazed fenestration.
- c. See Section R402.2.8.
- d. Insulation R-values for heated slabs shall be installed to the depth indicated or to the top of the footing, whichever is less.
- e. Or insulation sufficient to fill the framing cavity, R-19 minimum.
- f. First value is cavity insulation, second is continuous insulation or insulated siding, so "13+5" means R-13 cavity insulation plus R-5 continuous insulation or insulated siding. If structural sheathing covers 40 percent or less of the exterior, continuous insulation R-value shall be permitted to be reduced by no more than R-3 in the locations where structural sheathing is used to maintain a consistent total sheathing thickness.
- g. The second R-value applies when more than half the insulation is on the interior of the mass wall.
- h. When using log-type construction for thermal mass walls the following applies:
  - (1) a minimum of a 7-inch diameter log shall be used; and
  - (2) the U-value of fenestration products shall be 0.29 overall on average or better.
- i. See Section 402.2.8. A minimum R-19 cavity insulation is required in wood foundation walls.
- j. Roof/ceiling assemblies shall have a minimum 6-inch energy heel.





### SECTION R316 FOAM PLASTIC

**R316.1 General.** The provisions of this section shall govern the materials, design, application, construction and installation of foam plastic materials.

**R316.2 Labeling and identification.** Packages and containers of foam plastic insulation and foam plastic insulation components delivered to the job site shall bear the *label* of an *approved agency* showing the manufacturer's name, the product listing, product identification and information sufficient to determine that the end use will comply with the requirements.

**R316.3 Surface burning characteristics.** Unless otherwise allowed in Section R316.5, foam plastic, or foam plastic cores used as a component in manufactured assemblies, used in building construction shall have a flame spread index of not more than 75 and shall have a smoke-developed index of not more than 450 when tested in the maximum thickness and density intended for use in accordance with ASTM E84 or UL 723. Loose-fill-type foam plastic insulation shall be tested as board stock for the flame spread index and smoke-developed index.

**Exception:** Foam plastic insulation more than 4 inches (102 mm) thick shall have a flame spread index of not more than 75 and a smoke-developed index of not more than 450 where tested at a thickness of not more than 4 inches (102 mm), provided that the end use is *approved* in accordance with Section R316.6 using the thickness and density intended for use.

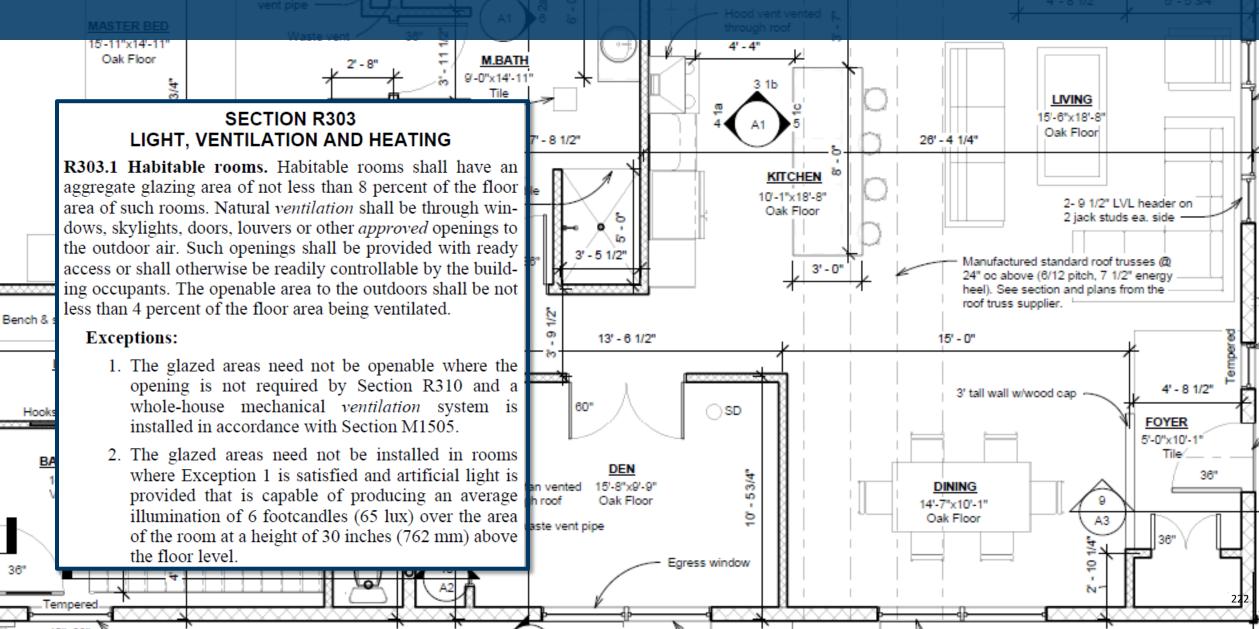
**R316.4 Thermal barrier.** Unless otherwise allowed in Section R316.5, foam plastic shall be separated from the interior of a building by an *approved* thermal barrier of not less than 1/2-inch (12.7 mm) gypsum wallboard, 23/32-inch (18.2 mm) wood structural panel or a material that is tested in accordance with and meets the acceptance criteria of both the Temperature Transmission Fire Test and the Integrity Fire Test of NFPA 275.

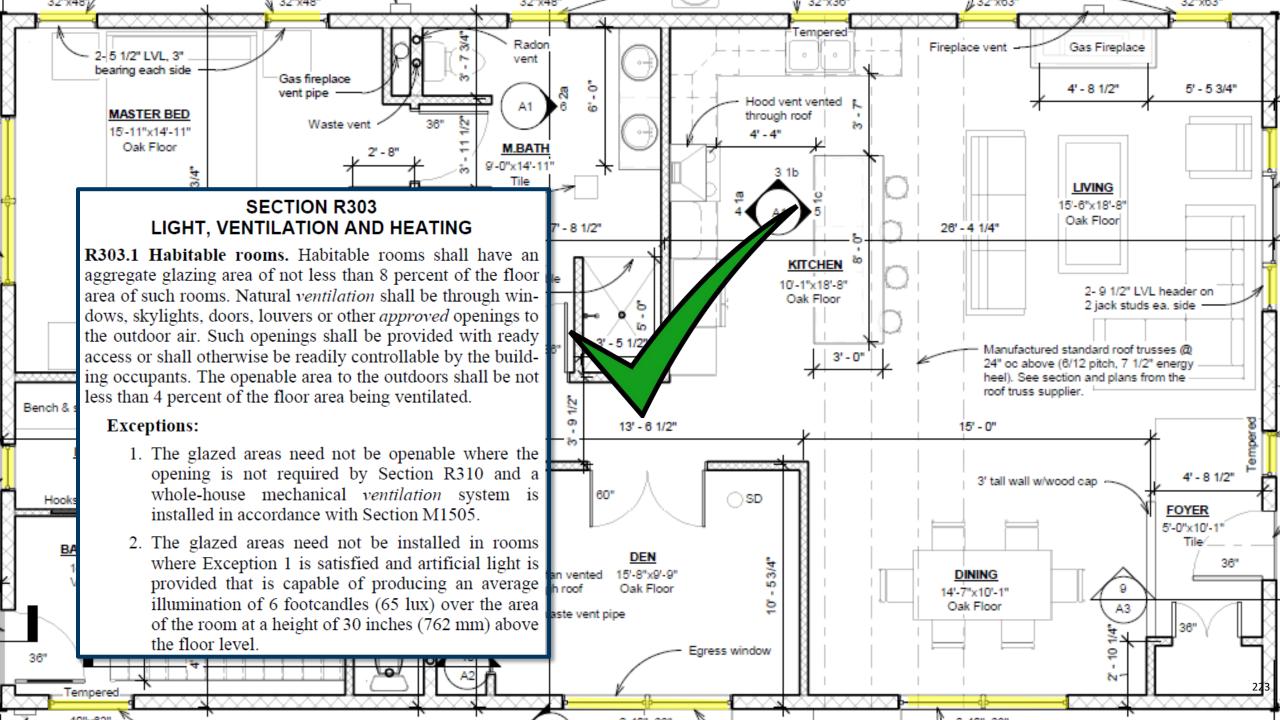
# Light, Ventilation and Heating – Chapter 3



https://knoxinspect.com/2018/10/furnace-vs-boiler-they-are-not-the-same/thompson-heating-and-cooling-duct-work-residential-ductwork/

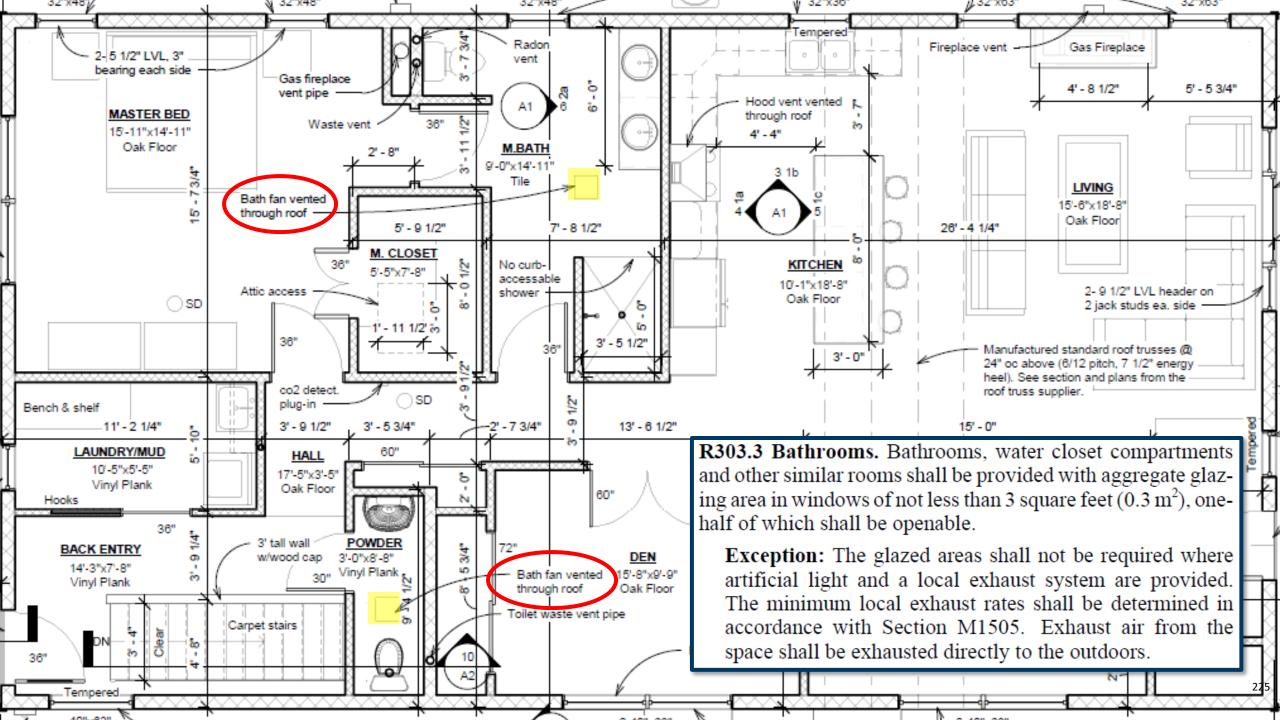
## Light, Ventilation and Heating



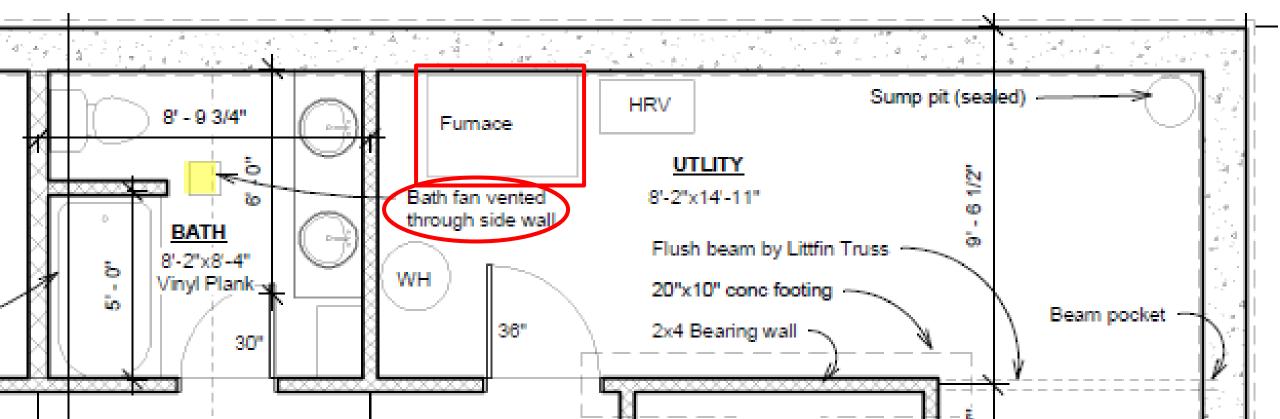


**R303.4 Mechanical ventilation.** Mechanical ventilation of a dwelling unit shall comply with either Minnesota Rules, Chapter 1322 or 1346.





**R303.10 Required heating.** Where the winter design temperature in Table R301.2(1) is below  $60^{\circ}$ F ( $16^{\circ}$ C), every *dwelling unit* shall be provided with heating facilities capable of maintaining a room temperature of not less than <u> $68^{\circ}$ F</u> ( $20^{\circ}$ C) at a point <u>3 feet (914 mm) above the floor and 2 feet</u> (610 mm) from exterior walls in habitable rooms at the design temperature. The installation of one or more portable space heaters shall not be used to achieve compliance with this section.



# Minimum Room Areas

## SECTION R304 MINIMUM ROOM AREAS

**R304.1 Minimum area.** Habitable rooms shall have a floor area of not less than 70 square feet  $(6.5 \text{ m}^2)$ .

Exception: Kitchens.

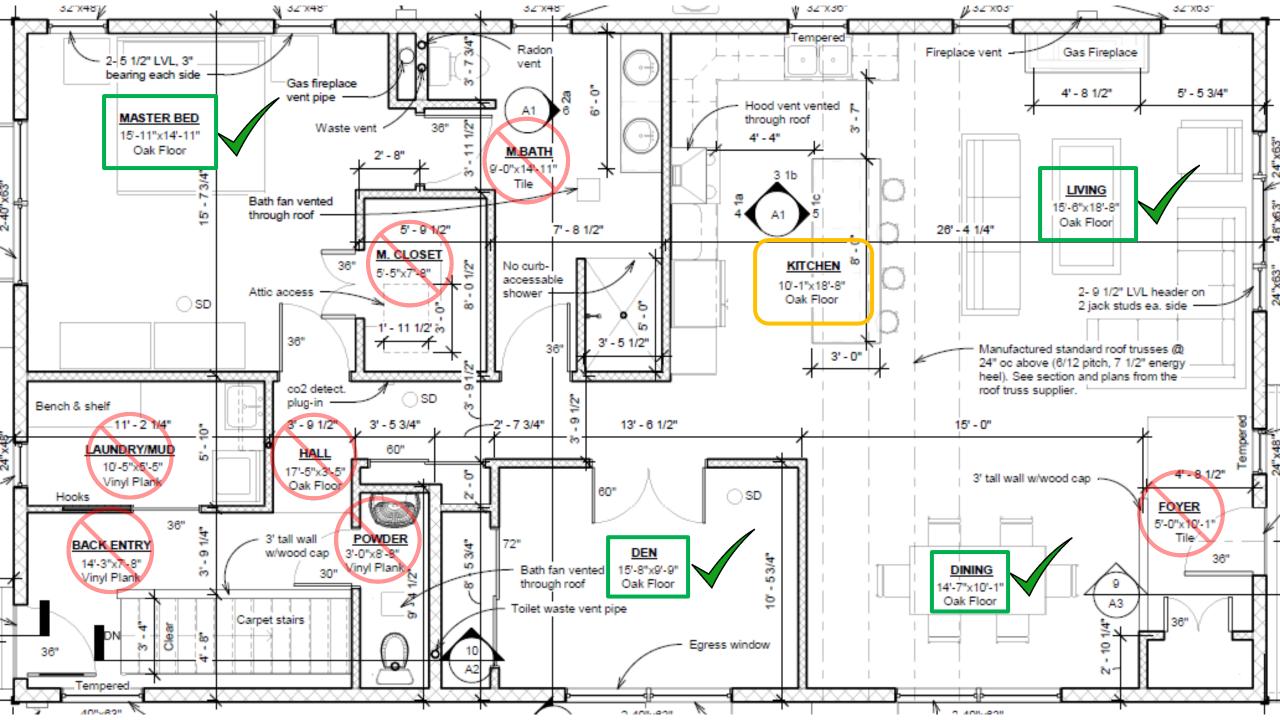
R304.2 Minimum dimensions. Habitable rooms shall be not less than 7 feet (2134 mm) in any horizontal dimension.

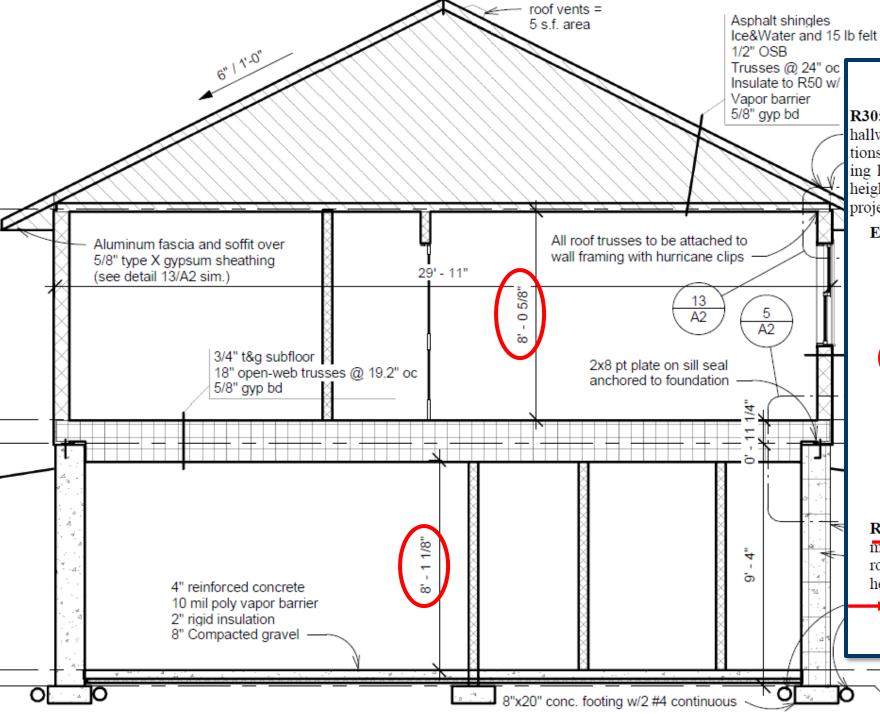
Exception: Kitchens.

**R304.3 Height effect on room area.** Portions of a room with a sloping ceiling measuring less than 5 feet (1524 mm) or a furred ceiling measuring less than 7 feet (2134 mm) from the finished floor to the finished ceiling shall not be considered as contributing to the minimum required habitable area for that room.

[RB] HABITABLE SPACE. A space in a building for living, sleeping, eating or cooking. Bathrooms, toilet rooms, closets, halls, storage or utility spaces and similar areas are not considered *habitable spaces*.







## SECTION R305 CEILING HEIGHT

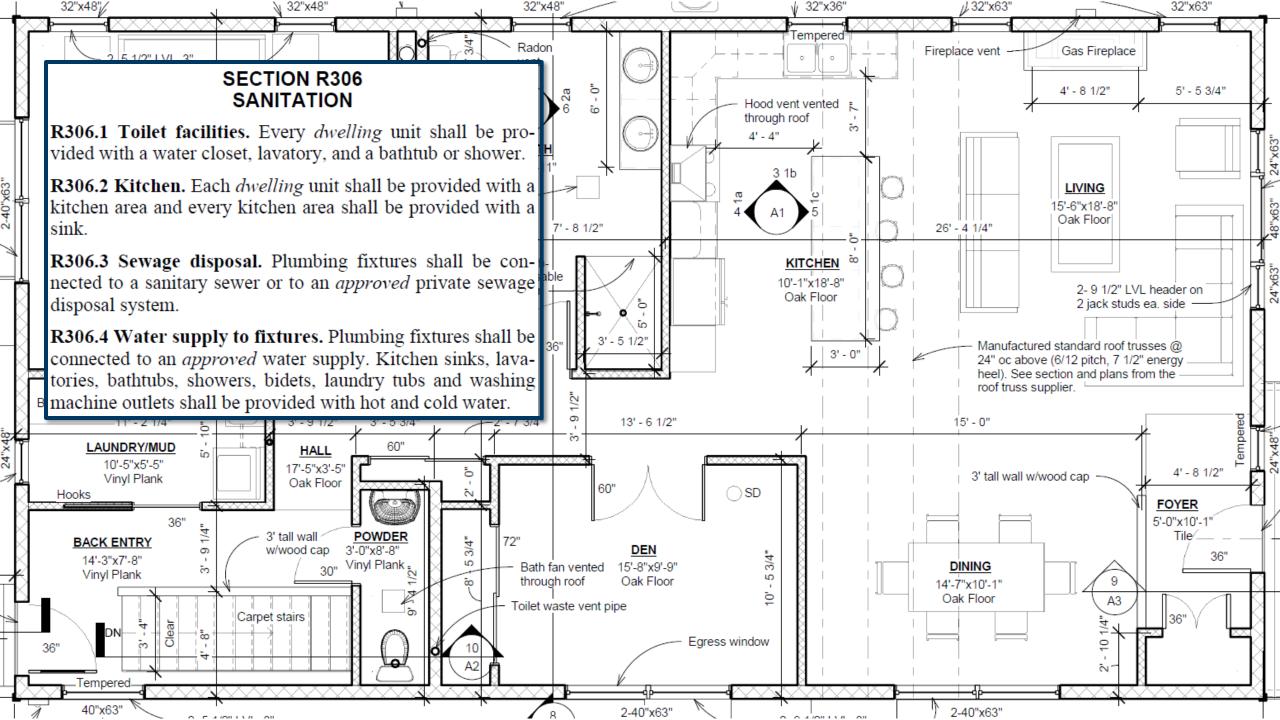
**R305.1 Minimum height, new buildings.** *Habitable space*, hallways, bathrooms, toilet rooms, laundry rooms, and portions of basements containing these spaces shall have a ceiling height of not less than 7 feet (2134 mm). The required height shall be measured from the finish floor to the lowest projection from the ceiling.

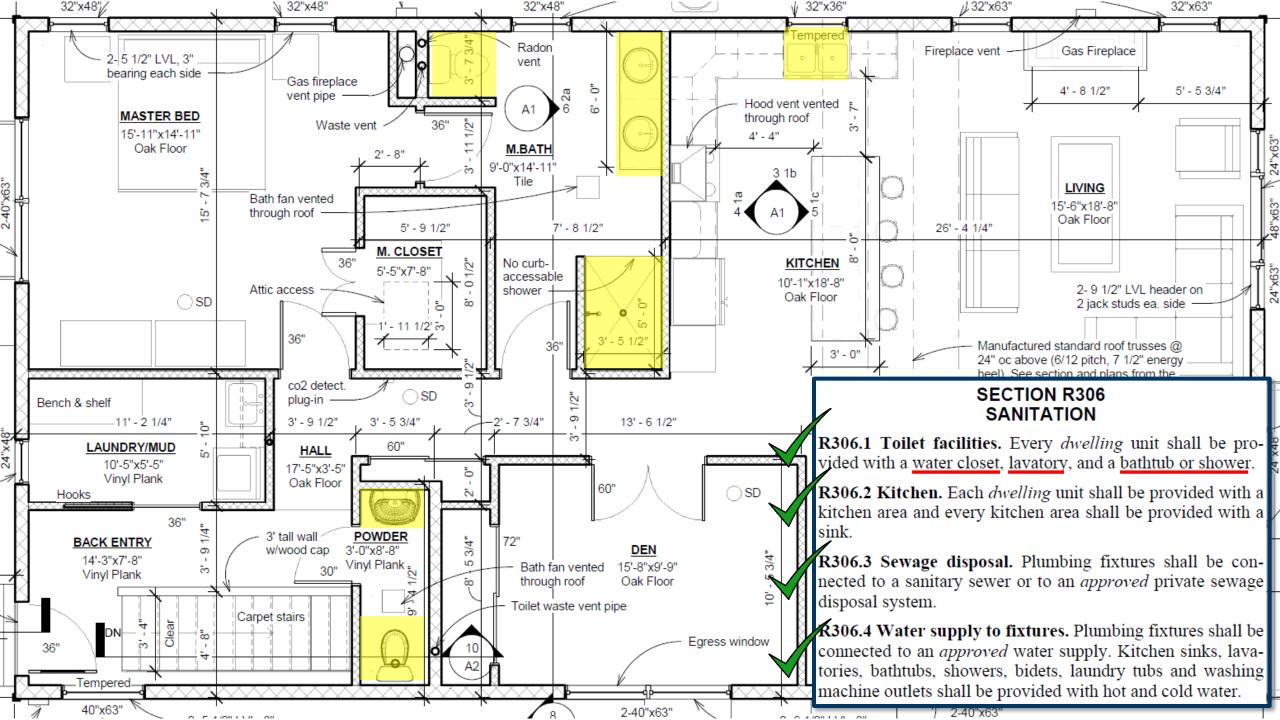
### Exceptions:

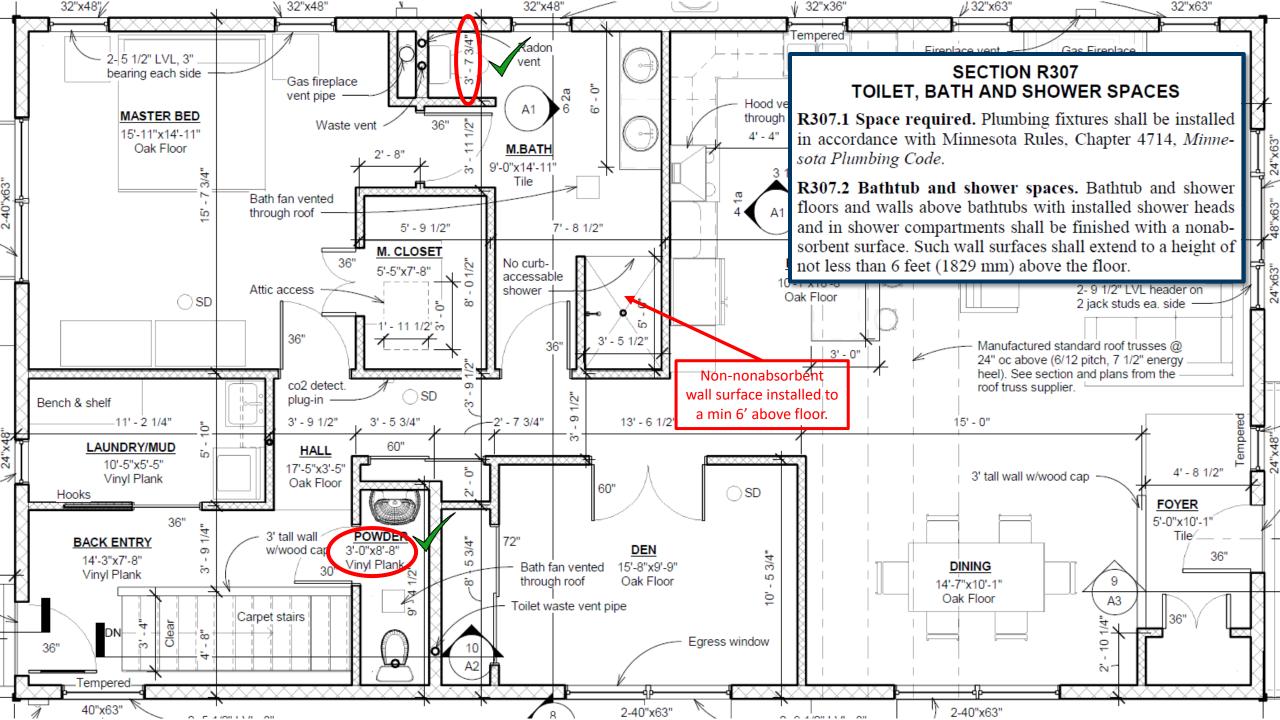
- 1. For rooms with sloped ceilings, at least 50 percent of the required floor area of the room shall have a ceiling height of at least 7 feet (2134 mm) and no portion of the required floor area may have a ceiling height of less than 5 feet (1524 mm).
- 2. Bathrooms shall have a minimum ceiling height of 6 feet 8 inches (2032 mm) at the center of the front clearance area for water closets, bidets, or sinks. The ceiling height above fixtures shall be such that the fixture is capable of being used for its intended purpose. A shower or tub equipped with a showerhead shall have a minimum ceiling height of 6 feet 8 inches (2032 mm) above a minimum area 30 inches (762 mm) by 30 inches (762 mm) at the showerhead.

**R305.1.1 Basements, new buildings.** Portions of basements that do not contain habitable space, hallways, bathrooms, toilet rooms, and laundry rooms shall have a ceiling height of not less than 6 feet 8 inches (2032 mm).

**Exception:** Beams, girders, ducts, or other obstructions may project to within 6 feet 4 inches (1931 mm) of the finished floor.







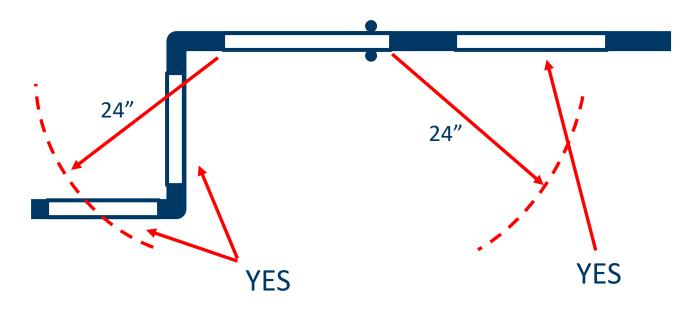


Glazing

- 1. Where the glazing is within 24 inches (610 mm) of either side of the door in the plane of the door in a closed position.
- Where the glazing is on a wall less than 180 degrees (3.14 rad) from the plane of the door in a closed position and within 24 inches (610 mm) of the hinge side of an in-swinging door.

## Exceptions:

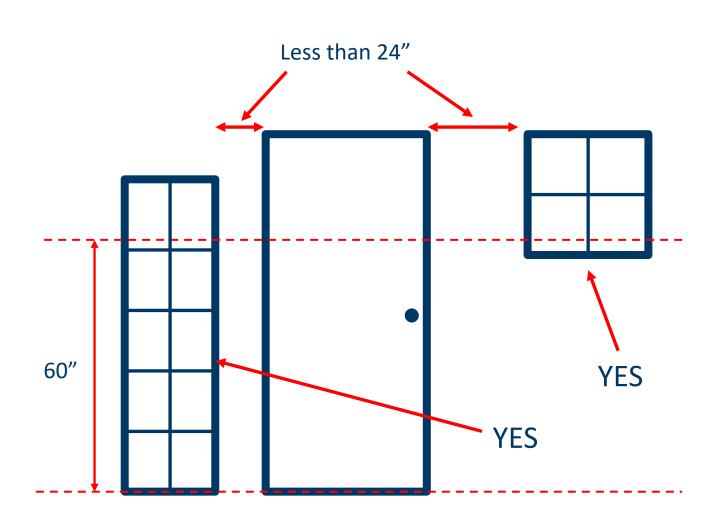
- 1. Decorative glazing.
- Where there is an intervening wall or other permanent barrier between the door and the glazing.
- Where access through the door is to a closet or storage area 3 feet (914 mm) or less in depth. Glazing in this application shall comply with Section R308.4.3.
- Glazing that is adjacent to the fixed panel of patio doors.



- 1. Where the glazing is within 24 inches (610 mm) of either side of the door in the plane of the door in a closed position.
- Where the glazing is on a wall less than 180 degrees (3.14 rad) from the plane of the door in a closed position and within 24 inches (610 mm) of the hinge side of an in-swinging door.

## Exceptions:

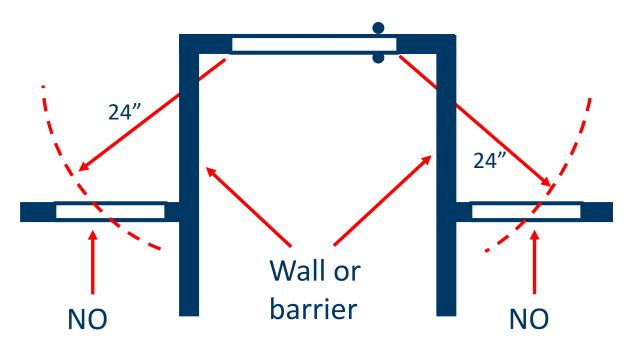
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- 1. Where the glazing is within 24 inches (610 mm) of either side of the door in the plane of the door in a closed position.
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## Exceptions:

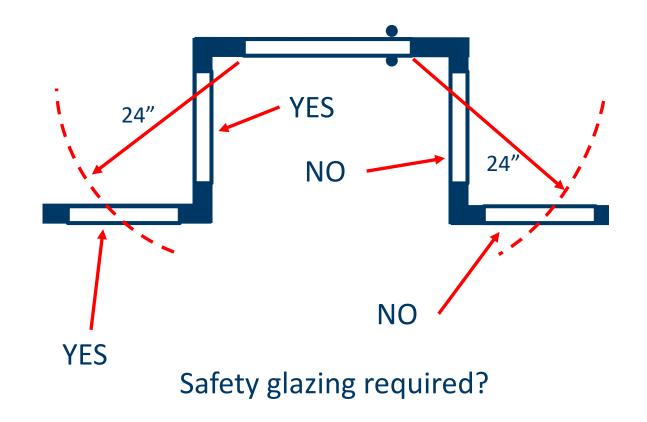
- 1. Decorative glazing.
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- Glazing that is adjacent to the fixed panel of patio doors.

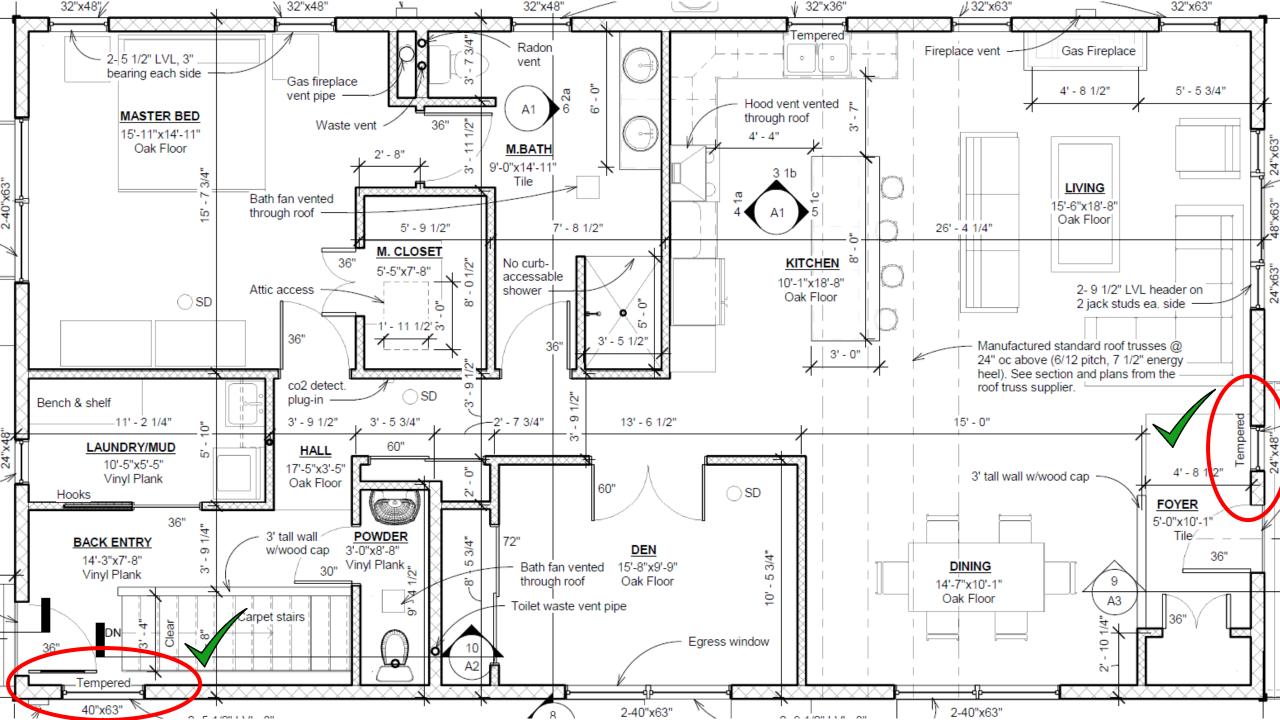


- 1. Where the glazing is within 24 inches (610 mm) of either side of the door in the plane of the door in a closed position.
- Where the glazing is on a wall less than 180 degrees (3.14 rad) from the plane of the door in a closed position and within 24 inches (610 mm) of the hinge side of an in-swinging door.

## Exceptions:

- 1. Decorative glazing.
- Where there is an intervening wall or other permanent barrier between the door and the glazing.
- Where access through the door is to a closet or storage area 3 feet (914 mm) or less in depth. Glazing in this application shall comply with Section R308.4.3.
- Glazing that is adjacent to the fixed panel of patio doors.



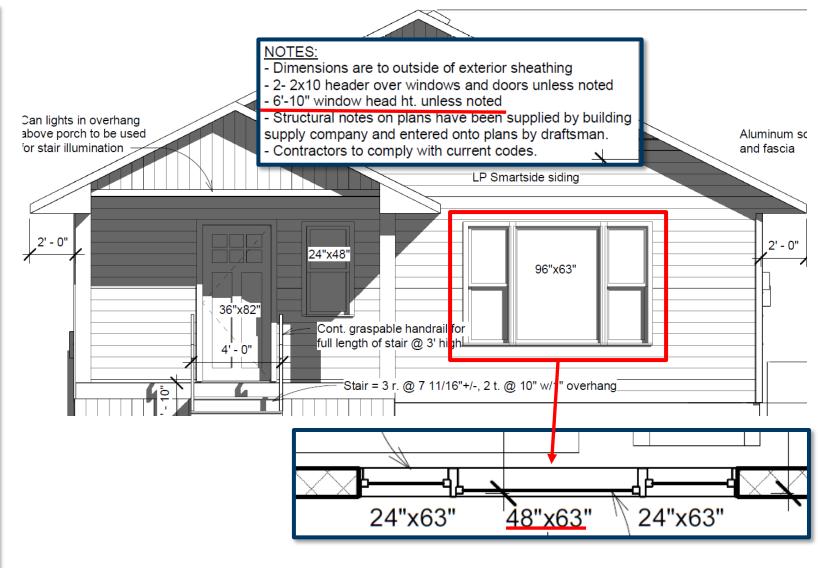


**R308.4.3 Glazing in windows.** Glazing in an individual fixed or operable panel that <u>meets all</u> of the following conditions shall be considered to be a hazardous location:

- The exposed area of an individual pane is larger than 9 square feet (0.836 m<sup>2</sup>).
- The bottom edge of the glazing is less than 18 inches (457 mm) above the floor.
- The top edge of the glazing is more than 36 inches (914 mm) above the floor.
- One or more walking surfaces are within 36 inches (914 mm), measured horizontally and in a straight line, of the glazing.

## Exceptions:

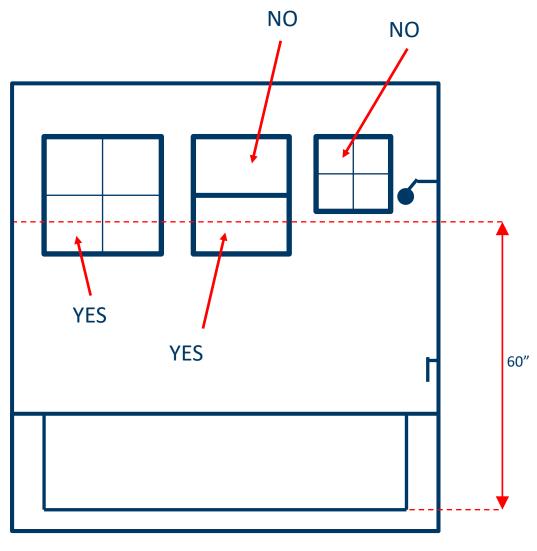
- 1. Decorative glazing.
- 2. Where glazing is adjacent to a walking surface and a horizontal rail is installed 34 to 38 inches (864 to 965 mm) above the walking surface. The rail shall be capable of withstanding a horizontal load of 50 pounds per linear foot (730 N/m) without contacting the glass and have a cross-sectional height of not less than 1<sup>1</sup>/<sub>2</sub> inches (38 mm).
- 3. Outboard panes in insulating glass units and other multiple glazed panels where the bottom edge of the glass is 25 feet (7620 mm) or more above *grade*, a roof, walking surfaces or other horizontal [within 45 degrees (0.79 rad) of horizontal] surface adjacent to the glass exterior.



- #1. 4 x 5 = 20sf Yes
- #2. 82" 63" = 19" (Approximately) Maybe...?
- #3. Yes
- #4. Yes
- Must be Tempered? Maybe contingent on criteria #2.

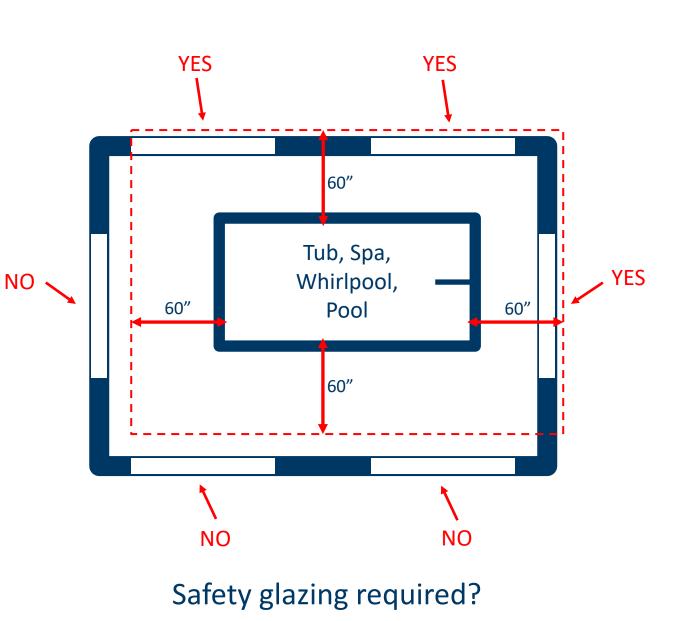
**R308.4.5 Glazing and wet surfaces.** Glazing in walls, enclosures or fences containing or facing hot tubs, spas, whirlpools, saunas, steam rooms, bathtubs, showers and indoor or outdoor swimming pools where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface shall be considered to be a hazardous location. This shall apply to single glazing and each pane in multiple glazing.

**Exception:** Glazing that is more than 60 inches (1524 mm), measured horizontally and in a straight line, from the water's edge of a bathtub, hot tub, spa, whirlpool or swimming pool or from the edge of a shower, sauna or steam room.



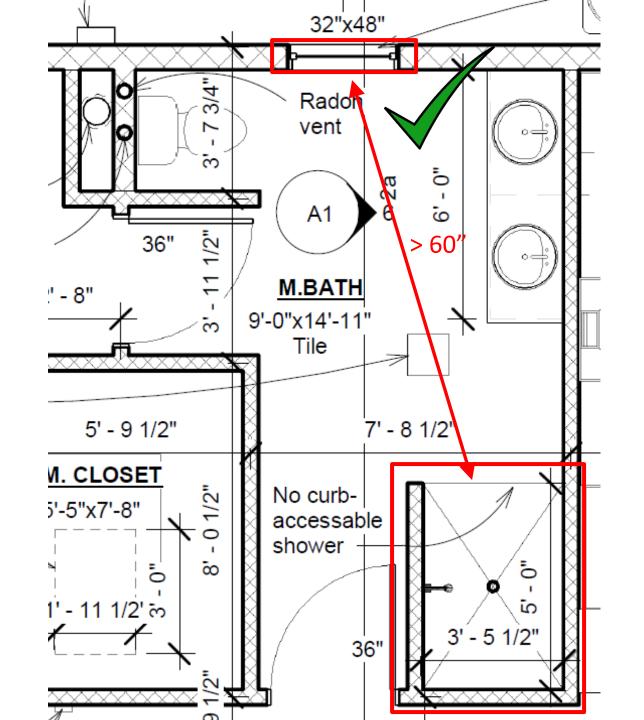
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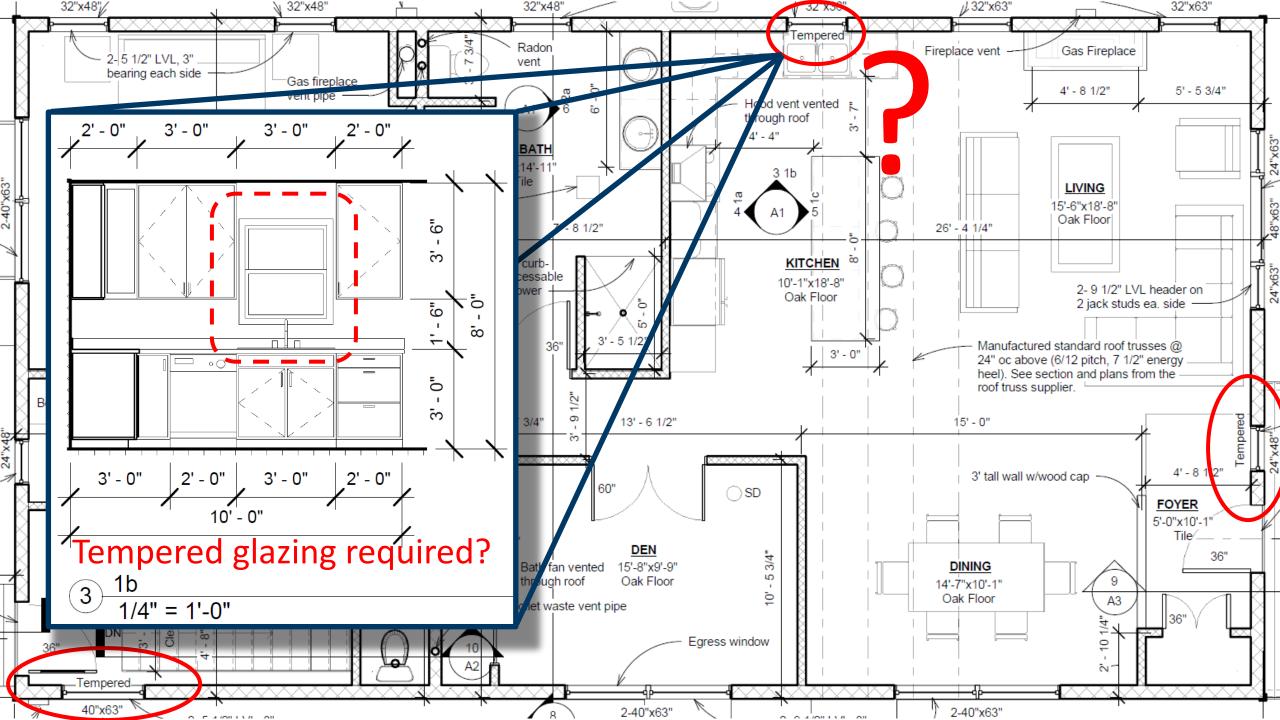
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**Exception:** Glazing that is more than 60 inches (1524 mm), measured horizontally and in a straight line, from the water's edge of a bathtub, hot tub, spa, whirlpool or swimming pool or from the edge of a shower, sauna or steam room.





**R308.4.6 Glazing adjacent to stairs and ramps.** Glazing where the bottom exposed edge of the glazing is less than 36 inches (914 mm) above the plane of the adjacent walking surface of stairways, landings between flights of stairs and ramps shall be considered to be a hazardous location.

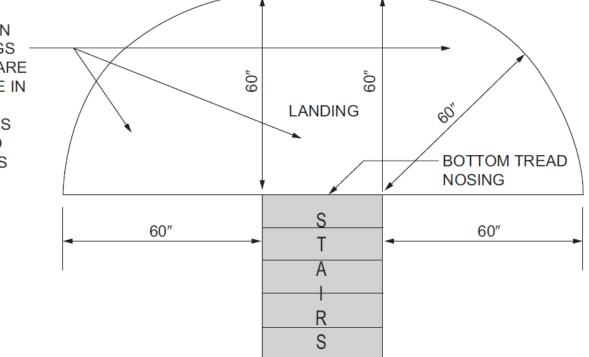
## Exceptions:

- Where glazing is adjacent to a walking surface and a horizontal rail is installed at 34 to 38 inches (864 to 965 mm) above the walking surface. The rail shall be capable of withstanding a horizontal load of 50 pounds per linear foot (730 N/m) without contacting the glass and have a cross-sectional height of not less than 1<sup>1</sup>/<sub>2</sub> inches (38 mm).
- 2. Glazing 36 inches (914 mm) or more measured horizontally from the walking surface.

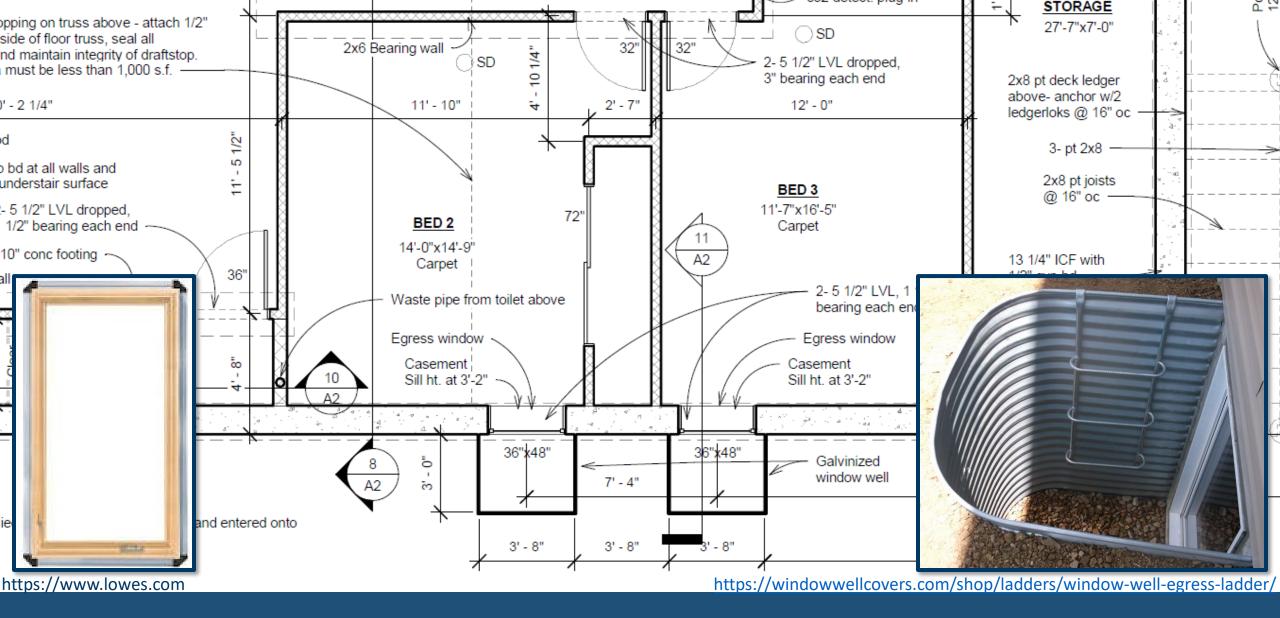
**R308.4.7 Glazing adjacent to the bottom stair landing.** Glazing adjacent to the landing at the bottom of a stairway where the glazing is less than 36 inches (914 mm) above the landing and within a 60-inch (1524 mm) horizontal arc less than 180 degrees (3.14 rad) from the bottom tread nosing shall be considered to be a hazardous location. (See Figure R308.4.7.)

**Exception:** Where the glazing is protected by a *guard* complying with Section R312 and the plane of the glass is more than 18 inches (457 mm) from the *guard*.

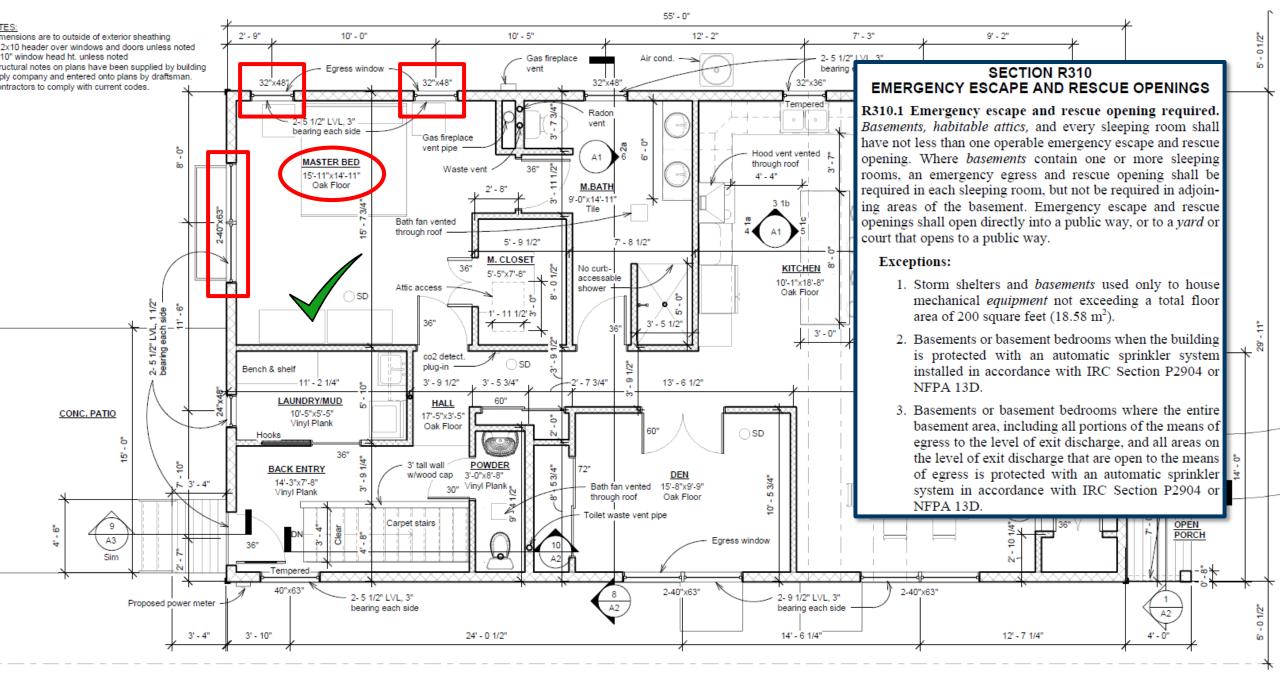
GLAZING LESS THAN 36" ABOVE LANDINGS WITHIN THIS AREA ARE CONSIDERED TO BE IN HAZARDOUS LOCATIONS, UNLESS THE EXCEPTION TO SECTION R308.4.7 IS SATISFIED



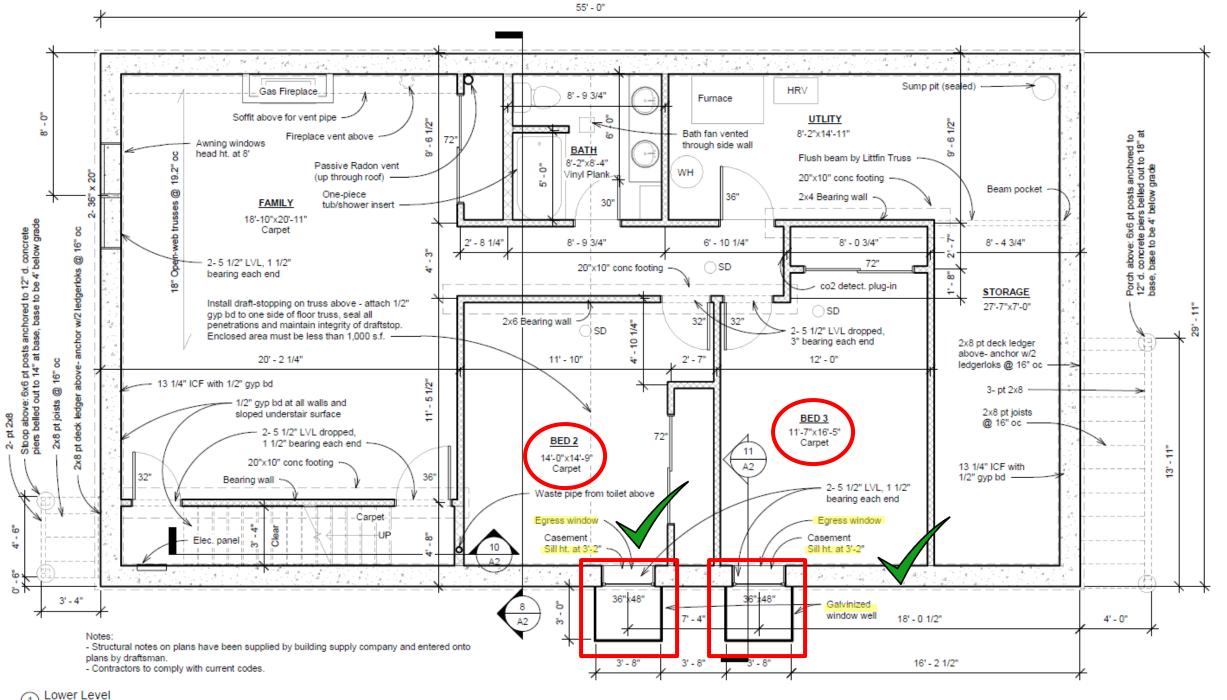
### FIGURE R308.4.7 HAZARDOUS GLAZING LOCATIONS AT BOTTOM STAIR LANDINGS



**Emergency Escape & Rescue Openings** 



Main Level 1/4" = 1'-0"



1/4" = 1'-0"

R309.2 Carports. Carports shall be open on at least two sides. Carport floor surfaces may be concrete, asphalt, sand, gravel, crushed rock, or natural earth. Carports not open on at least two sides shall be considered a garage and shall comply with the provisions of this section for garages.

R309.3 Flood hazard areas. See Minnesota Rules, Chapter 1335.

R309.4 Automatic garage door opening systems. All automatic garage door opening systems that are installed, serviced, or repaired for garages serving residential buildings shall comply with the provisions of Minnesota Statutes, Sections 325F.82 and 325F.83.

R309.5 Fire sprinklers. Attached garages of two-family dwellings and townhouses shall be protected by fire sprinklers and installed in compliance with Section R313.3.

#### SECTION R310 EMERGENCY ESCAPE AND RESCUE OPENINGS

R310.1 Emergency escape and rescue opening required. Basements, habitable attics, and every sleeping room shall have not less than one operable emergency escape and rescue opening. Where basements contain one or more sleeping rooms, an emergency egress and rescue opening shall be required in each sleeping room, but not be required in adjoining areas of the basement. Emergency escape and rescue openings shall open directly into a public way, or to a yard or court that opens to a public way.

#### Exceptions:

- Storm shelters and basements used only to house mechanical equipment not exceeding a total floor area of 200 square feet (18.58 m<sup>2</sup>).
- Basements or basement bedrooms when the building is protected with an automatic sprinkler system installed in accordance with IRC Section P2904 or NFPA 13D.
- 3. Basements or basement bedrooms where the entire basement area, including all portions of the means of egress to the level of exit discharge, and all areas on the level of exit discharge that are open to the means of egress is protected with an automatic sprinkler system in accordance with IRC Section P2904 or NFPA 13D.

R310.1.1 Operational constraints and opening control devices. Emergency escape and rescue openings shall be operational from the inside of the room without the use of keys, tools or special knowledge. Window opening control devices on windows serving as a required emergency escape and rescue opening shall comply with ASTM F2090.

R310.2 Emergency escape and rescue openings. Emergency escape and rescue openings shall have minimum dimensions as specified in this section.

R310.2.1 Minimum opening area. Emergency and escape rescue openings shall have a net clear opening of not less than 5.7 square feet (0.530 m<sup>2</sup>). The net clear opening

dimensions required by this section shall be obtained by the normal operation of the emergency escape and rescue opening from the inside. The net clear height of the opening shall be not less than 24 inches (610 mm) and the net clear width shall be not less than 20 inches (508 mm).

Exception: Grade floor openings or below-grade openings shall have a net clear opening area of not less than 5 square feet (0.465 m<sup>2</sup>).

R310.2.2 Window sill height. Where a window is provided as the emergency escape and rescue opening, it shall have a sill height of not more than 44 inches (1118 mm) above the floor; where the sill height is below grade, it shall be provided with a window well in accordance with Section R310.2.3.

R310.2.3 Window wells. The horizontal area of the window well shall be not less than 9 square feet (0.9 m<sup>2</sup>), with a horizontal projection and width of not less than 36 inches (914 mm). The area of the window well shall allow the emergency escape and rescue opening to be fully opened.

Exception: The ladder or steps required by Section R310.2.3.1 shall be permitted to encroach not more than 6 inches (152 mm) into the required dimensions of the window well.

R310.2.3.1 Ladder and steps. Window wells with a vertical depth greater than 44 inches (1118 mm) shall be equipped with a permanently affixed ladder or steps usable with the window in the fully open position. Ladders or steps required by this section shall not be required to comply with Section R311.7. Ladders or rungs shall have an inside width of not less than 12 inches (305 mm), shall project not less than 3 inches (76 mm) from the wall and shall be spaced not more than 18 inches (457 mm) on center vertically for the full height of the window well.

R310.2.3.2 Drainage. Window wells shall be designed for proper drainage by connecting to the building's foundation drainage system required by Section R405.1 or by an approved alternative method.

Exception: A drainage system for window wells is not required where the foundation is on well-drained soil or sand-gravel mixture soils in accordance with the United Soil Classification System, Group I Soils, as detailed in Table R405.1.

R310.2.4 Emergency escape and rescue openings under decks and porches. Emergency escape and rescue openings installed under decks and porches shall be fully openable and provide a path not less than 36 inches (914 mm) in height to a yard or court.

R310.2.5 Replacement windows. Replacement windows installed in buildings meeting the scope of this code shall be exempt from the maximum sill height requirements of Section R310.2.2 and the requirements of Section R310.2.1, provided that the replacement window meets the following conditions:

 The replacement window is the manufacturer's largest standard size window that will fit within the existing frame or existing rough opening. The replacement window is of the same operating style as the existing window or a style that provides for an equal or greater window opening area than the existing window.

The replacement window is not part of a change of occupancy.

R310.2.5.1 Licensed facilities. Windows in rooms used for foster care or day care licensed or registered by the state of Minnesota shall comply with the provisions of Section R310.2.5, or all of the following conditions, whichever is more restrictive:

- Minimum of 20 inches (508 mm) in clear opening width;
- Minimum of 20 inches (508 mm) in clear opening height;
- Minimum of 648 square inches (4.5 square feet) clear opening; and
- Maximum of 48 inches (1219 mm) from the floor to the sill height.

R310.3 Emergency escape and rescue doors. Where a door is provided as the required emergency escape and rescue opening, it shall be a side-hinged door or a slider. Where the opening is below the adjacent grade, it shall be provided with an area well.

R310.3.1 Minimum door opening size. The minimum net clear height opening for any door that serves as an emergency and escape rescue opening shall be in accordance with Section R310.2.1.

R310.3.2 Area wells. Area wells shall have a width of not less than 36 inches (914 mm). The area well shall be sized to allow the emergency escape and rescue door to be fully opened.

R310.3.2.1 Ladder and steps. Area wells with a vertical depth greater than 44 inches (1118 mm) shall be equipped with a permanently affixed ladder or steps usable with the door in the fully open position. Ladders or steps required by this section shall not be required to comply with Section R311.7. Ladders or rungs shall have an inside width of not less than 12 inches (305 mm), shall project not less than 3 inches (76 mm) from the wall and shall be spaced not more than 18 inches (457 mm) on center vertically for the full height of the exterior stairwell.

R310.3.2.2 Drainage. Area wells shall be designed for proper drainage by connecting to the building's foundation drainage system required by Section R405.1 or by an *approved* alternative method.

Exception: A drainage system for area wells is not required where the foundation is on well-drained soil or sand-gravel mixture soils in accordance with the United Soil Classification System, Group I Soils, as detailed in Table R405.1.

R310.4 Bars, grilles, covers and screens. Where bars, grilles, covers, screens or similar devices are placed over emergency escape and rescue openings, area wells, or window wells, the minimum net clear opening size shall comply with Sections R310.2.1 through R310.2.3, and such devices shall be releasable or removable from the inside without the use of a key, tool, special knowledge or force greater than that required for the normal operation of the escape and rescue opening.

R310.5 Dwelling additions. Where dwelling additions contain sleeping rooms, an emergency escape and rescue opening shall be provided in each new sleeping room. Where dwelling additions have basements, an emergency escape and rescue opening shall be provided in the new basement.

#### Exceptions:

- An emergency escape and rescue opening is not required in a new basement that contains a sleeping room with an emergency escape and rescue opening.
- An emergency escape and rescue opening is not required in a new basement where there is an emergency escape and rescue opening in an existing basement that is accessed from the new basement.

R310.6 Alterations or repairs of existing basements. An emergency escape and rescue opening is not required where existing *basements* undergo alterations or repairs.

R310.6.1 Sleeping rooms in existing basements. New sleeping rooms created in an existing basement shall be provided with emergency escape and rescue openings in accordance with Section R310.1.

Exception: Emergency escape and rescue openings are not required to be provided where the entire basement area, including all portions of the means of egress to the level of exit discharge, and all areas on the level of exit discharge that are open to the means of egress are protected with an automatic sprinkler system in accordance with IRC Section P2904 or NFPA 13D.

#### SECTION R311 MEANS OF EGRESS

**R311.1 Means of egress**. *Dwellings* shall be provided with a means of egress in accordance with this section. The means of egress shall provide a continuous and unobstructed path of vertical and horizontal egress travel from all portions of the *dwelling* to the required egress door without requiring travel through a garage. The required egress door shall open directly into a public way or to a yard or court that opens to a public way.

R311.2 Egress door. Not less than one egress door shall be provided for each *dwelling* unit. The egress door shall be side-hinged, and shall provide a clear width of not less than 32 inches (813 mm) where measured between the face of the door and the stop, with the door open 90 degrees (1.57 rad). The clear height of the door opening shall be not less than 78 inches (1981 mm) in height measured from the top of the threshold to the bottom of the stop. Other doors shall not be required to comply with these minimum dimensions. Egress doors shall be readily openable from inside the *dwelling* without the use of a key or special knowledge or effort.

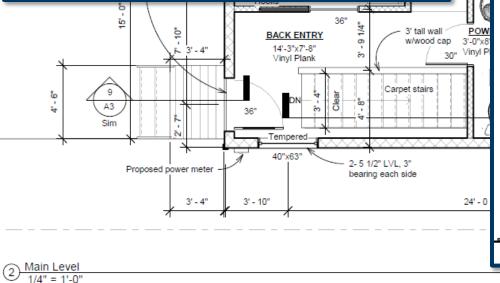


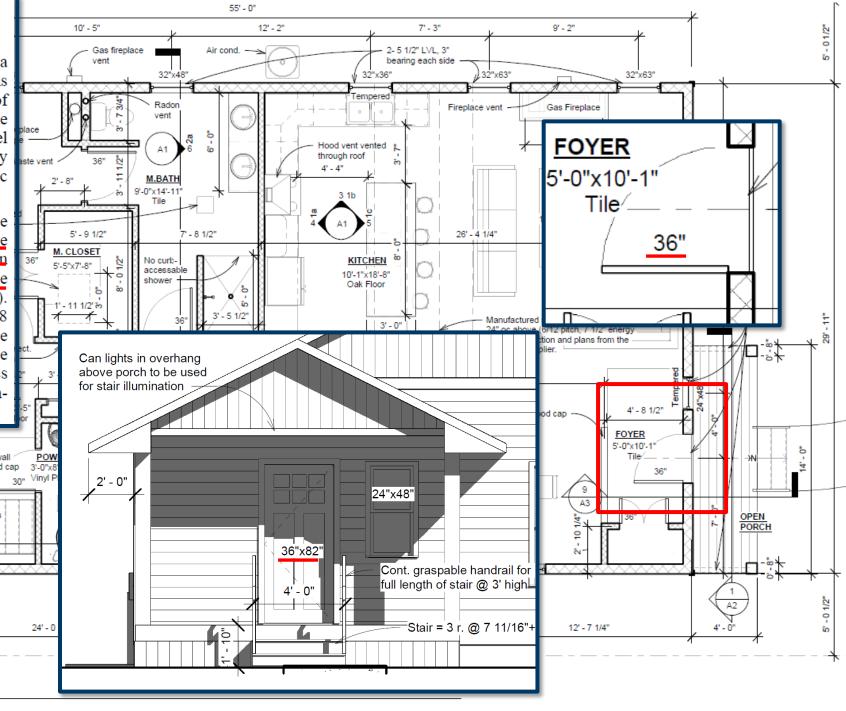
Means of Egress

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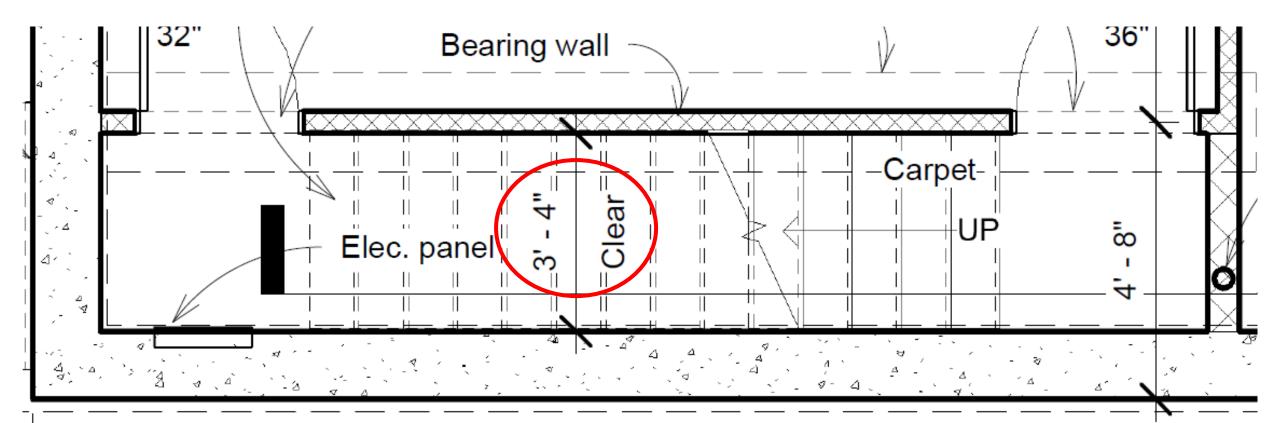
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**R311.7.1.2 Width.** Stairways shall be not less than 36 inches (914 mm) in clear width at all points above the permitted handrail height and below the required headroom height. Handrails shall not project more than 4.5 inches (114 mm) on either side of the stairway and the minimum clear width of the stairway at and below the handrail height, including treads and landings, shall be not less than  $31^{1}/_{2}$  inches (787 mm) where a handrail is installed on one side and 27 inches (698 mm) where handrails are provided on both sides.

**Exception:** The width of spiral stairways shall be in accordance with Section R311.7.10.1.

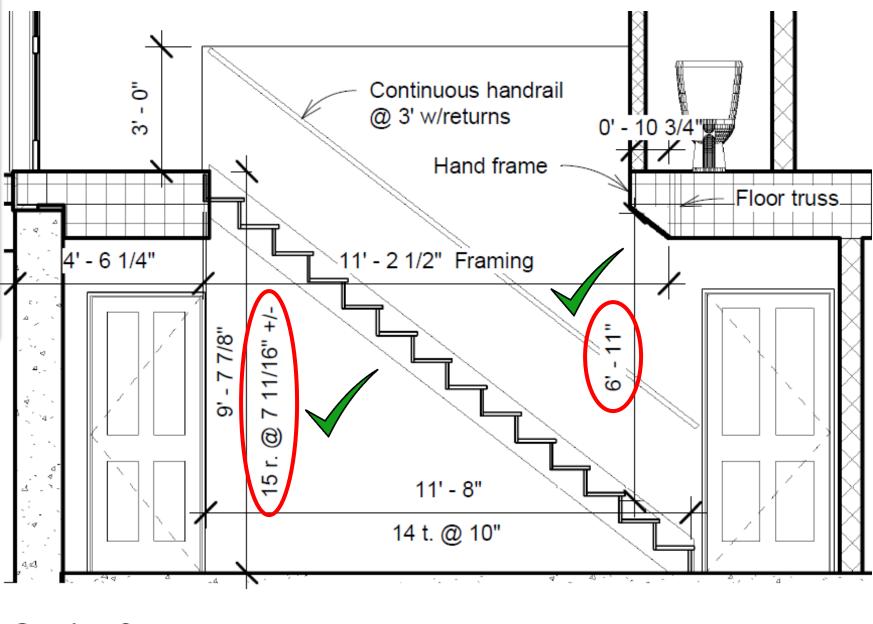


**R311.7.2 Headroom.** The minimum headroom in all parts of the stairway shall be not less than <u>6 feet 8 inches</u> (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stairway.

## Exceptions:

- 1. Where the nosings of treads at the side of a flight extend under the edge of a floor opening through which the stair passes, the floor opening shall be allowed to project horizontally into the required headroom a maximum of  $4^3/_4$  inches (121 mm).
- 2. The minimum headroom for existing buildings shall be in accordance with Section R305.2.2.
- 3. The headroom for spiral stairways shall be in accordance with Section R311.7.10.1.

**R311.7.3 Vertical rise.** A flight of stairs shall not have a vertical rise larger than <u>151 inches</u> (3835 mm) between floor levels or landings.

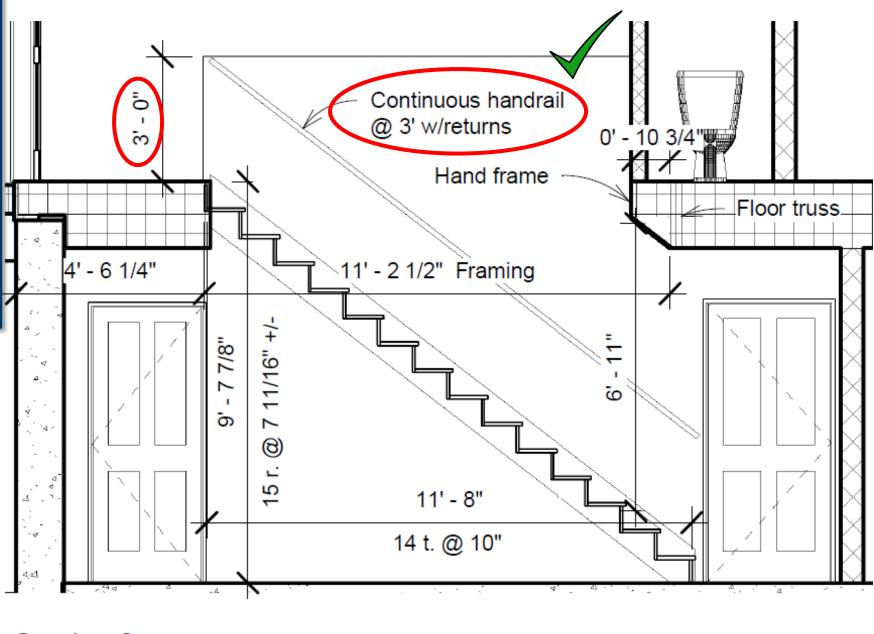


**R311.7.8 Handrails.** Handrails shall be provided on not less than one side of each flight of stairs with four or more risers.

**R311.7.8.1 Height.** Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm) and not more than 38 inches (965 mm).

## Exceptions:

- 1. The use of a volute, turnout or starting easing shall be allowed over the lowest tread.
- 2. Where handrail fittings or bendings are used to provide continuous transition between flights, transitions at winder treads, the transition from handrail to *guard*, or used at the start of a flight, the handrail height at the fittings or bendings shall be permitted to exceed 38 inches (956 mm).



## 1/2" PLYWOOD SECURED TO DBL STAIR HDR W/4 - 8DCC NAILS PER STRINGER STRINGER SECURED TO PLYWOOD W/4 - 8DCC NAILS PER STRINGER 4" MAX. NAILS PER STRINGER Shall not allow 4" DIA, MAXIMUM passage of? 2 Shall not allow passage of?

## STAIR DETAIL

· DOUBLE JOIST AROUND STAIR OPENING

· 3-2X12 STRINGERS

IX8 PINE RISERS

5/4 PRACTICAL BOARD TREADS

SECURED TO STRINGERS W/4-16DCC

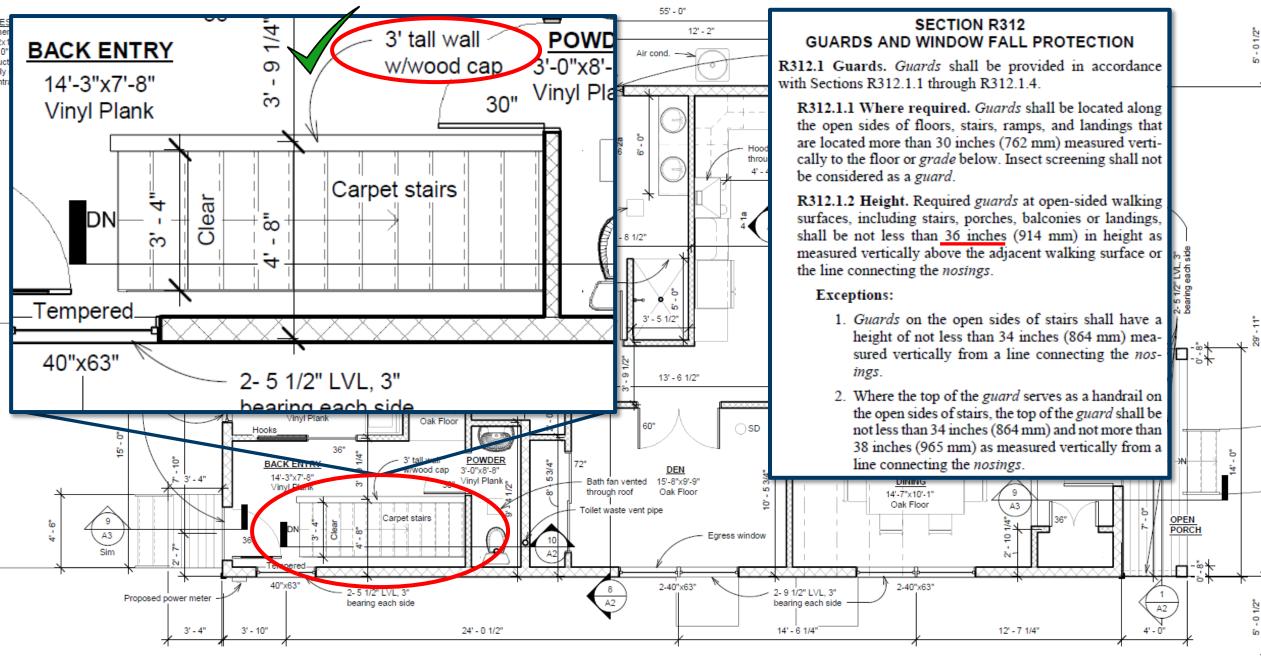
\* MAXIMUM RISE OF 1 3/4" & MINIMUM TREAD OF 10" A NOSING OF NOT LESS THAN 3/4" OR MORE THAN 1 1/2" IS REQUIRED, NOSING IS NOT REQUIRED IF TREAD IN MINIMUM OF 11", THE RISER MAY BE OPEN PROVIDED OPENING DOES NOT PERMIT THE PASSAGE OF A 4" SPHERE

\* ENCLOSED UNDER SIDE OF STAIR WITH 1/2" SHEETROCK · PROVIDE HANDRAIL 34" - 38" HIGH WITH 1 1/4" - 2" CROSS SECTIONAL DIMENSION TO BE CONTINUOUS & ENDS RETURNED TO WALL \* RAILING TO BE MINIMUM OF 36" IN HEIGHT WITH SPINDLES NOT TO PERMIT THE PASSAGE OF A 4" SPHERE 6'8" MIN HEADROOM MEASURED

VERTICAL FROM THE STAIR NOSING \* 5/8 TYPE X SHEETROCK ON CEILING UNDER STEPS, 1/2 " ON WALLS



## Guards & Window Fall Protection



2 Main Level 1/4" = 1'-0"

**R312.2 Window fall protection.** Window fall protection shall be provided in accordance with Sections R312.2.1 and R312.2.2.

**R312.2.1 Window sills.** In dwelling units, where the lowest part of the opening of an operable window is located more than 72 inches (1829 mm) above the finished grade or surface below, the lowest part of the window opening shall be a minimum of 36 inches (914 mm) above the finished floor of the room in which the window is located. Operable sections of windows shall not permit openings that allow passage of a 4-inch diameter (102 mm) sphere where such openings are located within 36 inches (914 mm) of the finished floor.

### Exceptions:

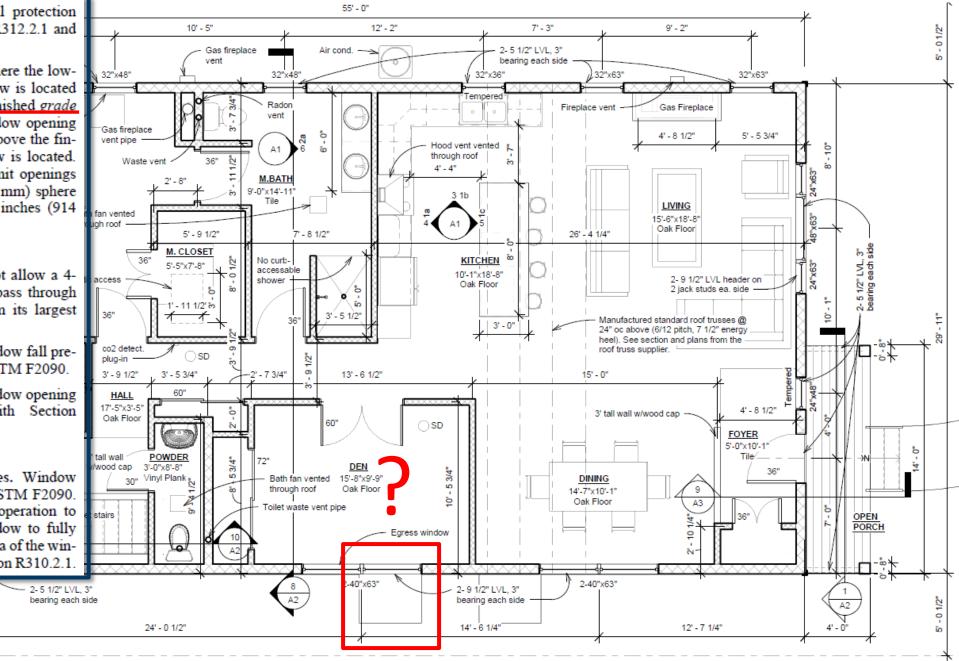
- Windows with openings that will not allow a 4inch-diameter (102 mm) sphere to pass through the opening when the window is in its largest opened position.
- Openings that are provided with window fall prevention devices that comply with ASTM F2090.
- Windows that are provided with window opening control devices that comply with Section R312.2.2.
- 4. Replacement windows.

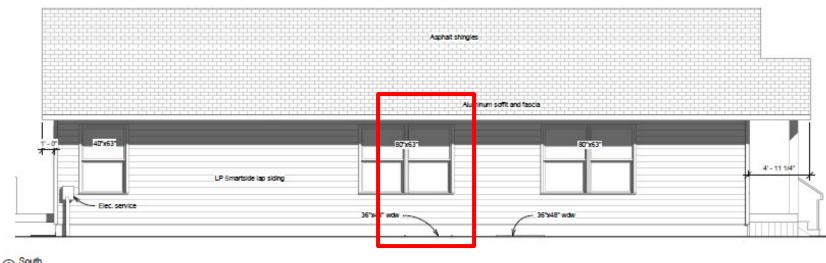
Proposed power meter

3' - 4"

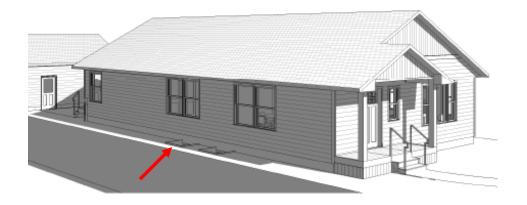
**R312.2.2** Window opening control devices. Window opening control devices shall comply with ASTM F2090. The window opening control device, after operation to release the control device allowing the window to fully open, shall not reduce the net clear opening area of the window unit to less than the area required by Section R310.2.1.

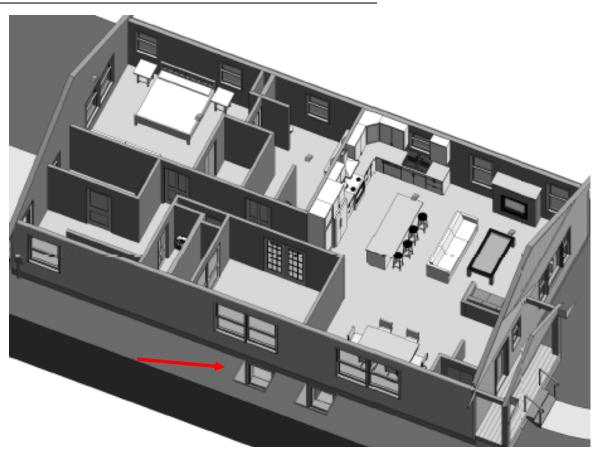
3' - 10"





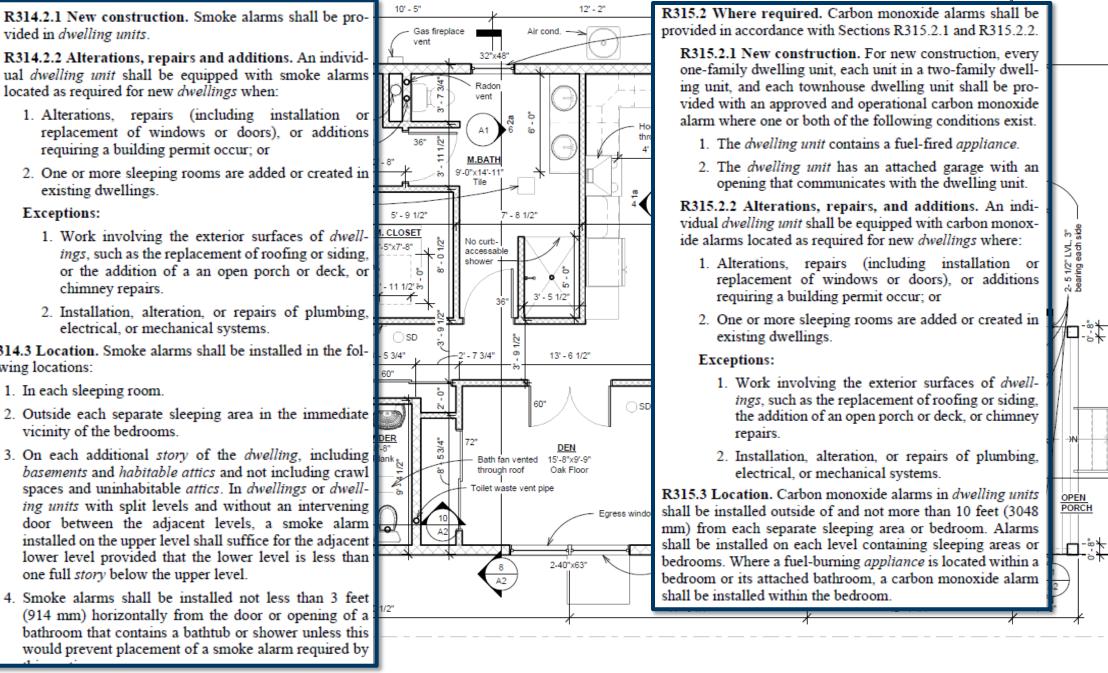
() South 1/4" = 1'-0"







## Smoke/CO Alarms



2x10 header over win 0" window head ht u uctural notes on plans ply company and enter intractors to comply wit

CONC.

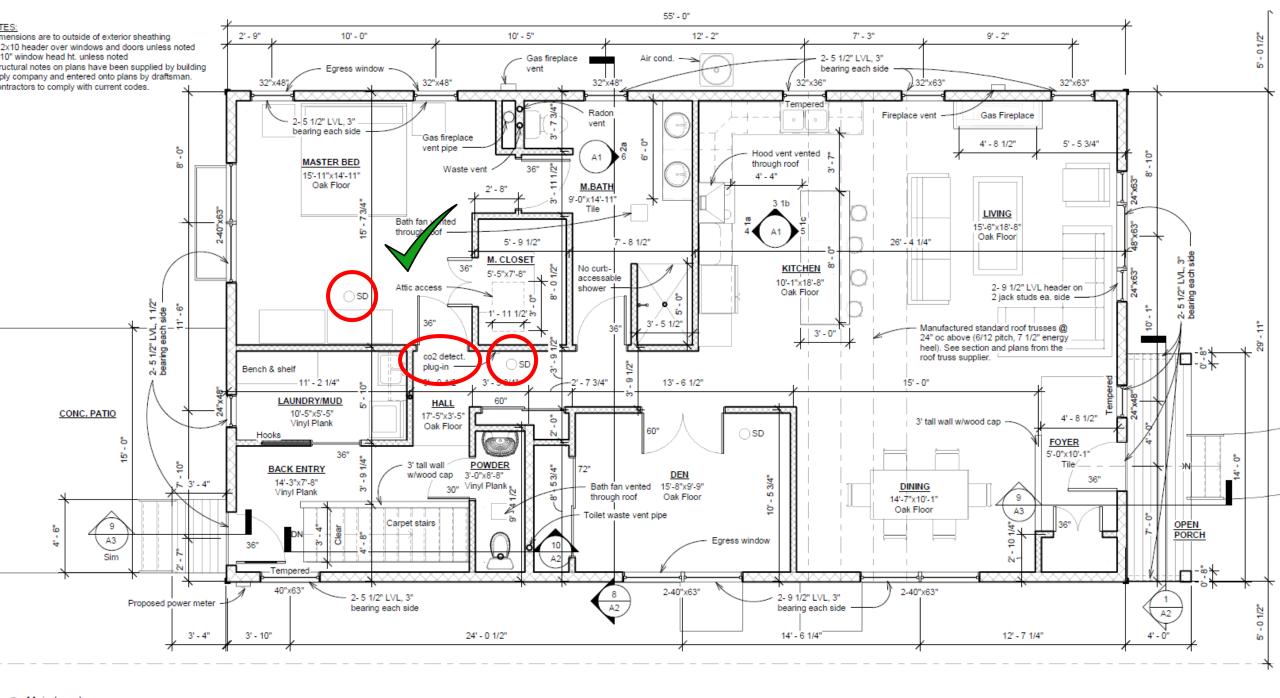
2 Main Level  $1/4'' = 1'_{-}0'$ 

Exceptions:

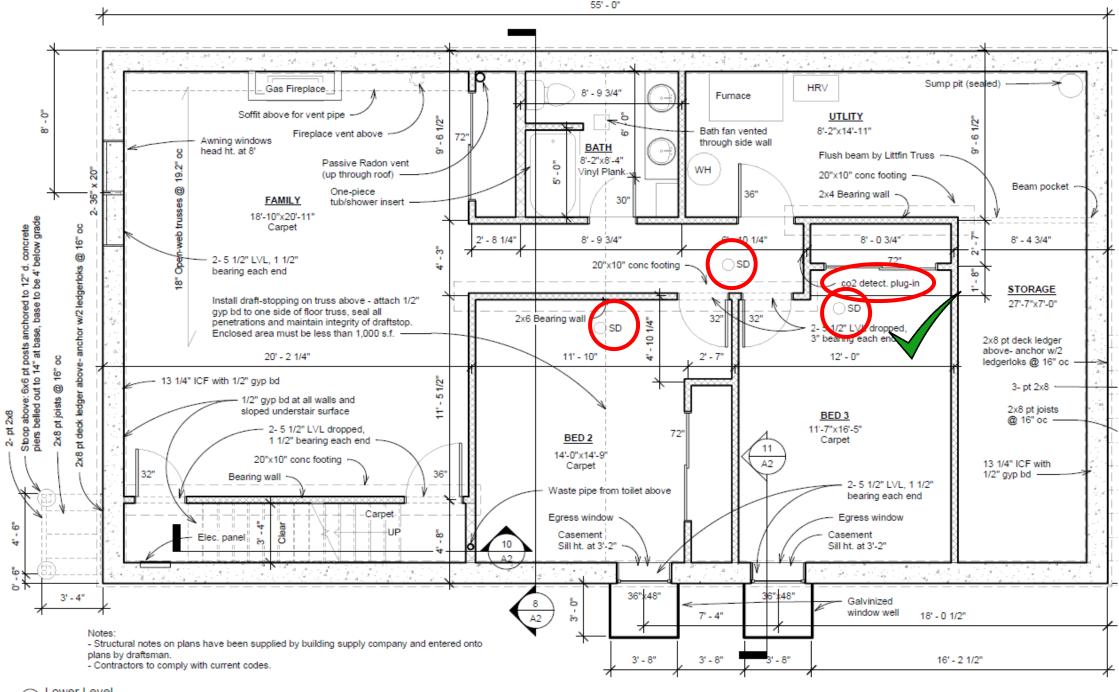
R314.3 Location. Smoke alarms shall be installed in the following locations:

- - 2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
  - 3. On each additional story of the dwelling, including basements and habitable attics and not including crawl spaces and uninhabitable attics. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.
  - 4. Smoke alarms shall be installed not less than 3 feet (914 mm) horizontally from the door or opening of a bathroom that contains a bathtub or shower unless this would prevent placement of a smoke alarm required by

55' - 0"



2 Main Level 1/4" = 1'-0"



Lower Level 1/4" = 1'-0"

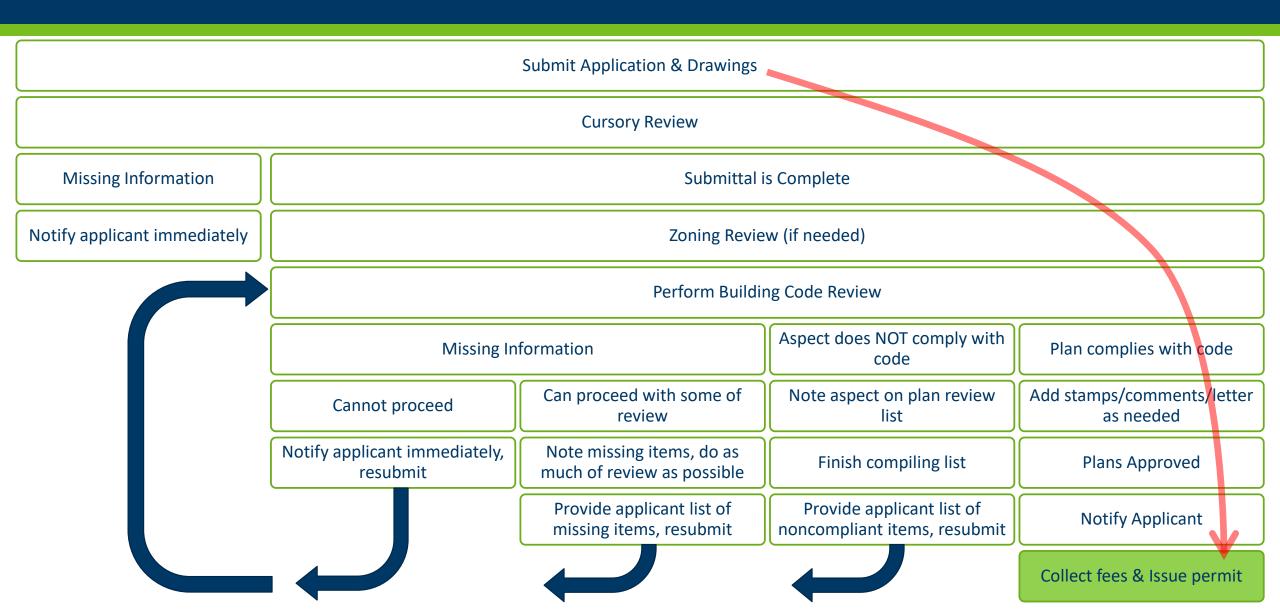
## SAC & WAC Fees

- SAC = Sewer Availability Charge.
- WAC = Water Access Charge.
- Each single-family home must pay 1 SAC & WAC fee when first connected to city sewer & water.
- Fees are set by Metropolitan Council (MET Council)
- Fees are collected by the city and paid to MET Council.
- More information: Metropolitan Council (metrocouncil.org)





# **Permit Application Process**



## Credits of Images

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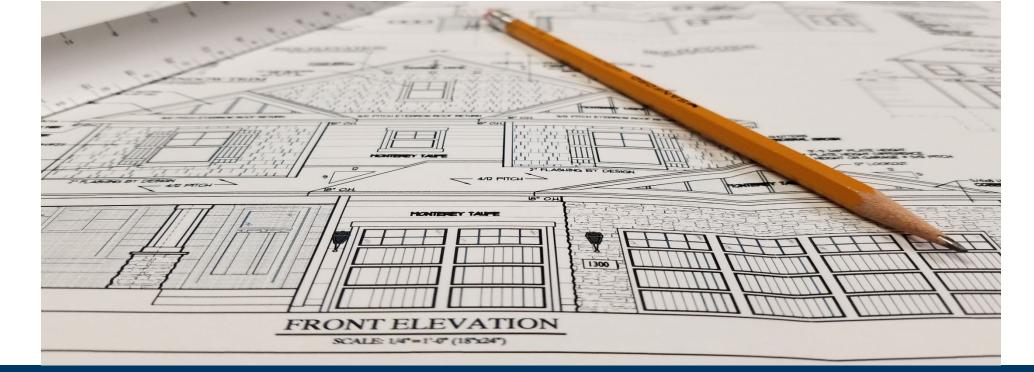
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# Residential Plan Review (The End)

**Steve Shold** 

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Construction Code Representative Code Services Construction Codes and Licensing

